



ALLJoist®

SPECIFIER GUIDE

Includes AJS[®] 140 / 150 / 20 / 190 / 25

and Versa-Lam® Beams



















product manufactured in St. Jacques, New Brunswick Canada



























ASG US 9/30/2014 r 04/10/2020









The SIMPLE FRAMING SYSTEM® Makes Designing Homes Easier

Architects, engineers and designers trust
Boise Cascade's engineered wood products
to provide a better system for
framing floors, roofs and walls.







It's the SIMPLE FRAMING SYSTEM®,

featuring beams, joists and rim boards that work together as a system, so you spend less time cutting and fitting. In fact, the SIMPLE FRAMING SYSTEM® uses fewer pieces and longer lengths than conventional framing, so you'll complete jobs in less time.

You'll Build Better Homes with the SIMPLE FRAMING SYSTEM®

Now it's easier than ever to design and build better floor systems. When you specify the SIMPLE FRAMING SYSTEM®, your clients will have fewer problems with squeaky floors and ceiling gypsum board cracks. The SIMPLE FRAMING SYSTEM® also means overall better floor and roof framing than dimension lumber allows.

Better Framing Doesn't Have to Cost More

Boise Cascade Engineered Wood Products' SIMPLE FRAMING SYSTEM® often costs less

than conventional framing methods when the resulting reduced labor and materials waste are considered. There's less sorting and cost associated with disposing of waste because you order only what you need. Although our longer lengths help your clients get the job done faster, they cost no more.

Environmentally Sound

As an added bonus, floor and roof systems built with AJS® Joists require about half the number of trees as those built with dimension lumber. This helps you design a home both you and future generations will be proud to own.

What Makes the SIMPLE FRAMING SYSTEM® So Simple?

☑ Floor and Roof Framing with AJS® Joists

Light in weight, but heavy-duty, AJS® Joists have a better strength / weight ratio than dimension lumber. Knockouts can be removed for cross-ventilation and wiring.

☑ Ceilings Framed with AJS® Joists

The consistent size of AJS® Joists helps keep gypsum board flat and free of unsightly nail pops and ugly shadows, while keeping finish work to a minimum.

✓ Versa-Lam® LVL Beams for Floor and Roof Framing

These highly-stable beams are free of the large-scale defects that plague dimension beams. The result is quieter, flatter floors (no camber) and no shrinkage-related call-backs.

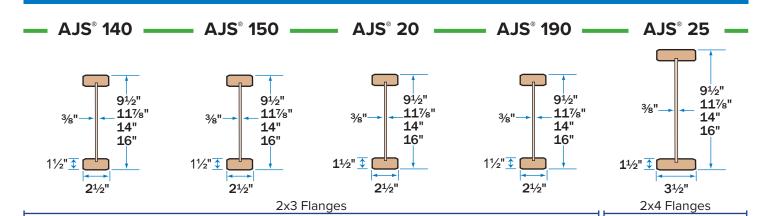
☑ Boise Cascade® Rimboard

Boise Cascade Engineered Wood
Products offer several engineered
rimboard products regionally, including
Boise Cascade® Rimboard OSB, Boise
Cascade® Rimboard and Versa-Rim®
(check supplier or Boise Cascade EWP
representative for availability). These
products work with AJS® Joists to provide
a solid connection at the critical floor/
wall intersection.

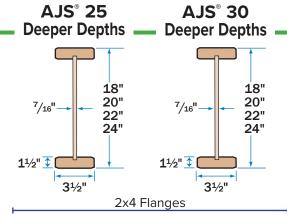
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Information on deeper depth AJS® joists is available in the ALLJOIST® Commercial Guide

ALLJOIST® Product Architectural Specifications

EVALUATION SUBJECT: AJS® Series Prefabricated Wood I-Joists

1.0 Evaluation Scope:

Compliance with the following codes:

- International Building Code® (IBC®)
- International Residential Code® (IRC®)

Properties Evaluated: Structural.

2.0 Uses: The AJS® Joists are prefabricated wood l-joists used as floor joists, roof rafters and blocking panels, to support code-required loads. Prefabricated wood l-joists described in this report comply with Section 2303.1.2 of the IBC® and Section R502.1.4 of the IRC®, for allowable stress design.

3.0 Description:

3.1 General: The AJS® Series prefabricated wood I-joists have solid-sawn lumber or composite lumber flanges and oriented strand board (OSB) webs. The top and bottom flanges are parallel, creating constant-depth joists. The web-to-web joints of the I-joists are square butt joints and conform to the specifications in the approved quality control manuals. The web-to-flange connection is a proprietary grooved connection, also conforming to the approved quality control manuals. The I-joists are available in various lengths and depths. See ICC-ES® / APA® ESR-1144 Table 1 for full description of the AJS® I-Joists.

3.2 Material Specifications:

3.2.1 Flanges: The flanges of the I-joists are sawn lumber or composite lumber conforming to the specifications in the approved quality control manuals. The composite lumber flanges are 1½ inch by 2½ inch (38 by 64 mm) spruce-

pine-fir (SPF) and are used interchangeably with any of the sawn lumber flanges of the same dimensions. The sawn lumber flange material, grade, width and depth are noted in ESR-1144, Table 1.

3.2.2 Web: Web material for the I-Joists is %-inchthick (10mm) or $7/_{16}$ -inch-thick (11mm) OSB conforming to Exposure 1 requirements of DOC PS-2, with further requirements set forth in the approved quality control manuals and manufacturing standards.

3.2.3 Adhesive: Adhesives used in the fabrication of the I-joists are exterior-type, heat durable adhesives complying with ASTM D 2559 and ASTM D 5055, and are specified in the quality control manuals and the manufacturing standards.

4.0 Design and Installation: Design of the prefabricated wood I-joists described in this report shall be in accordance with the applicable code. Additionally, the design and installation of the prefabricated wood I-joists shall comply with Sections 4.1 through 4.12 listed in ESR-1144 which include 4.1 Allowable Structural Capacity, 4.2 Fasteners, 4.3 Web Stiffeners, 4.4 Lateral Support, 4.5 Holes in I-Joist Web, 4.6 Duration of Load, 4.7 In-Service Moisture Conditions, 4.8 Repetetive-Member Use, 4.9 Member Spans, 4.10 Deflection, 4.11 Blocking Panels, & 4.12 Cantilevered Joists, and the manufacturer's installation instructions.

5.0 Conditions of Use: The AJS® Series I-joists described in this report comply with, or are suitable alternatives to what is specified in, those codes listed under ESR-1144, Section 1.0 Evaluation Scope of these specifications, subject to the following conditions:

- 5.1 AJS* joists must be installed in accordance with this report and the manufacturer's installation instructions.
- 5.2 Drawings and design details verifying compliance with this report must be submitted to the code official when requested. The drawings and calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.
- 5.3 Flanges of the I-joist may not be cut or notched, unless an engineered design prepared by a registered design professional is submitted to the code official for approval.
- 5.4 The AJS* joists are manufactured by Boise Cascade Wood Products, L.L.C. at their plant in St. Jacques, New Brunswick, Canada under an approved quality control program with inspections by APA – The Engineered Wood Association (AA-649).

6.0 Evidence Submitted:

Data in accordance with the ICC-ES Acceptance Criteria for Prefabricated Wood I-joists (AC14).

7.0 Identification:

AJS® I-joists are identified by a stamp indicating the joist model; company name (Boise Cascade Wood Products, L.L.C.); manufacturing location; evaluation report number (ESR-1144); and the name and logo of the inspection agency (APA).

AJS° Joists in Commercial Projects: The 18" and deeper depth AJS° joists are intended for commercial projects with heavier design loads and longer spans. All commercial projects utilizing AJS° joists shall have an engineer or architect of record.

About Floor Performance

Homeowner's expectations and opinions vary greatly due to the subjective nature of rating a new floor. Communication with the ultimate end user to determine their expectation is critical. *Vibration* is usually the cause of most complaints. Installing lateral bridging may help; however, squeaks may occur if not installed properly. Spacing the joists closer together does little to affect the perception of the floor's performance. The most common methods used to increase the performance and reduce vibration of wood floor systems is to *increase*

the joist depth, limit joist deflections, glue and screw a thicker, tongue-and-groove subfloor, install the joists vertically plumb with level-bearing supports, and install a direct-attached ceiling to the bottom flanges of the joists.

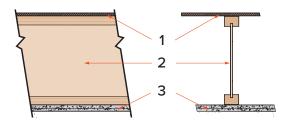
The floor span tables listed below offer three very different performance options, based on performance requirements of the homeowner.

		*	**THREE	STAR **	*	****FOUR STAR ****				CAUTION	★ MINIMUM ALLOWED E		CAUTION	
		common inc for residenti code minim still be an is:	eflection limi lustry and des al floor joists, um. Howeve sue in certain nd 11 ⁷ /8" deep iling.	ign communi 33% stiffer t r, floor perfor applications,	ty standard han L/360 mance may especially	In addition t stiffer than has been in a floor with	to providing a the three sta corporated in	ited to L/960 floor that is 1 ar floor, field to the values erformance le eowner.	00% experience to provide	Live Load deflection limited to L/360: Floors that meet the minimum building code L/360 criteria are structurally sound to carry the specified loads; however, there is a much higher risk of floor performance issues. This table should only be used for applications where floor performance is not a concern.				
Joist Depth	ALLJOIST® Series	12" o.c.	16" o.c.	19.2" o.c.	24" o.c.	12" o.c.	16" o.c.	19.2" o.c.	24" o.c.	12" o.c.	16" o.c.	19.2" o.c.	24" o.c.	
	140	17'-9"	16'–3"	15'–4"	13'–11"	13'–11"	12'-8"	11'—11"	11'—1"	19'–8"	17'-0"	15'–6"	13'–11"	
	150	18'–1"	16'–7"	15'–8"	14'-7"	14'–2"	12'–11"	12'-2"	11'–3"	20'-0"	18'–3"	16'–8"	14'–11"	
9½"	20	19'–1"	17'–5"	16'–5"	15'-4"	14'–10"	13'–6"	12'-9"	11'–10"	21'–1"	19'–3"	18'–2"	16'–4"	
	190	19'-4"	17'–8"	16'–8"	15'–6"	15'–1"	13'-9"	12'–11"	12'-0"	21'-4"	19'–7"	18'–6"	17'–3"	
	25	21'-0"	19'–1"	18'-0"	16'–9"	16'–4"	14'–10"	14'-0"	12'–11"	23'–2"	21'–1"	19'–3"	17'–2"	
	140	21'–2"	19'–4"	17'–8"	15'–10"	16'–7"	15'–1"	14'–3"	13'–3"	22'–5"	19'–5"	17'–8"	15'–10"	
	150	21'-7"	19'–8"	18'–7"	17'-0"	16'–10"	15'-4"	14'–6"	13'-5"	23'–10"	20'–10"	19'–0"	17'-0"	
111%"	20	22'-8"	20'-9"	19'–7"	18'–3"	17'–9"	16'–2"	15'–2"	14'–1"	25'–1"	22'–10"	20'–10"	18'–8"	
	190	23'-0"	21'-0"	19'–10"	18'–6"	18'–0"	16'-4"	15'–5"	14'-4"	25'–5"	23'–3"	21'–11"	19'–0"	
	25	24'–11"	22'-9"	21'–5"	18'-3"	19'–6"	17'-8"	16'–8"	15'-5"	27'-7"	24'-0"	21'–11"	18'–3"	
	140	24'-0"	21'–4"	19'–5"	17'–4"	18'–10"	17'-2"	16'–2"	15'-0"	24'–7"	21'–4"	19'–5"	17'–4"	
	150	24'-6"	22'-4"	20'–10"	18'-7"	19'–2"	17'–6"	16'–5"	15'-3"	26'–5"	22'–10"	20'–10"	18'–7"	
14"	20	25'–9"	23'–6"	22'–2"	19'–1"	20'–2"	18'-4"	17'–3"	16'-0"	28'–5"	25'–1"	22'–11"	19'–1"	
	190	26'–1"	23'–10"	22'–6"	19'–1"	20'–5"	18'-7"	17'–6"	16'-3"	28'–10"	26'–4"	23'–11"	19'–1"	
	25	28'-4"	25'–10"	22'–11"	18'-4"	22'–1"	20'–1"	18'–11"	17'-6"	30'–5"	26'-4"	22'–11"	18'–4"	
	140	26'-6"	22'–11"	20'–11"	18'-9"	20'–10"	19'-0"	17'–11"	16'–8"	26'-6"	22'–11"	20'–11"	18'–9"	
	150	27'–1"	24'-7"	22'-5"	19'–3"	21'–3"	19'-4"	18'–3"	16'–11"	28'-5"	24'–7"	22'-5"	19'–3"	
16"	20	28'–6"	26'-0"	24'–2"	19'–3"	22'–4"	20'-4"	19'–1"	17'-9"	31'–3"	27'-0"	24'-2"	19'–3"	
	190	28'–11"	26'–5"	24'–2"	19'–3"	22'–8"	20'-7"	19'–5"	18'-0"	31'–11"	28'–11"	24'-2"	19'–3"	
	25	31'–4"	27'–10"	23'–2"	18'–6"	24'–6"	22'-3"	20'–11"	18'–6"	32'-9"	27'–10"	23'-2"	18'–6"	

- Table values based on residential floor loads of 40 psf live load and 10 psf dead load (12 psf dead load for AJS* 25 joists).
- Table values assume that $^{23}/_{32}$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
 Analyze multiple span joists with BC Calc* sizing software if the length of any span is less than half the length of an adjacent span.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16" inches and less.
- Floor tile will increase dead load and may require specific deflection limits, contact Boise Cascade EWP Engineering for further information.
- This table was designed to apply to a broad range of applications. It may
 be possible to exceed the limitations of this table by analyzing a specific
 application with the BC Calc* sizing software.

Shaded values do not satisfy the requirements of the North Carolina State Building Code. Refer to the THREE STAR table when spans exceed 20 feet.

One-Hour Fire Resistance Assembly



See the US version of the Boise Cascade Fire Design & Installation Guide for specific assembly information and other fire resistive options or contact your local Boise Cascade representative.

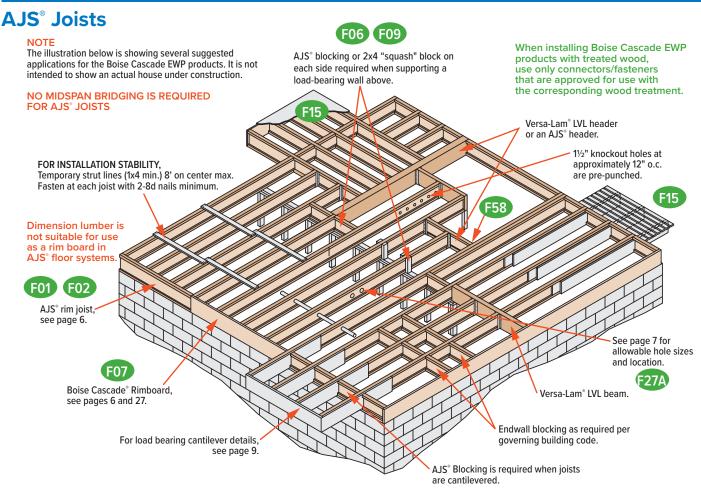
FIRE ASSEMBLY COMPONENTS

- Min. ²³/₃₂-inch T&G Wood Structural Panels. A construction adhesive must be applied to the top of the joists prior to placing sheathing. The sheets shall be installed with their long edge perpendicular to the joists with end joists centered over the top flange of joists and staggered one joist spacing with adjacent sheets.
- 2. AJS® Joists at 24" o.c. or less.
- 3. Two layers ½" Type C or two layers 5%" Type X gypsum board

SOUND ASSEMBLY COMPONENTS When constructed with resilient channels

- Add carpet & pad to fire assembly:
- Add 3½" glass fiber insulation to fire assembly:
- Add an additional layer of minimum ⁵/₈" sheathing and 9½" glass fiber insulation to fire assembly:

STC=54	IIC=68	0
STC=55	IIC=46	OI
STC=61	IIC=50	



BCI® Joists, Versa-Lam® LVL, and ALLJOIST® must be stored, installed and used in accordance with the Boise Cascade EWP Installation Guide, building codes, and to the extent not inconsistent with the Boise Cascade EWP Installation Guide, usual and customary building practices and standards. Versa-Lam® LVL, ALLJOIST® and BCI® Joists must be wrapped, covered, and stored off of the ground on stickers at all times prior to installation. Versa-Lam® LVL, ALLJOIST® and BCI® Joists are intended

only for applications that assure no exposure to weather or the elements and an environment that is free from moisture from any source, or any pest, organism or substance which degrades or damages wood or glue bonds. Failure to correctly store, use or install Versa-Lam* LVL, ALLJOIST* and BCl* Joist in accordance with the Boise Cascade EWP Installation Guide will void the limited warranty.

SAFETY WARNING

DO NOT ALLOW WORKERS ON AJS° JOISTS UNTIL ALL HANGERS, AJS° RIM JOISTS, RIM BOARDS, AJS° BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW. SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

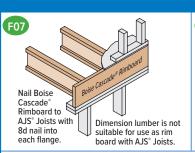
- Build a braced end wall at the end of the bay, or permanently install
 the first eight feet of AJS* Joists and the first course of sheathing. As
 an alternate, temporary sheathing may be nailed to the first four feet
 of AJS* Joists at the end of the bay.
- All hangers, AJS* rim joists, rim boards, AJS* blocking panels, and x-bracing must be completely installed and properly nailed as each AJS* Joist is set.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional AJS* Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS* Joist with two 8d nails.
- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS* Joists to within ½ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.
- Do not stack construction materials (sheathing, drywall, etc) in the middle of AJS* Joist spans, contact Boise Cascade EWP Engineering for proper storage and shoring information.

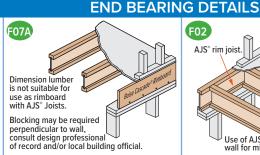
PRODUCT HANDLING TO AND AT JOB SITES

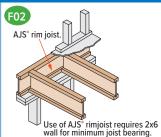
There are some differences between engineered wood products and traditional lumber products in terms of product handling: Avoid handling and storing AJS* joists in the flat direction. Versa-Lam* LVL is denser and due to the coating applied to the surface, can be more apt to sliding. Please consider these differences when transporting and handling engineered wood products.

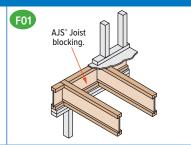


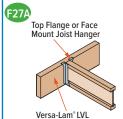
Additional floor framing details available with BC Framer® software

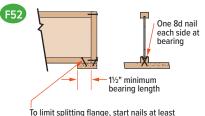




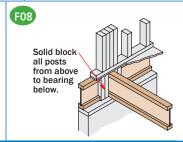


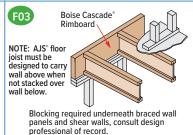




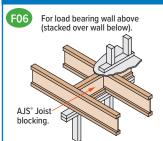


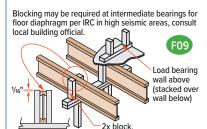
1½" from end. Nails may need to be driven at an angle to limit splitting of bearing plate.

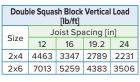




INTERMEDIATE BEARING DETAILS



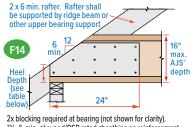




- Squash blocks are to be in full contact with upper floor and lower wall plate.
- Capacities shown are for a double squash blocks at each joist, SPF or better.

Sheathing or

rimboard closure

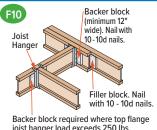


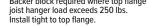
AJS® Joist Slope Cut Reinforcement Detail below restores original allowable shear/reaction value to cut end of AJS* joist. AJS* Joist shall not be

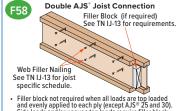
used as a collar or rafter tension tie.

²³/₃₂" min. plywood/OSB rated sheathing as reinforcement. Install reinforcement with face grain horizontal. Install on both sides of the joist, tight to bottom flange. Leave minimum 1/4" gap between reinforcement and bottom of top flange. Apply construction adhesive to contact surfaces and fasten with 3 rows of min. 10d box nails at 6" o.c. Alternate nailing from each side and clinch.

Minimum Heel Depth													
End	Roof Pitch												
Wall Bearing	6/12 7/12 8/12 9/12 10/12												
2 x 4	43/8"	45/16"	41/4"	41/4"	41/4"	41/4"							
2 x 6	3%"	33/16"	25/16"	2¾"	29/16"	21/4"							







- Filler block not required when all loads are to and evenly applied to each ply (except AJS® 2 Side loads and/or uneven top loads require fil See Boise Cascade Technical Note IJ-13 for fu information.
- Fasten floor sheathing to each ply per diaphra

p loaded 5 and 30). Ier block. rther	For load be and 9. Upli considered
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F05

AJS® Joist

required for

cantilever.

blocking

earing cantilever, see pages 8 ift on backspan shall be d in all cantilever designs

LATERAL SUPPORT

- AJS* Joists shall be laterally supported at the ends with hangers, rimboard, AJS* rim joist or blocking panels. AJS* blocking panels or rimboard are required at cantilever supports
- Blocking may be required at intermediate bearings for floor diaphragm per IRC® in high seismic areas, consult local building official.

- MINIMUM BEARING LENGTH FOR AJS® JOISTS

 Minimum end bearing: 1½" for all AJS® Joists. 3½" is required at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values. Refer to the building code evaluation report or the BC Calc[®] software.

NAILING REQUIREMENTS

- AJS° rim joist, rim board or closure panel to AJS° joist:
- Rims or closure panel 1¼ inches thick and less: 2-8d nails, one each in the top and bottom flange. AJS* 140/150/20/190 rim joist: 2-16d box nails, one
- each in the top and bottom flange. AJS® 25 rim joist: Toe-nail top flange to rim joist
- with 2-10d box nails, one each side of flange.

 AJS rim joist, rim board or AJS blocking panel to support
- Min. 8d nails @ 6" o.c. per IRC°. Connection per design professional of record's specification for shear transfer.
- S^{*} joist to support: 2-8d nails, one on each side of the web, placed 1½ inches minimum from the end of the AJS^{*} Joist to limit splitting.

- Sheathing to AJS* joist:

 Prescriptive residential floor sheathing nailing requires 8d common nails @ 6" o.c. on edges and @ 12" o.c. in the field (IRC* Table R602.3(1)).
 See closest allowable nail spacing limits on page 27
 - for floor diaphragm nailing specified at closer
 - spacing than IRC.

 Maximum bracing spacing for full lateral stability:
 18" for AJS* 140/150/20/190, 24" for larger AJS* joist
 - 14 gauge staples may be substituted for 8d nails if
 - the staples penetrate at least 1 inch into the joist. Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.

BACKER AND FILLER BLOCK DIMENSIONS

Series	Backer Block Thickness	Filler Block Thickness
140 150 20 190	1½" or two ½" wood panels	2 x _ + ⁷ / ₁₆ " or ½" wood panel
25	2 x _ lumber	Double 2 x _ lumber

- Cut backer and filler blocks to a maximum depth equal to the web depth minus \mathcal{V}_{a} to avoid a forced fit.
- For deeper AJS® Joists, stack 2x lumber or use multiple pieces of 3/4" wood panels.

WEB STIFFENER REQUIREMENTS

· See Web Stiffener Requirements on page 10.

PROTECT AJS° JOISTS FROM THE WEATHER

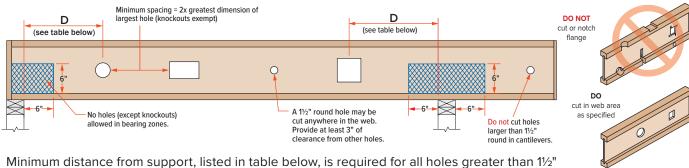
AJS® Joists are intended only for applications that provide permanent protection from the weather. Bundles of product should be covered and stored off of the ground on stickers.

AJS° RIM JOISTS AND BLOCKING

Joist Depth [in]	Vertical Load Transfer Capacity [plf}
9½"	1875
11%"	1680
14"	1500
16"	1340

Web stiffeners required at each end of blocking panel. Distance between stiffeners must be less than 24"

 AJS^* Joists are manufactured with $1\frac{1}{2}$ " round perforated knockouts in the web at approximately 12" on center



Minimum distance from support, listed in table below, is required for all holes greater than 1½"																
			MINIM	UM DIS	TANCE (D) FROM	ANY S	UPPOR'	т то тн	E CENT	ERLINE [,]	OF THE	HOLE			
Round Ho	le Diamet	er [in]	2	3	4	5	6	6½	7	8	8%	9	10	11	12	13
Rectangula	ar Hole Si	de [in]	-	-	2	4	6	6	-	-	-	-	-	-	-	-
		8	2'-0"	2'-5"	2'-11"	3'-5"	3'–10"	4'-0"								
Any 9½" Joist	Span [ft]	12	3'-0"	3'-8"	4'-5"	5'–1"	5'–10"	6'-0"								
20151		16	4'-0"	4'-11"	5'-11"	6'–10"	7'-9"	8'-0"								
Round Ho	le Diamet	er [in]	2	3	4	5	6	6½	7	8	8%	9	10	11	12	13
Rectangula	ar Hole Si	de [in]	-	-	-	2	3	4	5	7	8	-	-	-	-	-
		8	1'-0"	1'-5"	1'–10"	2'-3"	2'-8"	2'–11"	3'–1"	3'-6"	3'–11"					
Any	Span	12	1'-5"	2'–1"	2'-9"	3'-5"	4'-0"	4'-4"	4'-8"	5'-4"	5'–11"					
11%" Joist	[ft]	16	1'-11"	2'–10"	3'-8"	4'-6"	5'-5"	5'–10"	6'-3"	7'–1"	7'–10"					
		20	2'-5"	3'-6"	4'-7"	5'-8"	6'-9"	7'–3"	7'–10"	8'–11"	9'–10"					
Round Ho	le Diamet	er [in]	2	3	4	5	6	6½	7	8	8%	9	10	11	12	13
Rectangula	ar Hole Si	de [in]	-	-	-	-	2	3	3	5	6	6	8	9	-	-
		8	1'-0"	1'–1"	1'–2"	1'-4"	1'–8"	1'–11"	2'–1"	2'-6"	2'–10"	2'–11"	3'-4"	3'-9"		
		12	1'-0"	1'–1"	1'-4"	2'-0"	2'–7"	2'–11"	3'-2"	3'–10"	4'-4"	4'-5"	5'-0"	5'–7"		
Any 14"	Span [ft]	16	1'-0"	1'–1"	1'–10"	2'-8"	3'-5"	3'–10"	4'-3"	5'–1"	5'-9"	5'–11"	6'-8"	7'-6"		
Joist		20	1'-0"	1'–3"	2'-4"	3'-4"	4'-4"	4'–10"	5'-4"	6'-4"	7'–3"	7'–4"	8'-5"	9'–5"		
		24	1'-0"	1'-7"	2'-9"	4'-0"	5'-2"	5'–10"	6'-5"	7'-8"	8'-8"	8'–10"	10'–1"	11'–3"		
Round Ho	le Diamet	er [in]	2	3	4	5	6	6½	7	8	8%	9	10	11	12	13
Rectangula	ar Hole Si	de [in]	-	-	-	-	-	-	2	3	5	5	6	8	9	10
		8	1'-0"	1'–1"	1'–2"	1'–2"	1'–3"	1'–3"	1'–3"	1'-8"	2'-0"	2'–1"	2'-5"	2'–10"	3'–2"	3'–7"
		12	1'-0"	1'–1"	1'–2"	1'–2"	1'-4"	1'-8"	1'–11"	2'-6"	3'-0"	3'–1"	3'-8"	4'-3"	4'–10"	5'-5"
Any 16"	Span [ft]	16	1'-0"	1'–1"	1'–2"	1'–2"	1'–10"	2'-2"	2'-7"	3'-4"	4'-0"	4'-2"	4'–11"	5'-8"	6'-5"	7'–2"
Joist		20	1'-0"	1'–1"	1'–2"	1'-4"	2'-3"	2'-9"	3'-3"	4'-3"	5'–1"	5'–2"	6'–2"	7'–1"	8'–1"	9'-0"
		24	1'-0"	1'-1"	1'–2"	1'–7"	2'-9"	3'-4"	3'–11"	5'–1"	6'–1"	6'–3"	7'-4"	8'-6"	0' 2"	10'–10

- · Select a table row based on joist depth and the actual joist span rounded up to the nearest table span. Scan across the row to the column headed by the appropriate round hole diameter or rectangular hole side. Use the longest side of a rectangular hole. The table value is the closest that the centerline of the hole may be to the centerline of the nearest support.
- The entire web may be cut out. $\ensuremath{\text{DO NOT}}$ cut the flanges. Holes apply to either single or multiple joists in repetitive member conditions.
- · For multiple holes, the amount of uncut web between holes must equal at least twice the diameter (or longest side) of the largest hole.
- 11/2" round knockouts in the web may be removed by using a short piece of metal pipe and hammer.
- · Holes may be positioned vertically in the web, provided they don't extend into either flange.
- This table was designed to apply to design conditions covered by uniform load PLF tables only, shown elsewhere in this publication. Use BC Calc® software to check other hole sizes or holes under other design conditions, including joists supporting concentrated loads. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC Calc® software.

AJS[®] Joists

닭	ies	Roof Truss Span [ft]			Ro	of To		ad [p	sf]		
Joist Depth [in]	Series	T.E.		35			45			55	
ist	Joist 9	oof			J	loist	Spaci	ng [in]		
윽	윽	SS	16	19.2	24	16	19.2	24	16	19.2	24
		24	0	0	1	0	0	Χ	0	Х	Х
		26	0	0	1	0	1	Χ	1	X	X
		28	0	0	Χ	0	1	Χ	1	X	Х
	=	30	0	0	Χ	0	Χ	Χ	Χ	X	Χ
	91/2"	32	0	0	Χ	1	Χ	Χ	Χ	X	Х
	0,	34	0	1	Χ	1	Χ	Χ	Χ	X	Χ
		36	0	1	Χ	1	Χ	Χ	Χ	X	Х
		38	0	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ
		40	0	Χ	Χ	Χ	Х	Χ	Χ	Χ	X
		24	0	0	0	0	0	0	0	0	Χ
		26	0	0	0	0	0	1	0	0	Х
		28	0	0	0	0	0	1	0	1	X
	=	30	0	0	0	0	0	Χ	0	1	Χ
	117/8"	32	0	0	0	0	0	Χ	0	1	Χ
	`	34	0	0	1	0	0	Χ	0	X	Χ
		36	0	0	1	0	1	Χ	1	X	Χ
9		38	0	0	1	0	1	Χ	1	X	X
AJS® 140		40	0	0	Χ	0	1	Χ	1	X	Х
JS		24	0	0	0	0	0	0	0	0	WS
⋖		26	0	0	0	0	0	WS	0	0	WS
		28	0	0	0	0	0	WS	0	0	1
	-	30	0	0	0	0	0	WS	0	0	1
	14	32	0	0	0	0	0	WS	0	WS	Χ
		34	0	0	0	0	0	1	0	WS	Χ
		36	0	0	WS	0	0	1	0	1	Χ
		38	0	0	WS	0	0	1	0	1	Χ
		40	0	0	WS	0	WS	Χ	0	1	Χ
		24	0	0	0	0	0	0	0	0	WS
		26	0	0	0	0	0	WS	0	0	WS
		28	0	0	0	0	0	WS	0	0	WS
	_	30	0	0	0	0	0	WS	0	0	WS
	16"	32	0	0	0	0	0	WS	0	WS	1
		34	0	0	WS	0	0	WS	0	WS	1
		36	0	0	WS	0	0	WS	0	WS	1
		38	0	0	WS	0	WS	WS	0	WS	Х
		40	0	0	WS	0	WS	1	0	WS	X

£	es	SS			Ro	oof Total Load [psf]							
Joist Depth [in]	Joist Series	Roof Truss Span [ft]		35		45 55							
ist [[in	ist	of			J	Joist Spacing [in]							
9	욱	S _N	16	19.2	24	16	19.2	24	16	19.2	24		
		24	0	0	1	0	0	2	0	1	Χ		
		26	0	0	1	0	1	Χ	1	2	Χ		
		28	0	0	1	0	1	Χ	1	Χ	Χ		
	_	30	0	0	2	0	1	Χ	1	Χ	Χ		
	91/2"	32	0	0	2	1	2	Χ	2	Χ	Χ		
	0,	34	0	1	Χ	1	2	Χ	2	Χ	Χ		
		36	0	1	Χ	1	Χ	Χ	Χ	Χ	Χ		
		38	0	1	Χ	1	Χ	Χ	Χ	Χ	Χ		
		40	0	1	Χ	2	Χ	Χ	Χ	Χ	Χ		
		24	0	0	0	0	0	0	0	0	1		
		26	0	0	0	0	0	1	0	0	1		
		28	0	0	0	0	0	1	0	1	Χ		
	=	30	0	0	0	0	0	1	0	1	Χ		
	117/8"	32	0	0	0	0	0	1	0	1	Х		
	_	34	0	0	1	0	0	2	0	1	Χ		
		36	0	0	1	0	1	Χ	1	2	Χ		
00		38	0	0	1	0	1	Χ	1	2	Х		
AJS [®] 150		40	0	0	1	0	1	Χ	1	Χ	Χ		
JS		24	0	0	0	0	0	0	0	0	WS		
⋖		26	0	0	0	0	0	WS	0	0	WS		
		28	0	0	0	0	0	WS	0	0	1		
	_	30	0	0	0	0	0	WS	0	WS	1		
	14	32	0	0	0	0	0	WS	0	WS	1		
		34	0	0	WS	0	0	1	0	WS	Χ		
		36	0	0	WS	0	0	1	0	1	Χ		
		38	0	0	WS	0	WS	1	0	1	Χ		
		40	0	0	WS	0	WS	1	WS	1	Χ		
		24	0	0	0	0	0	0	0	0	WS		
		26	0	0	0	0	0	WS	0	0	WS		
		28	0	0	0	0	0	WS	0	0	WS		
	=	30	0	0	0	0	0	WS	0	WS	WS		
	16"	32	0	0	0	0	0	WS	0	WS	1		
		34	0	0	WS	0	0	WS	0	WS	1		
		36	0	0	WS	0	0	WS	0	WS	1		
		38	0	0	WS	0	WS	WS	0	WS	1		
		40	0	0	WS	0	WS	1	WS	WS	Χ		

ح	Ś	v			Ro	of To	tal Lo	ad [n	sf]													
ept	erie	rus		35		00	45	raa [p	0.j	55												
Joist Depth [in]	Joist Series	Roof Truss Span [ft]		- 55		loist !	Spaci	na (ir	1	- 55												
-je	jö	Sg	16	19.2	24	16	19.2	24	16	19.2	24											
		24	0	0	1	0	0	2	0	2	X											
		26	0	0	1	0	1	X	1	2	Х											
		28	0	0	1	0	1	X	1	2	Х											
		30	0	0	2	0	2	X	1	X	Х											
	91/2"	32	0	0	2	1	2	X	2	X	Х											
	0	34	0	1	2	1	2	X	2	X	Х											
		36	0	1	X	1	X	X	2	X	Х											
		38	0	1	Х	1	Х	Х	X	Х	Х											
		40	0	2	X	2	X	X	X	X	Х											
		24	0	0	0	0	0	WS	0	0	1											
		26	0	0	0	0	0	1	0	0	2											
		28	0	0	0	0	0	1	0	1	Х											
		30	0	0	0	0	0	1	0	1	Х											
	117/8"	32	0	0	WS	0	0	2	0	1	Х											
	÷	34	0	0	1	0	0	Х	0	1	Х											
		36	0	0	1	0	1	Х	1	2	Х											
0		38	0	0	1	0	1	Χ	1	2	Х											
AJS° 20		40	0	0	1	0	1	Χ	1	Χ	Х											
JS		24	0	0	0	0	0	0	0	0	WS											
⋖		26	0	0	0	0	0	WS	0	0	WS											
		28	0	0	0	0	0	WS	0	0	1											
		30	0	0	0	0	0	WS	0	WS	1											
	4	32	0	0	0	0	0	WS	0	WS	1											
		34	0	0	WS	0	0	1	0	WS	Χ											
		36	0	0	WS	0	WS	1	0	1	Χ											
		38			38	38	38	38	38	38	38	38		0	0	WS	0	WS	1	0	1	Χ
		40	0	0	WS	0	WS	1	WS	1	Χ											
		24	0	0	0	0	0	0	0	0	WS											
		26	0	0	0	0	0	WS	0	0	WS											
		28	0	0	0	0	0	WS	0	0	WS											
		30	0	0	0	0	0	WS	0	WS	WS											
	16"	32	0	0	0	0	0	WS	0	WS	1											
		34	0	0	WS	0	0	WS	0	WS	1											
		36	0	0	WS	0	WS	WS	0	WS	1											
		38	0	0	WS	0	WS	WS	WS	WS	1											
		40	0	0	WS	0	WS	1	WS	WS	Χ											

£	Se	SS			Ro	of To	tal Lo	ad [p	sf]		
Jep J	eri	ΞΞ		35			45			55	
Joist Depth [in]	Joist Series	Roof Truss Span [ft]			J	loist :	Spaci	ng [in]		
9	윽	50	16	19.2	24	16	19.2	24	16	19.2	24
		24	0	0	1	0	0	2	0	2	Χ
		26	0	0	1	0	1	Χ	1	2	Χ
		28	0	0	1	0	1	Χ	1	Χ	Χ
	_	30	0	0	2	0	2	Χ	1	Χ	Χ
	91/2"	32	0	0	2	1	2	Χ	2	Χ	Χ
	0,	34	0	1	Χ	1	2	Χ	2	Χ	Χ
		36	0	1	Χ	1	Χ	Χ	Χ	Χ	Χ
		38	0	1	Χ	2	Χ	Χ	Χ	Χ	Χ
		40	0	2	Χ	2	Χ	Χ	Χ	Χ	Χ
		24	0	0	0	0	0	WS	0	0	1
		26	0	0	0	0	0	1	0	0	2
		28	0	0	0	0	0	1	0	1	Χ
	=	30	0	0	0	0	0	1	0	1	Χ
	117/8"	32	0	0	WS	0	0	2	0	1	Х
	_	34	0	0	1	0	0	Χ	0	1	Х
		36	0	0	1	0	1	Χ	1	2	Х
0		38	0	0	1	0	1	Χ	1	Χ	Х
15		40	0	0	1	0	1	Χ	1	Χ	Χ
AJS [®] 190		24	0	0	0	0	0	0	0	0	WS
⋖		26	0	0	0	0	0	WS	0	0	WS
		28	0	0	0	0	0	WS	0	WS	1
	_	30	0	0	0	0	0	WS	0	WS	1
	<u>†</u>	32	0	0	0	0	0	WS	0	WS	1
		34	0	0	WS	0	0	1	0	WS	Χ
		36	0	0	WS	0	WS	1	0	1	Χ
		38	0	0	WS	0	WS	1	WS	1	Χ
		40	0	0	WS	0	WS	1	WS	1	Χ
		24	0	0	0	0	0	0	0	0	WS
		26	0	0	0	0	0	WS	0	0	WS
		28	0	0	0	0	0	WS	0	0	WS
	_	30	0	0	0	0	0	WS	0	WS	WS
	16"	32	0	0	0	0	0	WS	0	WS	1
		34	0	0	WS	0	0	WS	0	WS	1
		36	0	0	WS	0	WS	WS	0	WS	1
		38	0	0	WS	0	WS	WS	WS	WS	1
		40	0	0	WS	0	WS	1	WS	WS	Χ

STOP	-	S	S.			Ro	of To	tal Lo	ad [n	sfl		
\$\frac{24}{26} \text{ 0 } 0 & 1 & 0 & 0 & \text{ N } 0 & 2 & 2 \\ 26 & 0 & 0 & 1 & 0 & 1 & \text{ N } 1 & \text{ N } 1 & \text{ N } 2 \\ 28 & 0 & 0 & 2 & 0 & 1 & \text{ N } 1 & \text{ N } 1 & \text{ N } 1 \\ 30 & 0 & 0 & 2 & 0 & 2 & \text{ N } 2 & \text{ N } 2 \\ 32 & 0 & 0 & \text{ N } 1 & \text{ N } 1 & \text{ N } X & \text{ N } X \\ 36 & 0 & 1 & \text{ N } 1 & \text{ N } 1 & \text{ N } X & \text{ N } X \\ 38 & 0 & 2 & \text{ N } 2 & \text{ N } X & \text{ N } X \\ 40 & 0 & 2 & \text{ N } 2 & \text{ N } X & \text{ N } X \\ 24 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 28 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 1 \\ 30 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 1 \\ 30 & 0 & 0 & 0 & 0 & 0 & 0 & 2 & 0 & 1 \\ 34 & 0 & 0 & 1 & 0 & 1 & \text{ N } 1 & \text{ N } X \\ 38 & 0 & 0 & 1 & 0 & 1 & \text{ N } 1 & \text{ N } 1 \\ 30 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 1 \\ 30 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 1 \\ 34 & 0 & 0 & 1 & 0 & 1 & \text{ N } 1 & \text{ N } 1 \\ 40 & 0 & 0 & 2 & 0 & 1 & \text{ N } 1 & \text{ N } 1 \\ 24 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 28 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 28 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 36 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 30 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 38 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 34 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 24 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 24 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 24 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 24 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 24 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 25 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0	ept	erie	.≅Œ		35					,	55	
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												1
												1
40 0 0 0 0 0 1 0 WS :			40	0	0	0	0	0	1	0	WS	2

KEY TO TABLE

0..... No Reinforcement Required

WS..... Web Stiffeners at Support

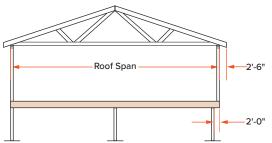
1..... Web Stiffeners Plus One Reinforcer

2..... Web Stiffeners Plus Two Reinforcers

X..... Use Deeper Joists or Closer Spacing

Notes

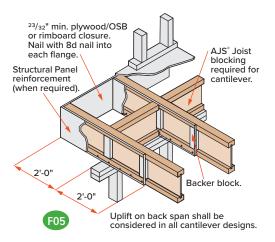
- Cut 48" long reinforcers to match the joist depth. Use ²³/₃₂" APA Rated Sheathing, Exposure 1, 48/24 Span Rating panels. The face grain must be horizontal (measure the 48" dimension along the long edge of the panel).
- Fasten the reinforcer to the joist flanges with 8d nails at 6" o. c. When reinforcing both sides, stagger the nails to avoid splitting the joist flanges.
- Attach web stiffeners per intermediate Web Stiffener Nailing Schedule on page 10.
- Use the BC Calc* software to analyze conditions that are not covered by this table.



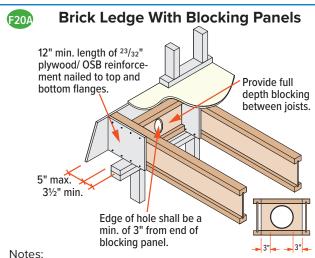
• The tables and details on pages 8 and 9 indicate the type of reinforcements, if any, that are required for loadbearing cantilevers up to a maximum length of 2'-0". Cantilevers longer than 2'-0" cannot be reinforced. However, longer cantilevers with lower loads may be allowable without reinforcement. Analyze specific applications with the BC Calc® software.

PLYWOOD / OSB REINFORCEMENT (If Required per Table on page 8)

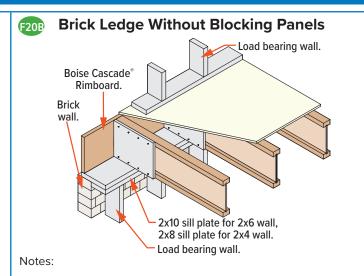
- $^{23}/_{32}$ " Min. x 48" long plywood / OSB rated sheathing must match the full depth of the AJS® Joist. Nail to the AJS Joist with 8d nails at 6" o.c. and nail with 4-8d nails into backer block. When reinforcing both sides, stagger nails to limit splitting. Install with horizontal face grain.
- These requirements assume a 100 PLF wall load and apply to AJS® Joists. Additional support may be required for other loadings. See BC Calc® software.
- Contact Boise Cascade EWP Engineering for reinforcement requirements on AJS® Joist depths greater than 16".



Brick Ledge Load Bearing Cantilever



- 1. Use ²³/₃₂" min plywood/OSB rated sheathing. Install full depth of joist with face grain parallel to joist. Plywood reinforcement to bear fully on wall plate. Nail plywood to top and bottom joist flanges with 21/2
- (8d) nails at 3" on center except 91/2" joists, install nails at 21/2" on center.
- 2. Provide full depth blocking between joists.
- 3. Edge of hole shall be at a minimum of 3" from end of blocking panel.



- 1. Use ²³/₃₂" min plywood/OSB rated sheathing. Install full depth of joist with face grain parallel to joist. Plywood reinforcement to bear fully on wall plate. Nail plywood to top and bottom joist flanges with 21/2
- (8d) nails at 3" on center except 91/2" joists, install nails at 21/2" on center.
- 2. See page 6 for joist and rimboard connection details.

	Roof					Ro	of Live	Load (osf)				
Joist	Truss		20 psf			30 psf	:		40 psf			50 psf	
Depth	Span					Jo	ist Spa						
(inches)	(ft)	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"
	24'	0	0	0	0	0	1	0	1	1	0	1	1
	26'	0	0	0	0	0	1	0	1	1	0	1	2
	28'	0	0	0	0	0	1	0	1	1	0	1	2
91/2"	30'	0	0	0	0	0	1	0	1	1	1	1	2
	32'	0	0	1	0	1	1	0	1	2	1	2	2
	34'	0	0	Х	0	X	X	0	1	Χ	1	2	Х
	36'	0	Χ	X	0	Χ	X	1	Χ	X	1	Χ	X
	24'	0	0	0	0	0	0	0	0	0	0	0	1
	26'	0	0	0	0	0	0	0	0	0	0	0	1
	28'	0	0	0	0	0	0	0	0	1	0	1	1
111//8"	30'	0	0	0	0	0	0	0	0	1	0	1	1
	32'	0	0	0	0	0	0	0	0	1	0	1	1
	34'	0	0	0	0	1	1	0	1	1	0	1	1
	36'	0	0	0	0	1	1	0	1	1	0	1	X
	24'	0	0	0	0	0	0	0	0	0	0	0	0
	26'	0	0	0	0	0	0	0	0	0	0	0	0
	28'	0	0	0	0	0	0	0	0	0	0	0	1
14"	30'	0	0	0	0	0	0	0	0	0	0	0	1
	32'	0	0	0	0	0	0	0	0	0	0	0	1
	34'	0	0	0	0	0	0	0	0	1	0	0	1
	36'	0	0	0	0	0	0	0	0	1	0	1	Χ

Brick Ledge Reinforcement Table

Table Design Assumptions

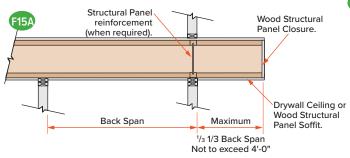
Roof Loading: 15 psf dead load plus a 100 PLF wall self-weight, in addition to roof live load shown. Maximum 2'-6" overhangs assumed on roof trusses.

Floor Loading: 40 psf live load plus 10 psf dead load, backspans not to exceed maximum floor spans shown on page 4.

KEY TO TABLE:

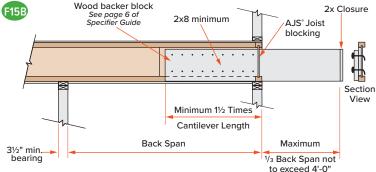
- 0 = No Reinforcement Required
- = Reinforcement Required One Side of Joist
- = Reinforcement Required Both Sides of Joist
- Use Deeper Joists or Closer Spacing

AJS* Joists are intended only for applications that provide permanent protection from the weather. Impervious moisture barrier systems shall be detailed and installed in details F15A and F15B in accordance with 2018 IBC* Sections 107.2.5 and 110.3.6.



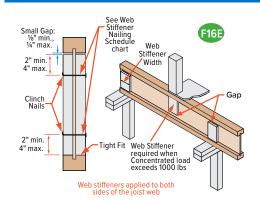
• Analyze AJS® Joist cantilever condition with BC Calc® software.

Fasten the 2x8 minimum to the AJS® Joist by nailing through the backer block and joist web with 2 rows of 10d nails at 6" on-center. Clinch all nails. For AJS® 25/30 joists, nail each side with 2 rows (4 rows total) of 16d nails at 6" on-center.



- Loading shall not exceed 60 psf live load and 10 psf dead load. At least three joist members shall be present and spaced at 24" o.c. or less.
- Lumber joist shall be No. 2 Dense Southern Pine, No.1/No.2 SPF, No.2 Hem-fir, or No.2 Douglas fir, or higher grade.
- Provide positive drainage, durable materials, and venting as required in 2018 IBC* Sections 2304.12.2.5 and 2304.12.2.6. Lumber joist shall be sloped.

Web Stiffener Requirements



W	eb Stiffener S	Specification	าร						
AJS [®] Series	For Structral Capacity (Min. Thick)	Lateral Restraint in Hanger	Minimum Width						
140 / 150 / 20 / 190	1"	1"	2 ⁵ /16"						
AJS [®] 25	2x4 lumber (vertical)								

Web	Stiffener Nailing Sche	dule
AJS [®] Series	Joist Depth	Nailing
140 / 150 / 20 /	9½" – 11%"	3-10d
190 / 25	14" – 16"	5-10d

NOTES

- · Web stiffeners are optional except as noted below.
- Web stiffeners are always required for 18" and deeper AJS® joists at all bearing locations.
- Web stiffeners are always required in hangers that do not extend up to support the top flange of the AJS* Joist.
 Web stiffeners may be required with certain sloped or skewed hangers or to achieve uplift values. Refer to the hanger manufacturer's installation requirements.
- Web stiffeners are always required in certain roof applications. See Roof Framing Details on page 15.
- Web stiffeners are always required under concentrated loads that exceed 1000 pounds. Install the web stiffeners snug to the top flange in this situation. Follow the nailing schedule for intermediate bearings.
- Web stiffeners may be cut from structural rated wood panels, engineered rimboard or 2x lumber (AJS* 25 only).
- For Structural Capacity: Web stiffeners needed to increase the AJS[®] Joist's reaction capacity at a specific bearing location.
- Lateral Restraint in Hanger: Web stiffeners required when hanger does not laterally support the top flange (e.g., adjustable height hangers). Web stiffeners may be of multiple thickness (e.g., AJS* 20, double ½" panel OK).
- Web stiffeners may be used to increase allowable reaction values. See AJS[®] Design Properties on page 26 or the BC Calc[®] software.

Large Rectangular Holes in AJS® Joists

Hole size table based on maximum uniform load of 40 psf live load and 15 psf dead load, at maximum spacing of 24" on-center.

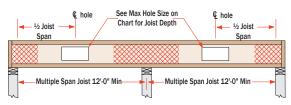
Single Span Joist See Max Hole Size on Chart for Joist Depth Minimum 2x diameter/width of largest hole Simple Span Joist 6'-0" Min

Notes:

Additional holes may be cut in the web provided they meet the specifications as shown in the hole distance chart shown above or as allowed using BC Calc* sizing software.

	Maximun	n Hole Size					
Joist Depth	Simple Span	Multiple Span					
91/2"	6" x 12"	6" x 7"					
11%"	8" x 13"	8" x 8"					
14"	9" x 16"	8" x 13"					
14"	10" x 14"	9" x 11"					
16"	11" x 16"	10" x 14"					
10	12" x 15"	11" x 12"					

Multiple Span Joist



Larger holes may be possible for either Single or Multiple span joists; use BC Calc* sizing software for specific analysis.

(in pounds per linear foot [PLF])

									on ar	ion						
			AJ	S [®] 140	0 Seri	ies					AJ	S [®] 150	0 Ser	ies		
			_	_	ge W								ge W			
	91/	⁄2"	117		14		16	5"	91	/2"	117			1"	16	ĵ"
	AJS®		AJS		AJS		AJS		AJS		AJS	150	AJS		AJS	
	Live	Total	Live	Total	Live	Total	Live	Total	Live	Total	Live	Total	Live	Total	Live	Total
Length L	Load	Load 313	Load -	Load 318	Load -	Load 320	Load -	Load 323	Load	Load 313	Load -	Load 318	Load -	Load 320	Load -	Load 323
7	_	268		272		274		277		268	_	272	_	274	_	277
8	_	235		238	_	240		242		235	_	238	_	240	_	242
9	_	208	_	212	_	213	_	215	_	208	_	212	_	213	_	215
10	170	188	_	191	-	192	-	194	180	188	-	191	_	192	-	194
11	131	161	-	173	-	174	-	176	139	170	-	173	_	174	-	176
12 '	103	136	-	159	-	160	-	161	109	156	-	159	-	160	-	161
13	82	115	136	146	-	147	-	149	87	133	144	146	-	147	-	149
14	67	100	111	129	-	137	-	138	71	115	117	136	-	137	-	138
15	55	87	91	112	-	128	-	129	58	100	97	127	-	128	-	129
16	46	76	76	99	110	119	-	121	48	88	81	114	116	120	-	121
17			64	87	93	105	-	114	41	78	68	101	98	112	-	114
18			54	78	79	94	106	107			58	90	84	106	-	107
19			46	70	68	84	91	98			49	80	72	97	96	102
20			40	63	58	76	79	88			43	73	62	87	83	97
21					51	69	68	80					54	79	73	92
22					44	63	60	73					47	72	64	84
23							53	67					41	66	56	76
24							47	61							49	70
25							41	56							44	65
26 27																
28																
28																
30																

- Total Load values are limited by shear, moment, or deflection equal to L/240.
- Live Load values are limited by deflection equal to L/480.
 For deflection limits of L/360 and L/960, multiply the Live Load values by 1.33 and 0.50 respectively.
- Both the Total Load and Live Load columns must be checked. Where a Live Load value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists.
 Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC Calc* software if the length of any span is less than half the length of an adjacent span.

- Table values do not consider composite action from gluing and nailing floor sheathing (composite action is considered in floor span tables on page 4).
- Total Load values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- For assistance with floor design, consult the section *About Floor Performance* on page 4.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC Calc* software.

(in pounds per linear foot [PLF])

						100	% Lo	ad D	urat	ion						
) Seri ge W						_	S° 190 Flan				
	91 AJS		117 AJS	%" ® 20	1 ² AJS		16 AJS	5" ® 20	91 AJS	⁄2" ³ 190	117 AJS		1 ² AJS ⁶		16 AJS	
Span Length	Live Load	Total Load	Live Load	Total Load	Live Load	Total Load	Live Load	Total Load	Live Load	Total Load	Live Load	Total Load	Live Load	Total Load	Live Load	Total Load
6	-	313	-	318	-	320	-	323	-	313	-	318	-	320	-	323
7	-	268	-	272	-	274	-	277	-	268	-	272	-	274	-	277
8	-	235	-	238	-	240	-	242	-	235	-	238	-	240	-	242
9	-	208	-	212	-	213	-	215	-	208	-	212	-	213	-	215
10	-	188	-	191	-	192	-	194	-	188	-	191	-	192	-	194
11	161	170	-	173	-	174	-	176	168	170	-	173	-	174	-	176
12	128	156	-	159	-	160	-	161	133	156	-	159	-	160	-	161
13	102	144	-	146	-	147	-	149	107	144	-	146	-	147	-	149
14	83	134	-	136	-	137	-	138	87	134	-	136	-	137	-	138
15	69	120	113	127	-	128	-	129	72	125	118	127	-	128	-	129
16	57	106	95	119	-	120	-	121	60	117	99	119	-	120	-	121
17	48	93	80	112	-	112	-	114	50	101	83	112	-	112	-	114
18	41	82	68	106	98	106	-	107	43	86	71	106	102	106	-	107
19			58	97	84	101	-	102			61	100	88	101	-	102
20			50	88	73	96	-	97			53	95	76	96	-	97
21			44	79	63	91	85	92			46	90	66	91	89	92
22					55	87	74	88			40	80	58	87	78	88
23					49	80	65	84					51	83	69	84
24					43	73	58	80					45	80	61	80
25							52	77					40	76	54	77
26 27							46	72							48	74
28							41	67							43	71
28																
30																
30																

- Total Load values are limited by shear, moment, or deflection equal to L/240.
- Live Load values are limited by deflection equal to L/480.
 For deflection limits of L/360 and L/960, multiply the Live Load values by 1.33 and 0.50 respectively.
- Both the Total Load and Live Load columns must be checked. Where a Live Load value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists.
 Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC CALC* software if the length of any span is less than half the length of an adjacent span.

- Table values do not consider composite action from gluing and nailing floor sheathing (composite action is considered in floor span tables on page 4).
- Total Load values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- For assistance with floor design, consult the section *About Floor Performance* on page 4.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC Calc* software.

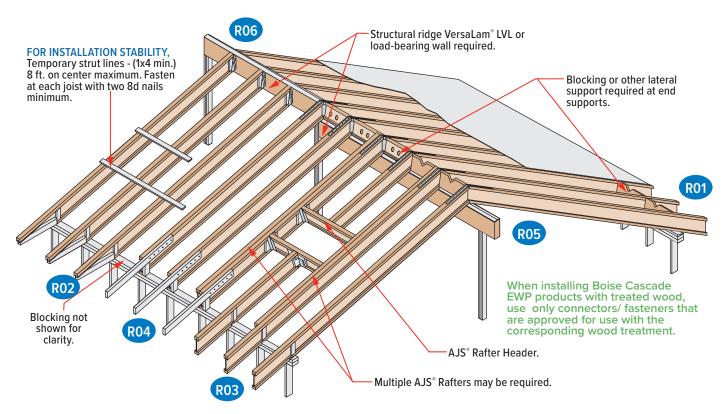
Allowable Uniform Floor Load

(in pounds per linear foot [PLF])

100% Load Duration AJS® 25 Series 3½" Flange Width 91/2" 111/8" 14" 16" AJS® 25 AJS[®] 25 AJS® 25 AJS® 25 Span Length Live Load **Total Load** Live Load Total Load Live Load **Total Load** Live Load **Total Load** _ -



AJS® Rafters



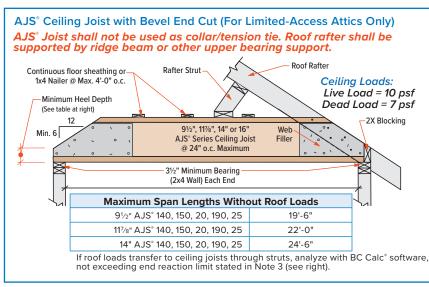
SAFETY WARNING

DO NOT ALLOW WORKERS ON AJS® JOISTS UNTIL ALL HANGERS, AJS® RIM JOISTS, RIM BOARDS, AJS® BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW.

- Build a braced end wall at the end of the bay, or permanently install
 the first eight feet of AJS* Joists and the first course of sheathing. As
 an alternate, temporary sheathing may be nailed to the first four feet of
 AJS* Joists at the end of the bay.
- All hangers, AJS* rim joists, rim boards, AJS* blocking panels, and x-bracing must be completely installed and properly nailed as each AJS* Joist is set.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional AJS* Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS* Joist with two 8d nails.

SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS* Joists to within ½ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.

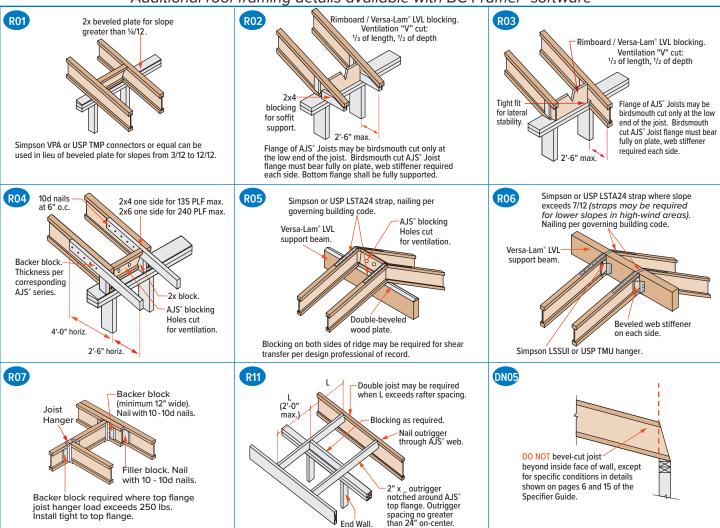


	Joist	Er	nd Wall
Minimo	Depth	2 x 4	2 x 6
Minimum Heel	9½"	2½"	11/2"
Depths	11%"	3½"	2½"
	14"	41/2"	31/2"

Notes:

- Detail is to be used only for ceiling joists with no access to attic space.
- 2) Ceiling joist must be designed to carry all roof load transferred through rafter struts as shown.
- 3) AJS® ceiling joist end reaction may not exceed 550 pounds.
- 4) Minimum roof slope is 6/12.
- 5) Nail roof rafter to AJS® top flange with 1 10d (3" long) box or larger nail.
- 6) 1x4 nailers must be continuous and nailed to a braced end wall.
- 7) Install a 24" long web stiffener on each side of AJS® Joist at beveled ends. Nail roof rafter to AJS® Joist per building code requirements for ceiling joist to roof rafter connection.

Additional roof framing details available with BC Framer® software



LATERAL SUPPORT

 AJS® Joists must be laterally supported at end supports (including supports adjacent to overhangs) with hangers, rimboard, or blocking (Versa-Lam® LVL, Boise Cascade® Rimboard or AJS® Joist). Metal cross bracing or other x-bracing provides adequate lateral support for AJS® Joists, consult governing building code for roof diaphragm connection provisions.

MINIMUM BEARING LENGTH FOR AJS° JOISTS

- Minimum end bearing: 1½" for all AJS° Joists. 3½" is required at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values.
 Refer to the building code evaluation report or the BC Calc* software.

NAILING REQUIREMENTS

- AJS° rim joist, rim board or closure panel to AJS° joist:
- Rims or closure panel 1¼ inches thick and less:
 2-8d nails, one each in the top and bottom flange.
- AJS* 140/150/20/190 rim joist: 2-16d box nails, one each in the top and bottom flange.
- AJS* 25 rim joist: Toe-nail top flange to rim joist with 2-10d box nails, one each side of flange.
- AJS° rim joist, rim board or AJS° blocking panel to support:
 - Min. 8d nails @ 6" o.c. per IRC®.
 - Connection per design professional of record's specification for shear transfer.
- AJS[®] joist to support:
 - 2-8d nails, one on each side of the web, placed 1½ inches minimum from the end of the AJS* Joist to limit splitting.

- · Sheathing to AJS® joist:
 - Prescriptive residential floor sheathing nailing requires 8d common nails @ 6" o.c. on edges and @ 12" o.c. in the field (IRC® Table R602.3(1)).
 - See closest allowable nail spacing limits on page 24 for floor diaphragm nailing specified at closer spacing than IRC®.
 - Maximum nail spacing for minimum lateral stability: 18" for AJS* 140/150/20/190, 24" for larger AJS* joist series.
 - 14 gauge staples may be substituted for 8d nails if the staples penetrate at least 1 inch into the joist.
 - Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.

BACKER AND FILLER BLOCK DIMENSIONS

Series	Backer Block Thickness	Filler Block Thickness
140 150 20 190	11/8" or two ½" wood panels	2x _ + 5½" wood panel
25	2 x _ lumber	Double 2 x _ lumber

- Cut backer and filler blocks to a maximum depth equal to the web depth minus ¼" to avoid a forced fit.
- For deeper AJS® Joists, stack 2x lumber or use multiple pieces of ¾" wood panels.

WEB STIFFENER REQUIREMENTS

• See Web Stiffener Requirements on page 9.

PROTECT AJS° JOISTS FROM THE WEATHER

 AJS[®] Joists are intended only for applications that provide permanent protection from the weather.
 Bundles of AJS[®] Joists should be covered and stored off of the ground on stickers.

MAXIMUM SLOPE

 Unless otherwise noted, all roof details are valid for slopes of 12 in 12 or less.

VENTILATION

 The 1½ inch, pre-stamped knock-out holes spaced at 12 inches on center along the AJS* Joist may all be knocked out and used for cross ventilation.
 Deeper joists that what is structurally needed may be advantageous in ventilation design. Consult local building official and/or ventilation specialist for specific ventilation requirements.

BIRDSMOUTH CUTS

 AJS® Joists may be birdsmouth cut only at the low end support. AJS® Joists with birdsmouth cuts may cantilever up to 2'-6" past the low end support. The bottom flange must sit fully on the support and may not overhang the inside face of the support. High end supports and intermediate supports may not be birdsmouth cut.

				1	15%	and '	125 %	Loa	d Du	ratio	n				
										0 Series ge Widt					
					9½" AJS [®] 140			11%" AJS [®] 140			14" AJS [®] 140			16" AJS [®] 140	
		Live Load [psf]	Dead Load [psf]	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12
	Non- Snow	20	10	25'–2" 23'–10"	23'–9" 22'–5"	22'-0" 20'-8"	30'–1" 28'–6"	28'–5" 26'–9"	26'-4" 24'-9"	34'–4" 32'–5"	32'-4" 30'-6"	30'-0" 28'-2"	37'–10" 34'–11"	35'–11" 33'–10"	33'–4" 31'–3"
	125%	20	20	22'-9"	21'-4"	19'-7"	27'-3"	25'-6"	23'-5"	30'-3"	29'-0"	26'-8"	32'-7"	31'-6"	29'–7"
		25	10	23'–11"	22'–7"	21'-0"	28'-5"	27'-0"	25'–2"	31'–2"	30'-7"	28'-8"	33'-7"	32'–11"	31'–10"
		25	15	22'–10"	21'–6"	19'–10"	26'-6"	25'-8"	23'-9"	29'–1"	28'-4"	27'–1"	31'–4"	30'-6"	29'–5"
12" o.c.		30	10	22'–10"	21'–8"	20'-2"	26'-7"	25'–11"	24'–2"	29'–2"	28'-8"	27'-6"	31'–6"	30'–11"	30'-2"
	Snow	30	15	21'–11"	20'–8"	19'–2"	25'-0"	24'–5"	22'–11"	27'–5"	26'–10"	25'–11"	29'–7"	28'–11"	28'-0"
	115%	40	10	20'–10"	19'–11"	18'–10"	23'-9"	23'-5"	22'-6"	26'–2"	25'-9"	25'-3"	28'-2"	27'-9"	27'–2"
		40	15	19'–10"	19'–4"	18'–1"	22'-7"	22'-2"	21'-7"	24'–10"	24'-4"	23'-9"	26'-9"	26'-3"	25'-7"
		50	10	19'-0"	18'-6"	17'-6"	21'-8"	21'-5"	21'-0"	23'–10"	23'-7"	23'-2"	25'-8"	25'-5"	25'-0"
		50	15	18'-3"	17'–11"	17'-2"	20'-10"	20'-6"	20'-0"	22'–10"	22'-6"	22'-0"	24'-8"	24'-3"	23'-8"
	Non-	20 20	10 15	22'–10" 21'–7"	21'–6" 20'–3"	20'-0" 18'-9"	27'–4" 25'–6"	25'-9" 24'-3"	23'–11" 22'–5"	30'–5" 28'–1"	29'–4" 27'–2"	27'–3" 25'–6"	32'-9" 30'-3"	32'-0" 29'-4"	30'–3" 28'–2"
	Snow 125%	20	20		19'-3"			23'-0"			25'-3"				
		25	10	20'–7" 21'–7"	20'-6"	17'–9" 19'–1"	23'–10" 24'–7"	24'–1"	21'–3" 22'–10"	26'–2" 27'–0"	26'-5"	24'–1" 25'–8"	28'–2" 29'–1"	27'–3" 28'–6"	26'-0" 27'-8"
		25	15	20'-1"	19'-5"	18'-0"	22'-11"	24-1	21'-6"	25'-2"	24'-6"	23'-8"	27'-1"	26'-5"	27 –8 25'–6"
16" o.c.		30	10	20'-2"	19'-7"	18'-3"	23'-0"	22'-7"	21'–11"	25'–3"	24'-10"	24'-2"	27'-3"	26'-9"	26'-1"
10 0.0.	Snow	30	15	18'–11"	18'-6"	17'-5"	21'-7"	21'–1"	20'-5"	23'-9"	23'-2"	22'-5"	25'-7"	25'-0"	24'-2"
	115%	40	10	18'-0"	17'-9"	17'-1"	20'-7"	20'-3"	19'-10"	22'-7"	22'-3"	21'–10"	24'-4"	24'-0"	23'-6"
		40	15	17'-2"	16'-10"	16'-4"	19'-7"	19'-2"	18'-8"	21'–6"	21'-1"	20'-6"	23'-2"	22'-8"	22'-1"
		50	10	16'–5"	16'-3"	15'-10"	18'-9"	18'-6"	18'-3"	20'-7"	20'-4"	20'-0"	22'-3"	21'–11"	21'-7"
		50	15	15'–9"	15'-6"	15'-2"	18'-0"	17'-8"	17'-3"	19'–9"	19'-5"	19'-0"	21'-4"	20'-11"	20'-6"
	Non-	20	10	21'–5"	20'-3"	18'-9"	25'-3"	24'-2"	22'-5"	27'-9"	27'–1"	25'-7"	29'–10"	29'-2"	28'-3"
	Snow	20	15	20'-3"	19'–1"	17'-7"	23'-3"	22'-7"	21'–1"	25'-7"	24'-10"	23'-10"	27'-7"	26'-9"	25'-8"
	125%	20	20	19'–1"	18'–1"	16'-8"	21'-9"	21'-0"	19'–11"	23'–10"	23'-0"	22'-0"	25'-9"	24'–10"	23'-8"
		25	10	19'–8"	19'-3"	17'–11"	22'-5"	21'–11"	21'-4"	24'-7"	24'–1"	23'-5"	26'-6"	26'-0"	25'-3"
		25	15	18'–4"	17'–10"	16'–11"	20'–11"	20'-4"	19'–7"	22'–11"	22'-4"	21'-7"	24'-9"	24'–1"	23'–3"
19.2" o.c.		30	10	18'–5"	18'–1"	17'–2"	21'-0"	20'-7"	20'–1"	23'-0"	22'-7"	22'–1"	24'–10"	24'–5"	23'-9"
	Snow	30	15	17'–3"	16'–10"	16'-4"	19'–8"	19'–3"	18'-7"	21'–8"	21'-2"	20'–6"	23'–4"	22'–9"	22'–1"
	115%	40	10	16'–5"	16'-2"	15'–10"	18'-9"	18'–6"	18'–1"	20'-7"	20'-4"	19'–11"	22'-2"	21'–11"	21'–5"
		40	15	15'-8"	15'-4"	14'-11"	17'-10"	17'-6"	17'-0"	19'-7"	19'-2"	18'-8"	21'–1"	20'-8"	20'-2"
		50	10	15'-0"	14'-10"	14'-7"	17'-1"	16'-11"	16'-7"	18'-10"	18'-7"	18'-3"	20'-3"	20'-0"	19'-8"
		50	15	14'-4"	14'-2"	13'–10"	16'-5"	16'–1"	15'-9"	18'-0"	17'-9"	17'-4"	19'-5"	19'–1"	18'-8"
	Non-	20	10 15	19'-9"	18'-9"	17'-5"	22'-6"	22'-0"	20'-10"	24'-9"	24'-2"	23'-5"	26'-8"	26'-1"	25'-3"
	Snow 125%	20 20	20	18'–3" 17'–0"	17'–8" 16'–5"	16'–3" 15'–5"	20'–10" 19'–5"	20'–2" 18'–9"	19'–4" 17'–10"	22'–10" 21'–4"	22'–2" 20'–7"	21'–3" 19'–8"	24'-7" 23'-0"	23'–11" 22'–2"	22'–11" 21'–2"
		25	10	17'-7"	17'-2"	16'-7"	20'-0"	19'-7"	19'-1"	22'-0"	21'-6"	20'–11"	23'-8"	23'-3"	21-2
		25	15	16'-4"	15'–11"	15'-4"	18'-8"	18'-2"	17'-6"	20'-6"	20'-0"	19'-3"	22'–1"	21'-6"	20'-9"
24" o.c.		30	10	16'-5"	16'-1"	15'-9"	18'-9"	18'-5"	17'-11"	20'-7"	20'-2"	19'-8"	22'-2"	21'-9"	21'-3"
0.0.	Snow	30	15	15'-5"	15'-1"	14'-7"	17'-7"	17'-2"	16'-8"	19'-4"	18'-11"	18'-3"	20'–10"	20'-4"	19'-8"
	115%	40	10	14'-8"	14'-6"	14'-2"	16'-9"	16'-6"	16'-2"	18'-5"	18'-2"	17'-9"	19'–10"	19'-7"	19'-2"
		40	15	13'–11"	13'-8"	13'-4"	15'–11"	15'-7"	15'-2"	17'-6"	17'-2"	16'-8"	18'–10"	18'-6"	18'-0"
		50	10	13'–5"	13'-3"	13'-0"	15'-3"	15'–1"	14'-10"	16'-9"	16'-7"	16'-4"	18'-1"	17'–11"	17'-5"
		50	15	12'-10"	12'-7"	12'-4"	14'-8"	14'-5"	14'-1"	16'–1"	15'–10"	15'-5"	17'-0"	16'-5"	15'-8"

- Table values are limited by shear, moment, total load deflection equal to L/180 and live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Table values represent the most restrictive of simple or multiple span applications. Analyze multiple span joists with the BC Calc* software if the length of any span is less than half the length of an adjacent span.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- This table was designed to apply to a broad range of applications. It may
 be possible to exceed the limitations of this table by analyzing a specific
 application with the BC Calc* software.
- Slope roof joists at least 1/4" over 12" to minimize ponding.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

	115% and 125% Load Duration														
										0 Series ge Widt					
					9½" AJS [®] 150			11%" AJS° 150			14" AJS [®] 150			16" AJS [®] 150	
		Live Load [psf]	Dead Load [psf]	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12
	Non-	20	10	25'-9"	24'-3"	22'-6"	30'-9"	29'-0"	26'–11"	35'–1"	33'–1"	30'-8"	38'–11"	36'-8"	34'-0"
	Snow 125%	20 20	15 20	24'–4" 23'–3"	22'–10" 21'–9"	21'–1" 20'–0"	29'–2" 27'–10"	27'–5" 26'–0"	25'–3" 23'–11"	33'–2" 31'–8"	31'–2" 29'–8"	28'-9" 27'-3"	36'–10" 34'–11"	34'–7" 32'–11"	31'–11" 30'–3"
		25	10	24'-5"	23'–1"	21'-6"	29'-3"	27'-8"	25'-9"	33'-3"	31'-6"	29'-3"	36'-0"	34'–11"	32'-6"
		25	15	23'-3"	21'–11"	20'-4"	27'–11"	26'-3"	24'-4"	31'–2"	29'–11"	27'-8"	33'-7"	32'-9"	30'-9"
12" o.c.		30	10	23'-4"	22'-1"	20'-7"	27'–11"	26'-6"	24'-8"	31'-4"	30'-2"	28'-1"	33'-9"	33'–1"	31'–2"
	Snow	30	15	22'–5"	21'–1"	19'-7"	26'–10"	25'-3"	23'-6"	29'-5"	28'-9"	26'-9"	31'–9"	31'-0"	29'–8"
	115%	40	10	21'–3"	20'-4"	19'-3"	25'-5"	24'-5"	23'-0"	28'-0"	27'-7"	26'-3"	30'-2"	29'-9"	29'–1"
		40	15	20'–11"	19'-9"	18'-5"	24'-3"	23'-8"	22'-1"	26'-8"	26'–1"	25'-2"	28'-8"	28'-2"	27'–5"
		50	10	19'–8"	18'–10"	17'–11"	23'-3"	22'-7"	21'–5"	25'-7"	25'-3"	24'-5"	27'–7"	27'–3"	26'-9"
		50	15	19'–7"	18'-9"	17'-6"	22'–4"	21'–11"	21'-0"	24'–6"	24'–1"	23'–7"	26'–5"	26'-0"	25'–5"
	Non-	20	10	23'–4"	22'-0"	20'-5"	27'–11"	26'-4"	24'–5"	31'–9"	30'-0"	27'–10"	35'–1"	33'–3"	30'–10"
	Snow	20	15	22'–1"	20'-9"	19'–2"	26'–5"	24'–10"	22'–11"	30'–1"	28'-3"	26'–1"	32'–5"	31'–4"	28'–11"
	125%	20	20	21'–1"	19'–8"	18'–1"	25'–2"	23'–7"	21'–8"	28'–1"	26'–11"	24'–9"	30'–3"	29'–2"	27'–5"
		25	10	22'–1"	20'–11"	19'–6"	26'–4"	25'–1"	23'–4"	28'–11"	28'–4"	26'-7"	31'–2"	30'–6"	29'–6"
		25	15	21'–1"	19'–10"	18'-5"	24'-7"	23'-9"	22'-0"	27'-0"	26'-3"	25'–1"	29'–1"	28'-4"	27'-4"
16" o.c.		30	10	21'-2"	20'-0"	18'-8"	24'-8"	24'-0"	22'-4"	27'–1"	26'-7"	25'-6"	29'–2"	28'-8"	27'–11"
	Snow	30	15	20'-3"	19'–1"	17'-9"	23'-2"	22'-8"	21'–3"	25'-5"	24'–10"	24'-1"	27'–5"	26'-9"	25'–11"
	115%	40	10	19'-3"	18'-5"	17'-5"	22'-1"	21'-9"	20'-10"	24'-3"	23'–10"	23'-5"	26'-1"	25'-9"	25'-3"
		40	15	18'-5"	17'–11"	16'-8"	21'-0"	20'-7"	20'-0"	23'-0"	22'-7"	22'-0"	24'–10"	24'-4"	23'-8"
		50 50	10	17'–8" 16'–11"	17'–1" 16'–8"	16'-3"	20'–2" 19'–4"	19'–11" 19'–0"	19'-5"	22'–1" 21'–2"	21'–10" 20'–10"	21'–6" 20'–4"	23'–10"	23'–6" 22'–6"	23'-2"
		20	15 10	21'–11"	20'-8"	15'–10" 19'–2"	26'-3"	24'-9"	18'–7" 22'–11"	29'-9"	28'-2"	26'-2"	22'–10" 32'–0"	31'-3"	21'–11" 29'–0"
	Non- Snow	20	15	20'-9"	19'-6"	18'-0"	26 –3	24 –9	21'-6"	29 –9 27'–5"	26'-7"	26 –2	29'-7"	28'-8"	29 –0
	125%	20	20	19'-9"	18'-6"	17'-0"	23'-4"	23 –4	20'-5"	27 – 5 25'–7"	24'-8"	23'-3"	29 –7	26'-7"	27 –2 25'–5"
		25	10	20'-9"	19'-8"	18'-3"	24'-0"	23'-6"	21'–11"	26'-5"	25'–10"	24'–11"	28'-5"	27'–10"	27'–1"
		25	15	19'-8"	18'-8"	17'-3"	22'-5"	21'–10"	20'-8"	24'-7"	24'-0"	23'–1"	26'-6"	25'–10"	24'–11"
19.2" o.c.		30	10	19'-9"	18'-10"	17'-7"	22'-6"	22'-1"	21'-0"	24'-8"	24'-3"	23'-8"	26'-7"	26'-2"	25'-6"
	Snow	30	15	18'-7"	17'-11"	16'-8"	21'–2"	20'-8"	20'-0"	23'-3"	22'-8"	21'–11"	25'-0"	24'-5"	23'-8"
	115%	40	10	17'–8"	17'-4"	16'-4"	20'–1"	19'–10"	19'-5"	22'–1"	21'-9"	21'-4"	23'–10"	23'-6"	23'-0"
		40	15	16'-9"	16'-5"	15'-8"	19'-2"	18'-9"	18'-3"	21'-0"	20'-7"	20'-1"	22'-8"	22'-2"	21'–7"
		50	10	16'–1"	15'-11"	15'-3"	18'-4"	18'-2"	17'–10"	20'-2"	19'–11"	19'-7"	21'–9"	21'-6"	21'–1"
		50	15	15'-5"	15'-2"	14'–10"	17'-7"	17'-4"	16'–11"	19'–4"	19'-0"	18'-7"	20'–10"	20'-6"	19'–8"
	Non-	20	10	20'-3"	19'–1"	17'-9"	24'-2"	22'–11"	21'-3"	26'-7"	25'–11"	24'-3"	28'-7"	27'–11"	26'–11"
	Snow	20	15	19'–2"	18'-0"	16'-8"	22'-4"	21'–7"	19'–11"	24'–6"	23'-9"	22'-8"	26'–5"	25'–7"	24'–7"
	125%	20	20	18'–3"	17'–2"	15'-9"	20'–10"	20'–1"	18'–10"	22'–10"	22'–1"	21'–1"	24'–8"	23'–9"	22'–8"
		25	10	18'–10"	18'-2"	16'–11"	21'–6"	21'-0"	20'-3"	23'–7"	23'–1"	22'-5"	25'–5"	24'–11"	24'–2"
		25	15	17'–7"	17'–1"	16'-0"	20'-0"	19'–6"	18'–10"	22'-0"	21'–5"	20'-8"	23'–8"	23'–1"	22'–3"
24" o.c.		30	10	17'–8"	17'-4"	16'-3"	20'–1"	19'–9"	19'–3"	22'–1"	21'–8"	21'–2"	23'–9"	23'–4"	22'–9"
	Snow	30	15	16'-7"	16'-2"	15'-5"	18'–11"	18'-5"	17'–10"	20'-9"	20'-3"	19'-7"	22'-4"	21'–10"	21'–1"
	115%	40	10	15'-9"	15'-6"	15'-2"	18'-0"	17'-8"	17'-4"	19'-9"	19'-5"	19'–1"	21'-3"	20'-11"	20'-6"
		40	15	15'-0"	14'-8"	14'-4"	17'-1"	16'-9"	16'-4"	18'-9"	18'-5"	17'-11"	20'-1"	19'-3"	18'-3"
		50	10	14'-5"	14'-2"	14'-0"	16'-5"	16'-2"	15'–11"	18'-0"	17'-9"	17'-2"	18'-6"	18'-0"	17'-5"
		50	15	13'–9"	13'–7"	13'-3"	15'–9"	15'–5"	15'–1"	16'–9"	16'-2"	15'-5"	17'-0"	16'–5"	15'–8"

- Table values are limited by shear, moment, total load deflection equal to L/180 and live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Table values represent the most restrictive of simple or multiple span applications. Analyze multiple span joists with the BC Calc software if the length of any span is less than half the length of an adjacent span.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- This table was designed to apply to a broad range of applications. It may
 be possible to exceed the limitations of this table by analyzing a specific
 application with the BC Calc* software.
- Slope roof joists at least 1/4" over 12" to minimize ponding.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

	115% and 125% Load Duration														
									AJS® 20 ½" Flan) Series ge Widt	h				
					9½" AJS° 20			11%" AJS [®] 20		-	14" AJS [®] 20			16" AJS [®] 20	
		Live Load [psf]	Dead Load [psf]	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12
	Non-	20	10	27'–4"	25'-9"	23'–11"	32'-7"	30'-9"	28'-7"	37'–1"	35'-0"	32'–6"	41'-2"	38'–10"	36'-0"
	Snow	20	15	25'–10"	24'-3"	22'-5"	30'–11"	29'-0"	26'-9"	35'-2"	33'-0"	30'-5"	39'-0"	36'-7"	33'-9"
	125%	20	20	24'–8"	23'–1"	21'–3"	29'–6"	27'-7"	25'–4"	33'–6"	31'–5"	28'–10"	37'–2"	34'–10"	32'-0"
		25	10	25'–11"	24'-6"	22'–10"	31'-0"	29'–3"	27'-3"	35'–3"	33'-4"	31'-0"	39'–1"	36'–11"	34'–4"
		25	15	24'–9"	23'-3"	21'–6"	29'–7"	27'–10"	25'-9"	33'–7"	31'–8"	29'–4"	36'–11"	35'–1"	32'–6"
12" o.c.		30	10	24'–9"	23'-6"	21'–11"	29'–7"	28'–0"	26'–2"	33'–8"	31'–11"	29'–9"	37'–1"	35'–4"	33'-0"
	Snow	30	15	23'–9"	22'-5"	20'–10"	28'–5"	26'–9"	24'–10"	32'–4"	30'–6"	28'–3"	34'–10"	33'–10"	31'–4"
	115%	40	10	22'–6"	21'-7"	20'–5"	26'–11"	25'–10"	24'-5"	30'–8"	29'–5"	27'–9"	33'–2"	32'–7"	30'–9"
		40	15	22'–2"	21'-0"	19'–7"	26'–6"	25'–1"	23'-5"	29'–3"	28'–7"	26'-7"	31'–7"	30'–11"	29'–6"
		50	10	20'–10"	20'-0"	19'–0"	24'–11"	23'–11"	22'-9"	28'–1"	27'–3"	25'–10"	30'–3"	29'–11"	28'–8"
		50	15	20'–10"	19'–11"	18'–7"	24'–6"	23'–9"	22'–3"	26'–11"	26'–6"	25'–3"	29'-0"	28'–7"	27'–11"
	Non-	20	10	24'–9"	23'–4"	21'–8"	29'–7"	27'–11"	25'–11"	33'–8"	31'–9"	29'–5"	37'–4"	35'–2"	32'–8"
	Snow	20	15	23'–5"	22'-0"	20'–4"	28'-0"	26'–4"	24'-3"	31'–10"	29'–11"	27'–7"	35'–4"	33'–2"	30'–7"
	125%	20	20	22'–4"	20'–11"	19'–3"	26'–8"	25'-0"	23'-0"	30'–5"	28'–5"	26'–2"	33'–3"	31'–7"	29'–0"
		25	10	23'–6"	22'–2"	20'–8"	28'–1"	26'–6"	24'–8"	31'–10"	30'–2"	28'–1"	34'–3"	33'–6"	31'–2"
		25	15	22'–5"	21'–1"	19'–6"	26'–9"	25'–2"	23'–4"	29'–8"	28'–8"	26'–7"	32'-0"	31'–1"	29'–5"
16" o.c.		30	10	22'–5"	21'–3"	19'–10"	26'–10"	25'–5"	23'–9"	29'–9"	28'–11"	27'-0"	32'–1"	31'–6"	29'–11"
	Snow	30	15	21'–6"	20'–4"	18'–10"	25'–6"	24'–3"	22'–6"	28'–0"	27'–4"	25'–8"	30'–2"	29'–5"	28'–5"
	115%	40	10	20'–5"	19'–7"	18'–6"	24'–3"	23'–5"	22'–1"	26'–8"	26'–3"	25'–2"	28'–8"	28'–3"	27'–9"
		40	15	20'–1"	19'-0"	17'–9"	23'–1"	22'–7"	21'–2"	25'–4"	24'–10"	24'–1"	27'–3"	26'–9"	26'–1"
		50	10	18'–11"	18'–1"	17'–2"	22'–2"	21'–8"	20'-7"	24'–4"	24'-0"	23'–5"	26'–2"	25'–11"	25'–5"
		50	15	18'–7"	18'-0"	16'–10"	21'–3"	20'–10"	20'–1"	23'–4"	22'–11"	22'–5"	25'–1"	24'-8"	23'–7"
	Non-	20	10	23'–3"	21'–11"	20'–4"	27'–9"	26'–2"	24'–4"	31'–7"	29'–10"	27'–8"	35'–1"	33'–1"	30'–8"
	Snow	20	15	22'-0"	20'–8"	19'–1"	26'–4"	24'–8"	22'–10"	29'–11"	28'–1"	25'–11"	32'–6"	31'–2"	28'–9"
	125%	20	20	21'–0"	19'–8"	18'–1"	25'–1"	23'–6"	21'–7"	28'–1"	26'–9"	24'–7"	30'–4"	29'–3"	27'–3"
		25	10	22'–1"	20'–10"	19'–5"	26'–4"	24'–11"	23'–3"	29'–0"	28'–4"	26'–5"	31'–3"	30'–7"	29'–3"
		25	15	21'–0"	19'–10"	18'–4"	24'–8"	23'–8"	21'–11"	27'–1"	26'–4"	24'–11"	29'–2"	28'–5"	27'–5"
19.2" o.c.		30	10	21'–1"	19'–11"	18'–8"	24'-9"	23'–10"	22'-3"	27'–2"	26'-8"	25'-4"	29'–3"	28'-9"	28'-0"
	Snow	30	15	20'–2"	19'–1"	17'–8"	23'–3"	22'–8"	21'–2"	25'–6"	24'–11"	24'–1"	27'–6"	26'–10"	26'-0"
	115%	40	10	19'–2"	18'-5"	17'-4"	22'–1"	21'–10"	20'-9"	24'–3"	23'–11"	23'–5"	26'-2"	25'–9"	25'–3"
		40	15	18'-5"	17'-10"	16'–8"	21'-0"	20'-7"	19'–11"	23'–1"	22'-8"	22'-0"	24'–11"	24'-2"	22'–11"
		50	10	17'–8"	17'-0"	16'-2"	20'-2"	19'–11"	19'-4"	22'-2"	21'–11"	21'–6"	23'-2"	22'-7"	21'–10"
		50	15	17'-0"	16'-8"	15'-9"	19'-4"	19'-0"	18'-7"	21'-0"	20'-3"	19'-4"	21'–3"	20'-7"	19'-8"
	Non-	20	10	21'–6"	20'-4"	18'–10"	25'-9"	24'–3"	22'-6"	29'–2"	27'-7"	25'-8"	31'–5"	30'-8"	28'-5"
	Snow 125%	20	15	20'-4"	19'–1"	17'-8"	24'-4"	22'–10"	21'–1"	26'–11"	26'-0"	24'-0"	29'-0"	28'-2"	26'-8"
	12370	20	20	19'–5"	18'-2"	16'-9"	22'–11"	21'-9"	20'-0"	25'-2"	24'-3"	22'-9"	27'-1"	26'-2"	24'–11"
		25	10	20'-5"	19'-4"	18'-0"	23'-7"	23'–1"	21'–6"	25'–11"	25'-5"	24'-6"	27'–11"	27'-4"	26'-7"
24"		25	15	19'-4"	18'-4"	17'-0"	22'-0"	21'-5"	20'-4"	24'-2"	23'-6"	22'-8"	26'-1"	25'-4"	24'-3"
24" o.c.		30	10	19'–4"	18'-6"	17'-3"	22'-1"	21'–8"	20'-8"	24'-3"	23'–10"	23'-3"	26'-2"	25'-8"	25'-0"
	Snow	30	15	18'-2"	17'-8"	16'-5"	20'-9"	20'-3"	19'-7"	22'-10"	22'-3"	21'-7"	24'-6"	23'-4"	21'–11"
	115%	40	10	17'-4"	17'-0"	16'-1"	19'-9"	19'–6"	19'–1"	21'-8"	21'–3"	20'-5"	22'-2"	21'–6"	20'-8"
		40	15	16'-6"	16'-2"	15'-5"	18'-9"	18'-5"	17'-8"	19'-9"	19'-0"	18'-0"	20'-1"	19'-3"	18'-3"
		50	10	15'–10"	15'-7"	14'-11"	17'–11"	17'-6"	16'-11"	18'-2"	17'-9"	17'-2"	18'-6"	18'-0"	17'-5"
		50	15	15'–2"	14'–11"	14'-7"	16'–5"	15'–11"	15'–2"	16'–9"	16'-2"	15'–5"	17'-0"	16'–5"	15'–8"

- Table values are limited by shear, moment, total load deflection equal to L/180 and live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Table values represent the most restrictive of simple or multiple span applications. Analyze multiple span joists with the BC Calc software if the length of any span is less than half the length of an adjacent span.
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- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

	115% and 125% Load Duration														
										0 Series ge Widt					
					9½" AJS [®] 190			11%" AJS [®] 190		<u> </u>	14" AJS [®] 190			16" AJS [®] 190	
		Live Load [psf]	Dead Load [psf]	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12
	Non-	20	10	27'–9"	26'–2"	24'-3"	33'–2"	31'–3"	29'-0"	37'–9"	35'–7"	33'-0"	41'–10"	39'–6"	36'–7"
	Snow	20	15	26'–3"	24'-8"	22'-9"	31'–5"	29'-6"	27'-3"	35'-9"	33'-7"	31'-0"	39'–8"	37'-3"	34'–4"
	125%	20	20	25'–1"	23'-6"	21'-7"	30'-0"	28'–1"	25'-9"	34'–1"	31'–11"	29'-4"	37'–10"	35'-5"	32'-7"
		25	10	26'-4"	24'–11"	23'-2"	31'–6"	29'–9"	27'-8"	35'–10"	33'–10"	31'–6"	39'–9"	37'–7"	34'–11"
		25	15	25'–2"	23'-8"	21'–11"	30'-0"	28'-3"	26'-2"	34'–2"	32'-2"	29'-9"	37'–11"	35'-8"	33'-0"
12" o.c.		30	10	25'–2"	23'–10"	22'–3"	30'–1"	28'–6"	26'–7"	34'–3"	32'–5"	30'–3"	38'–0"	36'-0"	33'–7"
	Snow	30	15	24'–2"	22'-9"	21'–2"	28'–10"	27'–3"	25'–3"	32'–10"	31'–0"	28'–9"	36'–5"	34'–4"	31'–11"
	115%	40	10	22'–11"	22'-0"	20'-9"	27'–5"	26'–3"	24'–10"	31'–2"	29'–11"	28'-3"	34'–7"	33'–2"	31'–4"
		40	15	22'–7"	21'–4"	19'–11"	27'-0"	25'–6"	23'–9"	30'–8"	29'–1"	27'–1"	33'–10"	32'–3"	30'-0"
		50	10	21'–3"	20'-4"	19'–4"	25'–4"	24'-4"	23'–1"	28'–11"	27'–8"	26'–3"	32'-0"	30'-9"	29'–2"
		50	15	21'–3"	20'-3"	18'–11"	25'–4"	24'–2"	22'–7"	28'–10"	27'–6"	25'–8"	31'–1"	30'–6"	28'–6"
	Non-	20	10	25'–2"	23'–9"	22'-0"	30'–1"	28'–4"	26'–4"	34'–2"	32'-3"	29'–11"	37'–11"	35'-9"	33'–2"
	Snow	20	15	23'–10"	22'–4"	20'–8"	28'–6"	26'–9"	24'–8"	32'–5"	30'-5"	28'–1"	35'–11"	33'–9"	31'–2"
	125%	20	20	22'–9"	21'–3"	19'–7"	27'–2"	25'–5"	23'–4"	30'–11"	28'–11"	26'–7"	34'–3"	32'–1"	29'–6"
		25	10	23'–10"	22'–7"	21'-0"	28'–6"	27'-0"	25'–1"	32'–6"	30'–8"	28'–7"	36'–0"	34'–1"	31'–8"
		25	15	22'–9"	21'–5"	19'–10"	27'–2"	25'–7"	23'–9"	31'–0"	29'–2"	27'-0"	34'–3"	32'–4"	29'–11"
16" o.c.		30	10	22'–10"	21'–7"	20'–2"	27'–3"	25'–10"	24'–1"	31'–0"	29'–5"	27'–5"	34'–4"	32'–7"	30'–5"
	Snow	30	15	21'–10"	20'-8"	19'–2"	26'–2"	24'–8"	22'–11"	29'–9"	28'–1"	26'–1"	32'–4"	31'–2"	28'–11"
	115%	40	10	20'–9"	19'–11"	18'–10"	24'–10"	23'–10"	22'–6"	28'–3"	27'–1"	25'–7"	30'–9"	30'–1"	28'–5"
		40	15	20'–5"	19'-4"	18'-0"	24'-5"	23'–1"	21'-7"	27'–2"	26'-4"	24'-6"	29'–3"	28'-8"	27'–2"
		50	10	19'–2"	18'-5"	17'-6"	22'–11"	22'-0"	20'-11"	26'–1"	25'–1"	23'–10"	27'–10"	27'-2"	26'–3"
		50	15	19'-2"	18'-4"	17'-1"	22'-9"	21'–11"	20'-5"	25'-0"	24'-5"	23'-3"	25'-7"	24'-9"	23'-7"
	Non-	20	10	23'-7"	22'-3"	20'-8"	28'-3"	26'-8"	24'-9"	32'-2"	30'-4"	28'-2"	35'-8"	33'-7"	31'–3"
	Snow 125%	20	15	22'-4"	21'-0"	19'-5"	26'-9"	25'–1"	23'-2"	30'-5"	28'-7"	26'-5"	33'-9"	31'–8"	29'–3"
	12070	20 25	20 10	21'–4"	20'–0" 21'–2"	18'-4"	25'-6"	23'–10" 25'–4"	21'–11"	29'–0" 30'–6"	27'-2"	25'-0"	32'-2"	30'-2"	27'-9"
		25 25	15	22'–5" 21'–4"	20'-2"	19'–9" 18'–8"	26'–10" 25'–7"	24'-1"	23'–7" 22'–3"	29'-0"	28'–10" 27'–5"	26'–10" 25'–4"	33'–6" 31'–3"	32'–0" 30'–5"	29'–9" 28'–2"
19.2" o.c.		30	10	21'–4	20'-4"	18'–11"	25'-7"	24'-3"	22'-8"	29'-1"	27'-5"	25'–9"	31-3	30'-8"	28'–7"
19.2 0.0.	C	30	15	20'-6"	19'-5"	18'-0"	24'-7"	23'-2"	21'–6"	27'-4"	26'-4"	24'-6"	29'-6"	28'-9"	27'–2"
	Snow 115%	40	10	19'-6"	18'-8"	17'-8"	23'-3"	22'-4"	21'–1"	26'-0"	25'-5"	24'-0"	27'-9"	27'-0"	25'–11"
		40	15	19'-2"	18'-2"	16'–11"	22'-6"	21'–8"	20'-3"	24'-9"	23'–10"	22'-7"	25'-2"	24'-2"	22'–11"
		50	10	18'-0"	17'-3"	16'-5"	21'-6"	20'-8"	19'-8"	22'–10"	22'-3"	21'-6"	23'-2"	22'-7"	21'–10"
		50	15	18'-0"	17'-2"	16'–1"	20'-7"	19'–11"	19'-0"	21'-0"	20'-3"	19'-4"	21'–3"	20'-7"	19'-8"
	Nimo	20	10	21'–10"	20'-8"	19'-2"	26'-2"	24'-8"	22'–11"	29'-9"	28'-1"	26'–1"	33'-0"	31'–2"	28'–11"
	Non- Snow	20	15	20'-8"	19'-5"	18'-0"	24'-9"	23'-3"	21'–6"	28'-2"	26'-6"	24'-5"	31'–1"	29'–4"	27'–1"
	125%	20	20	19'-9"	18'-6"	17'-0"	23'-7"	22'–1"	20'-4"	26'-10"	25'-2"	23'–2"	29'-0"	27'-9"	25'–3"
		25	10	20'-9"	19'-8"	18'-3"	24'–10"	23'-6"	21'–10"	27'-9"	26'-9"	24'–11"	29'–11"	29'-4"	27'-7"
		25	15	19'-9"	18'-8"	17'-3"	23'-7"	22'-3"	20'-8"	25'–11"	25'-3"	23'-6"	27'–6"	26'-1"	24'-3"
24" o.c.		30	10	19'–10"	18'-9"	17'-6"	23'-8"	22'-6"	21'-0"	26'-0"	25'-6"	23'–10"	27'-9"	26'-9"	25'-5"
	Snow	30	15	19'-0"	17'-11"	16'-8"	22'-3"	21'-5"	19'–11"	24'-2"	23'-0"	21'-7"	24'-6"	23'-4"	21'–11"
	115%	40	10	18'-0"	17'-3"	16'-4"	21'–2"	20'-8"	19'-7"	21'–10"	21'-3"	20'-5"	22'-2"	21'–6"	20'-8"
		40	15	17'–8"	16'-9"	15'-8"	19'–5"	18'-8"	17'-8"	19'–9"	19'-0"	18'-0"	20'–1"	19'-3"	18'-3"
		50	10	16'–8"	16'-0"	15'-2"	17'–11"	17'-6"	16'–11"	18'-2"	17'-9"	17'-2"	18'–6"	18'-0"	17'–5"
		50	15	16'–2"	15'–8"	14'–10"	16'–5"	15'–11"	15'–2"	16'-9"	16'-2"	15'–5"	17'-0"	16'-5"	15'–8"

- Table values are limited by shear, moment, total load deflection equal to L/180 and live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Table values represent the most restrictive of simple or multiple span applications. Analyze multiple span joists with the BC Calc software if the length of any span is less than half the length of an adjacent span.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less. 18" joists require web stiffeners at all bearing locations.
- This table was designed to apply to a broad range of applications. It may
 be possible to exceed the limitations of this table by analyzing a specific
 application with the BC CALC* software.
- Slope roof joists at least 1/4" over 12" to minimize ponding.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

					•	115%	and 1	25 %	Load	Durat	tion				
								Δ IS [®] 25	Series —	. 91/2" - 16	" Depths				
					-						ange Wic			,	
					9½" AJS [®] 25			11%" AJS [®] 25			14" AJS [®] 25			16" AJS [®] 25	
		Live Load [psf]	Dead Load [psf]	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12	4/12 or Less	4/12 to 8/12	8/12 to 12/12
	Non-	20	10	30'-5"	28'-9"	26'-8"	36'-4"	34'-3"	31'–10"	41'-4"	38'–11"	36'-2"	45'-9"	43'-2"	40'-1"
	Snow	20	15	28'–10"	27'–1"	25'-0"	34'-5"	32'-4"	29'–10"	39'–1"	36'-9"	33'–11"	43'-4"	40'-8"	37'-7"
	125%	20	20	27'-6"	25'-9"	23'-8"	32'-10"	30'-9"	28'-3"	37'-4"	34'–11"	32'-2"	41'-4"	38'-9"	35'-7"
		25	10	28'–11"	27'-4"	25'-5"	34'-6"	32'-7"	30'-4"	39'-3"	37'–1"	34'-6"	43'-5"	41'–1"	38'-3"
4011		25	15	27'–7"	25'–11"	24'-0"	32'-11"	31'-0"	28'-8"	37'-5"	35'-3"	32'-7"	41'-5"	39'-0"	36'-2"
12" o.c.		30	10	27'-8"	26'-2"	24'-5"	33'-0"	31'-3"	29'-2"	37'-6"	35'-6"	33'-2"	41'-6"	39'-4"	36'-8"
O.C.	Snow	30	15	26'-6"	25'-0"	23'-2"	31'-8"	29'–10"	27'-8"	35'–11"	33'–11"	31'-6"	39'–10"	37'–7"	34'–11"
	115%	40	10	25'–2"	24'-1"	22'-9"	30'-0"	28'-9"	27'-2"	34'–1"	32'-9"	30'–11"	37'–10"	36'-3"	34'-3"
		40	15	24'-9"	23'-5"	21'–10"	29'–7"	28'-0"	26'–1"	33'–7"	31'–10"	29'-8"	37'–3"	35'-3"	32'–10"
		50	10	23'-3"	22'-4"	21'–2"	27'–9"	26'-8"	25'-4"	31'–7"	30'-4"	28'-9"	35'-0"	33'–7"	31'–11"
		50	15	23'–3"	22'-2"	20'-9"	27'–9"	26'-6"	24'-9"	31'–7"	30'–1"	28'–2"	35'-0"	33'-4"	31'–2"
	Non-	20	10	27'–7"	26'-0"	24'–2"	32'–11"	31'–1"	28'–10"	37'–5"	35'-4"	32'-9"	41'-6"	39'–2"	36'-4"
	Snow	20	15	26'–1"	24'-6"	22'-8"	31'–2"	29'-3"	27'–1"	35'-5"	33'-4"	30'-9"	39'–3"	36'–11"	34'–1"
	125%	20	20	24'–11"	23'-4"	21'–5"	29'–9"	27'–10"	25'–7"	33'–10"	31'–8"	29'–1"	37'-6"	35'–1"	32'-3"
		25	10	26'–2"	24'-9"	23'–1"	31'–3"	29'–7"	27'-6"	35'-6"	33'–7"	31'–3"	39'–5"	37'–3"	34'-8"
16"		25	15	25'-0"	23'-6"	21'–9"	29'–10"	28'–1"	26'-0"	33'–11"	31'–11"	29'–7"	37'–7"	35'-4"	32'-9"
0.C.	Snow	30	10	25'-0"	23'-8"	22'–1"	29'–10"	28'-4"	26'-5"	34'-0"	32'–2"	30'-0"	37'-8"	35'-8"	33'–3"
		30	15	24'-0"	22'-8"	21'-0"	28'-8"	27'-0"	25'–1"	32'–7"	30'-9"	28'-6"	36'–1"	34'–1"	31'–7"
	115%	40	10	22'–9"	21'–10"	20'–7"	27'–2"	26'–1"	24'-8"	30'–11"	29'–8"	28'-0"	34'–3"	32'–10"	31'-0"
		40	15	22'–5"	21'–2"	19'-9"	26'-9"	25'-4"	23'–7"	30'-5"	28'–10"	26'–10"	33'-8"	31'–11"	29'-9"
		50	10	21'-0"	20'-2"	19'–2"	25'–2"	24'–2"	22'–11"	28'–7"	27'–5"	26'–1"	31'–8"	30'-5"	28'–11"
		50	15	21'-0"	20'–1"	18'-9"	25'–2"	24'-0"	22'-5"	28'–7"	27'-3"	25'-6"	29'–7"	28'-8"	27'-4"
	Non-	20	10	25'–11"	24'-5"	22'-8"	30'–11"	29'–2"	27'–1"	35'–2"	33'–2"	30'–10"	39'-0"	36'-9"	34'–2"
	Snow 125%	20	15	24'–6"	23'-0"	21'–3"	29'-3"	27'-6"	25'-5"	33'–3"	31'–3"	28'–11"	36'–11"	34'-8"	32'-0"
	123/0	20	20	23'-5"	21'–11"	20'-2"	27'–11"	26'-2"	24'–1"	31'-9"	29'-9"	27'-4"	35'-2"	32'–11"	30'-4"
		25	10	24'–7"	23'-3"	21'-8"	29'-4"	27'-9"	25'–10"	33'-5"	31'–7"	29'-5"	37'-0"	35'-0"	32'-7"
19.2"		25	15	23'-5"	22'-1"	20'-5"	28'-0"	26'-4"	24'-5"	31'–10"	30'-0"	27'-9"	35'-3"	33'-3"	30'-9"
o.c.		30	10	23'-6"	22'-3"	20'-9"	28'-1"	26'-7"	24'–10"	31'–11"	30'-3"	28'-3"	35'-4"	33'-6"	31'–3"
	Snow 115%	30	15	22'-6"	21'-3"	19'-9"	26'-11"	25'-5"	23'-7"	30'-7"	28'-10"	26'-10"	33'–11"	32'-0"	29'-8"
	11376	40 40	10	21'–4" 21'–0"	20'-6"	19'–4" 18'–7"	25'–6" 25'–1"	24'-6" 23'-9"	23'–2" 22'–2"	29'-0" 28'-3"	27'–10" 27'–0"	26'-4" 25'-3"	32'–2" 29'–1"	30'-10"	29'–2" 26'–6"
		50	15 10		19'–11"									28'-0"	
		50		19'–9" 19'–9"	18'–11" 18'–10"	18'-0" 17'-7"	23'–7" 23'–3"	22'-8" 22'-6"	21'-6" 21'-0"	26'–1" 23'–11"	25'–5" 23'–2"	24'-6" 22'-1"	26'–10" 24'–8"	26'–2" 23'–10"	25'–3" 22'–9"
		20	15 10	24'-0"	22'-8"	21'-0"	28'-8"	27'-0"	25'-1"	32'-7"	30'-9"	28'-6"	36'–1"	34'-1"	31'-7"
	Non- Snow	20	15	24 –0	21'-4"	19'-8"	27'–1"	25'-6"	23'-6"	30'-10"	29'-0"	26'-9"	34'-2"	32'-1"	29'-8"
	125%	20	20	21'-8"	20'-3"	18'-8"	27 –1 25'–10"	24'-3"	23-6	29'-5"	29 –0	25'-4"	34 –2	30'-6"	29 – 8
		25	10	22'-9"	21'-6"	20'-1"	27'–2"	25'-9"	23'–11"	30'-11"	29'-3"	27'–3"	34'-3"	32'-5"	30'-2"
		25	15	21'-8"	20'-5"	18'–11"	25'–11"	24'-5"	22'-7"	29'-5"	27'-9"	25'-9"	31'–10"	30'-2"	28'–1"
24"		30	10	21'-8"	20'-7"	19'-3"	25'–11"	24'-7"	23'-0"	29'-6"	28'-0"	26'-2"	31-10	30'-11"	28'–11"
o.c.	Cnow	30	15	20'–10"	19'-8"	18'-3"	24'–10"	23'-6"	21'–10"	27'-7"	26'-3"	24'-7"	28'-4"	27'-0"	25'-4"
	Snow 115%	40	10	19'-9"	18'–11"	17'–11"	23'-7"	22'-8"	21'-10	24'–11"	24'-3"	23'-3"	25'-8"	24'–11"	23'–11"
		40	15	19'-5"	18'-5"	17'-11	21'–11"	21'–1"	19'–11"	22'-7"	21'-8"	20'-7"	23'-3"	22'-4"	21'–2"
		50	10	18'-3"	17'-6"	16'-8"	20'-2"	19'-8"	19'-0"	20'-9"	20'-3"	19'-7"	21'–5"	20'-11"	20'-2"
		50	15	17'–11"	17'-4"	16'-3"	18'-7"	17'–11"	17'–1"	19'–1"	18'-6"	17'-8"	19'-8"	19'-0"	18'-2"
		30	13	17 -11	17 -4	10 -3	10 -/	17 -11	171	15-1	10 -0	17 -0	13 -0	13 -0	10 -2



(in pounds per linear foot [PLF])

115% and 125% Load Duration

Use of these tables should be limited to roof slopes of $3\frac{1}{2}$ " per foot or less. For steeper slopes, see pages 16-20.

AJS[®] 140 Series 2½" Flange Width

	91	⁄2" AJS [®] 14	10	113	%" AJS® 1∙	40	1	4" AJS [®] 14	0	1	6" AJS [®] 14	.0
	Total	Load	Deflect.	Total	Load	Deflect.	Total	Load	Deflect.	Total	Load	Deflect.
		Non-			Non-			Non-			Non-	
Span Length	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240
6	353	383	-	358	389	-	360	392	-	364	396	-
7	302	329	-	307	334	-	309	336	-	312	339	-
8	264	287	-	269	292	-	270	294	-	273	297	-
9	235	255	-	239	259	-	240	261	-	242	264	-
10	211	230	-	215	233	-	216	235	-	218	237	-
11	182	198	-	195	212	-	196	213	-	198	216	-
12	153	166	-	179	194	-	180	196	-	182	198	-
13	130	142	-	165	179	-	166	180	-	168	182	-
14	112	122	-	146	158	-	154	168	-	156	169	-
15	98	106	-	127	138	-	144	156	-	145	158	-
16	86	93	85	111	121	-	134	146	-	136	148	-
17	76	83	71	99	107	-	119	129	-	128	139	-
18	68	74	60	88	96	-	106	115	-	121	132	-
19	61	66	51	79	86	_	95	103	-	110	120	_
20	55	58	44	71	77	-	86	93	-	99	108	-
21	50	50	38	64	70	-	78	85	-	90	98	-
22	44	44	33	59	64	56	71	77	-	82	89	-
23				54	58	49	65	70	-	75	82	-
24				49	54	44	59	65	-	69	75	-
25				45	49	39	55	59	-	63	69	_
26				42	45	34	51	55	-	59	64	-
27							47	51	45	54	59	-
28							43	47	41	51	55	_
29							41	44	37	47	51	-
30										44	48	_
31										41	45	_
32												
33												
34												

- Total Load values are limited by shear, moment, or deflection equal to L/180.
- Deflection values (Deflect.) are limited by live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC Calc* software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least ¼ inch over 12 inches to minimize ponding.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC Calc* software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

(in pounds per linear foot [PLF])

115% and 125% Load Duration

Use of these tables should be limited to roof slopes of $3\frac{1}{2}$ " per foot or less. For steeper slopes, see pages 16-20.

		AJS® 150 Series 2½" Flange Width												
	Ω1	/2" AJS [®] 15	5 0	11	∠: %" AJS° 1!			4" AJS [®] 15	0	1	6" AJS [®] 15	0		
	9:	/2 AJS 13	50	11	78 AJS 1:	50	I'	4 AJS 15	U	I'	0 AJS 13	U		
	Total	Load	Deflect.	Total	Load	Deflect.	Total	Load	Deflect.	Total	Load	Deflect.		
		Non-			Non-			Non-			Non-			
Span Length	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240		
6	353	383	-	358	389	-	360	392	-	364	396	-		
7	302	329	_	307	334	_	309	336	_	312	339	_		
8	264	287	-	269	292	-	270	294	-	273	297	-		
9	235	255	-	239	259	-	240	261	-	242	264	-		
10	211	230	-	215	233	-	216	235	-	218	237	-		
11	192	209	-	195	212	-	196	213	-	198	216	-		
12	176	191	-	179	194	-	180	196	-	182	198	-		
13	150	163	-	165	179	-	166	180	-	168	182	-		
14	129	141	-	153	167	-	154	168	-	156	169	-		
15	113	122	109	143	155	-	144	156	-	145	158	-		
16	99	107	90	128	139	-	135	147	-	136	148	-		
17	87	95	76	113	123	-	127	138	-	128	139	-		
18	78	84	64	101	110	-	120	130	-	121	132	-		
19	70	72	55	91	99	-	109	119	-	115	125	-		
20	62	62	47	82	89	79	98	107	-	109	118	-		
21	53	53	41	74	81	69	89	97	-	104	113	-		
22	47	47	36	67	73	60	81	88	-	94	103	-		
23	41	41	31	62	67	53	74	81	-	86	94	-		
24				57	61	47	68	74	-	79	86	-		
25				52	54	41	63	68	60	73	79	-		
26				48	48	37	58	63	54	67	73	-		
27				43	43	33	54	59	48	62	68	-		
28							50	54	43	58	63	-		
29							47	51	39	54	59	53		
30							43	46	35	50	55	48		
31							41	42	32	47	51	43		
32										44	48	40		
33										42	45	36		
34														

- Total Load values are limited by shear, moment, or deflection equal to L/180.
- Deflection values (Deflect.) are limited by live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC Calc* software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least ¼ inch over 12 inches to minimize ponding.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC Calc® software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

(in pounds per linear foot [PLF])

115% and 125% Load Duration

Use of these tables should be limited to roof slopes of $3\frac{1}{2}$ " per foot or less. For steeper slopes, see pages 16-20.

AJS® 20 Series 21/2" Flange Width 11%" AJS® 20 91/2" AJS® 20 14" AJS[®] 20 16" AJS[®] 20 Deflect. Deflect. Total Load Deflect. Deflect. Total Load Total Load Total Load Non-Non-Non-Non-Span Snow Snow Snow Snow Snow Snow Snow Snow Length (115%)L/240 L/240 L/240 (115%)L/240 (125%)(115%)(125%)(115%)(125%)(125%)

 Total Load values are limited by shear, moment, or deflection equal to L/180.

- Deflection values (Deflect.) are limited by live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC Calc* software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least ¼ inch over 12 inches to minimize ponding.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC Calc* software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

(in pounds per linear foot [PLF])

115% and 125% Load Duration

Use of these tables should be limited to roof slopes of $3\frac{1}{2}$ " per foot or less. For steeper slopes, see pages 16-20.

	1 of steeper slopes, see pages 10-20.											
					A	AJS® 190	3 Serie	S				
					21	⁄2" Flan	ge Wid	th				
	91	⁄2" AJS® 19	90	11 ⁷	⁄⁄₃" AJS [®] 19	90	1	4" AJS [®] 19	0	1	6" AJS [®] 19	0
	-		Б. б	.		D (1)				.		Б. (1.).
	Total		Deflect.	Total	Load	Deflect.	Total		Deflect.	lotal	Load	Deflect.
Span	C	Non-		C	Non-		C	Non-		C	Non-	
Length	Snow	Snow	1/240	Snow	Snow	1/240	Snow	Snow	1/240	Snow	Snow	1/240
	(115%)	(125%)	L/240	(115%)	(125%)	L/240	(115%)	(125%)	L/240	(115%)	(125%)	L/240
6	353	383	-	358	389	-	360	392	-	364	396	-
7	302	329	-	307	334	-	309	336	-	312	339	-
8	264	287	-	269	292	-	270	294	-	273	297	-
9	235	255	-	239	259	-	240	261	-	242	264	-
10	211	230	-	215	233	-	216	235	-	218	237	-
11	192	209	-	195	212	-	196	213	-	198	216	-
12	176	191	-	179	194	-	180	196	-	182	198	-
13	162	177	-	165	179	-	166	180	-	168	182	-
14	151	164	-	153	167	-	154	168	-	156	169	-
15	141	153	134	143	155	-	144	156	-	145	158	-
16	132	143	111	134	146	_	135	147	_	136	148	-
17	121	123	94	126	137	_	127	138	_	128	139	-
18	104	104	79	119	129	-	120	130	-	121	132	-
19	89	89	68	113	123	-	113	123	-	115	125	_
20	77	77	59	107	116	98	108	117	-	109	118	_
21	67	67	51	102	111	85	103	112	_	104	113	_
22	58	58	44	93	97	74	98	106	_	99	108	_
23	51	51	39	85	86	65	94	102	_	95	103	_
24	45	45	34	76	76	58	90	98	84	91	99	_
25	40	40	30	67	67	51	86	94	75	87	95	_
26	- 10			60	60	46	80	87	67	84	91	_
27				54	54	41	75	78	60	80	88	_
28				48	48	37	69	70	54	78	84	72
29				43	43	33	63	63	48	75	81	65
30				75	73	33	57	57	44	70	76	59
31							52	52	40	66	70	54
32							47	47	36	61	64	49
33							47	47	33	58	59	49
34							43	43	30	58 54	59	45
35							40	40	30			38
										49	49	
36										45	45	35
37										42	42	32
38												

- Total Load values are limited by shear, moment, or deflection equal to L/180.
- Deflection values (Deflect.) are limited by live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC Calc* software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least ¼ inch over 12 inches to minimize ponding.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC Calc* software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

(in pounds per linear foot [PLF])

115% and 125% Load Duration

Use of these tables should be limited to roof slopes of $3\frac{1}{2}$ " per foot or less. For steeper slopes, see pages 16-20.

AJS $^{\circ}$ 25 Series — $9\frac{1}{2}$ " - 16" Depths 3 %" Web Thickness — $3\frac{1}{2}$ " Flange Width

	9	½" AJS [®] 2	25	11	7⁄8" AJS® 2	25	1	4" AJS [®] 2!	5	1	6" AJS [®] 2	5
	Total	Load	Deflect.	Total	Load	Deflect.	Total	Load	Deflect.	Total	Load	Deflect.
		Non-			Non-			Non-			Non-	
Span Length	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240	Snow (115%)	Snow (125%)	L/240
6	356	387	-	358	389	-	360	392	-	364	396	-
7	305	332	-	307	334	-	309	336	-	312	339	-
8	267	290	-	269	292	-	270	294	-	273	297	-
9	237	258	-	239	259	-	240	261	-	242	264	-
10	214	232	-	215	233	-	216	235	-	218	237	-
11	194	211	-	195	212	-	196	213	-	198	216	-
12	178	193	-	179	194	-	180	196	-	182	198	-
13	164	179	-	165	179	-	166	180	-	168	182	-
14	152	166	_	153	167	_	154	168	_	156	169	-
15	142	155	_	143	155	_	144	156	_	145	158	_
16	133	145	_	134	146	_	135	147	-	136	148	-
17	125	136	121	126	137	_	127	138	_	128	139	_
18	118	129	103	119	129	-	120	130	-	121	132	-
19	112	116	88	113	123	-	113	123	-	115	125	-
20	100	100	76	107	116	_	108	117	_	109	118	-
21	87	87	66	102	111	-	103	112	-	104	113	-
22	76	76	58	93	102	-	98	106	-	99	108	-
23	67	67	51	85	93	_	94	102	-	95	103	-
24	59	59	45	78	85	75	90	98	_	91	99	_
25	52	52	40	72	79	67	86	94	_	87	95	-
26	46	46	35	67	73	59	80	87	-	84	91	-
27	42	42	32	62	67	53	75	81	-	80	88	-
28				58	63	48	69	75	_	78	84	_
29				54	57	43	65	70	63	75	81	_
30				50	51	39	60	66	57	70	76	_
31				47	47	35	56	61	52	66	71	_
32				42	42	32	53	58	47	61	67	_
33							50	54	43	58	63	-
34							47	51	39	54	59	53
35							44	47	36	51	56	49
36							42	44	33	48	53	45
37										46	50	41
38										43	47	38

- Total Load values are limited by shear, moment, or deflection equal to L/180.
- Deflection values (Deflect.) are limited by live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC Calc* software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least ¼ inch over
 12 inches to minimize ponding.
- Table values assume minimum bearing lengths without web stiffeners for joist depths
- of 16 inches and less. 18" joists require web stiffeners at all bearing locations.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC Calc* software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

							End Reaction [lbs]		Inter	mediate	Reaction	[lbs]		
AJS [®]	Damth	Maiodet	Moment	El x 10 ⁶	K x 10 ⁶	Shear V	11∕2" B€	earing	3½" B	earing	3½" Be	earing	51⁄4" B	earing
Joist Series	Depth [inches]	Weight [plf]	M [ft-lbs]	[lb-in ²]	[lbs]	v _r [lbs]	No WS ⁽¹⁾	WS ⁽²⁾						
	9½	2.2	2450	182	5.2	1160	950	1240	1175	1480	2350	2450	2350	2450
AJS®	117⁄8	2.5	3175	310	6.6	1490	955	1335	1215	1595	2390	2800	2390	2800
140	14	2.8	3825	457	7.8	1790	960	1420	1250	1700	2430	3130	2430	3130
	16	3.1	4435	623	9.0	2065	970	1500	1285	1800	2465	3435	2465	3435
	9½	2.2	2820	194	5.2	1160	950	1240	1175	1480	2350	2450	2350	2450
AJS [®]	117⁄8	2.5	3650	331	6.6	1490	955	1335	1215	1595	2390	2800	2390	2800
150	14	2.8	4390	487	7.8	1790	960	1420	1250	1700	2430	3130	2430	3130
	16	3.1	5090	664	9.0	2065	970	1500	1285	1800	2465	3435	2465	3435
	9½	2.5	3395	232	5.2	1160	950	1240	1175	1480	2350	2450	2350	2450
AJS [®]	117⁄8	2.8	4400	394	6.6	1490	955	1335	1215	1595	2390	2800	2390	2800
20	14	3.0	5295	578	7.8	1790	960	1420	1250	1700	2430	3130	2430	3130
	16	3.3	6140	786	9.0	2065	970	1500	1285	1800	2465	3435	2465	3435
	9½	2.5	3895	244	5.2	1160	950	1240	1175	1480	2350	2450	2350	2450
AJS [®]	117⁄8	2.8	5045	414	6.6	1490	955	1335	1215	1595	2390	2800	2390	2800
190	14	3.0	6070	608	7.8	1790	960	1420	1250	1700	2430	3130	2430	3130
	16	3.3	7040	827	9.0	2065	970	1500	1285	1800	2465	3435	2465	3435
	9½	3.1	5370	322	5.3	1160	950	1240	1175	1480	2600	2850	2600	2850
AJS [®]	117⁄8	3.4	6960	545	6.7	1490	955	1335	1215	1595	2690	3190	2690	3190
25	14	3.7	8380	798	7.9	1790	960	1420	1250	1700	2770	3500	2770	3500
	16	3.9	9720	1082	9.1	2065	970	1500	1285	1800	2850	3800	2850	3800

NOTES:

- (1) No web stiffeners required.
- (2) Web stiffeners required.
- (3) Not applicable, web stiffeners required.
- Moment, shear and reaction values based upon a load duration of 100% and may be adjusted for other load durations.
- Design values listed are applicable for Allowable Stress Design (ASD).
- No additional repetitive member increase allowed.

BUILDING CODE EVALUATION REPORT

- ICC-ES® / APA® ESR-1144 (IBC®, IRC®)

$$\Delta = \frac{5wl^4}{384 El} + \frac{wl^2}{K}$$

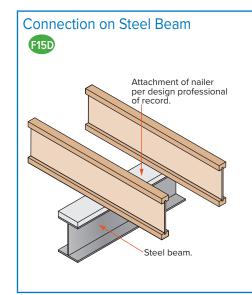
 Δ = deflection [in]

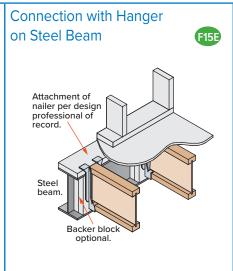
w = uniform load [lb/in]

// = clear span [in]

EI = bending stiffness [lb-in²]

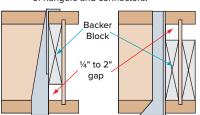
K = shear deformation coefficient [lb]





Hanger Connections to AJS Headers

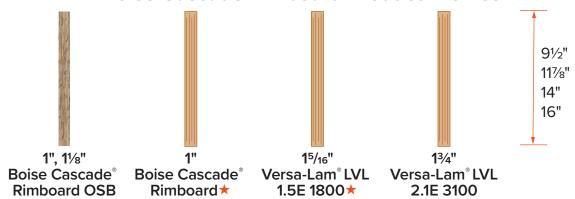
- Backer blocks shall be at least 12" long per hanger.
- Nails shall be clinched when possible.
- Verify capacity and fastening requirements of hangers and connectors.



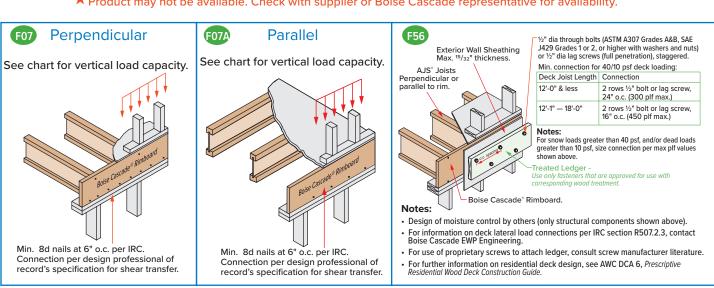
"Top Mount"
Backer block shall
be tight to bottom of
top flange with ¼" to
2" gap at top of
bottom flange.

"Face Mount"
Backer block shall be tight to top of bottom flange with ¼" to 2" gap at bottom of top flange.

Boise Cascade Rimboard Product Profiles



★ Product may not be available. Check with supplier or Boise Cascade representative for availability.



Boise Cascade Rimboard Properties

	Vertica Capa	al Load acity			Allowable D	esign Values	
Product	Uniform [plf]	Point [lb]	Maximum Floor Diaphragm Lateral Capacity [lb/ft]	Flexural Stress	Modulus of Elasticity [lb/in²]	Horizontal Shear [lb/in²]	Compression Perpendicular to Grain [lb/in²]
1" Boise Cascade [®] Rimboard ⁽²⁾ 1" Boise Cascade [®] Rimboard OSB ⁽²⁾	3300	3500	180	Limited span capabilities, see note 2			
11/8" Boise Cascade® Rimboard OSB (2)	4400	3500	180	Li	mited span capa	bilities, see note	2
1 ⁵ / ₁₆ " Versa-Lam [®] LVL 1.5E 1800 ⁽¹⁾	6000	4450	Permitted per building code for all nominal 2" thick framing floor diaphragms	1800	1,400,000	225	525
1¾" Versa-Lam [®] LVL 2.1E 3100 ⁽¹⁾	5700	4300	Permitted per building code for all nominal 2" thick framing floor diaphragms	3100	2,000,000	285	750

Closest Allowab	le Nail Snacing -		Product		
	Face [in]	1" Boise Cascade [®] Rimboard ⁽²⁾ 1" Boise Cascade [®] Rimboard OSB ⁽²⁾	11/8" Boise Cascade [®] Rimboard OSB ⁽²⁾	15/16" Versa-Lam® LVL 1.5E 1800 ⁽¹⁾	1¾" Versa-Lam [®] LVL 2.1E 3100 ⁽¹⁾
8d Box	(0.113"ø x 2.5")	3	3	3	3
8d Common	(0.131"ø x 2.5")	3	3	3	3
10d & 12d Box (0	.128"ø x 3", 3.25")			3	3
16d Box	(0.135"ø x 3.5")	See publication in r	note 2 for	3	3
10d & 12d Comm 16d Sinker (0	on & .148"ø x 3", 3.25")	further nailing info		4	4
16d Common	(0.162"ø x 3.5")			6	6

Notes

- See ICC-ES®/APA® ESR-1040 for further information.
- 2. See Performance Rated Rim Boards, APA® Form No. W345N for further product information.

An Introduction to Versa-Lam® LVL Products



When you specify Versa-Lam® laminated veneer headers/beams, you are building quality into your design. They are excellent as floor and roof framing supports or as headers for doors, windows and garage doors and columns.

Because they have no camber, Versa-Lam® LVL products provide flatter, quieter floors, and consequently, the builder can expect happier customers with significantly fewer call backs.

Versa-Lam® LVL Beam Architectural Specifications

Scope: This work includes the complete furnishing and installation of all Versa-Lam* LVL beams as shown on the drawings, herein specified and necessary to complete the work.

Materials: Primarily Southern Pine or Douglas fir veneers, laminated in a press with all grain parallel with the length of the member. Glues used in lamination are phenol formaldehyde and isocyanate exterior-type adhesives which comply with ASTM D2559.

Design: Versa-Lam* LVL beams shall be sized and detailed to fit the dimensions and loads indicated on the plans. All designs shall be in accordance with allowable values developed in accordance with ASTM D5456 and listed in the governing

code evaluation service's report and section properties based upon standard engineering principles. Verification of design of the Versa-Lam® LVL beams by complete calculations shall be available upon request.

Drawings: Additional drawings showing layout and detail necessary for determining fit and placement in the buildings are (are not) to be provided by the supplier.

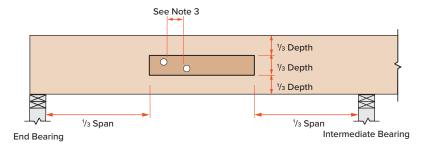
Fabrication: Versa-Lam* LVL beams shall be manufactured in a plant evaluated for fabrication by the governing code evaluation service and under the supervision of a third-party inspection agency listed by the corresponding evaluation service.

Storage and Installation: Versa-Lam*
LVL beams, if stored prior to erection, shall be stored on stickers spaced a maximum of 15 ft. apart. Beams shall be stored on a dry, level surface and protected from the weather. They shall be handled with care so they are not damaged.

Versa-Lam* LVL beams are to be installed in accordance with the plans and Boise Cascade EWP's Installation Guide. Temporary construction loads which cause stresses beyond design limits are not permitted. Erection bracing shall be provided to assure adequate lateral support for the individual beams and the entire system until the sheathing material has been applied.

Codes: Versa-Lam® LVL beams shall be evaluated by a model code evaluation service.

Allowable Holes in Versa-Lam® LVL Beams

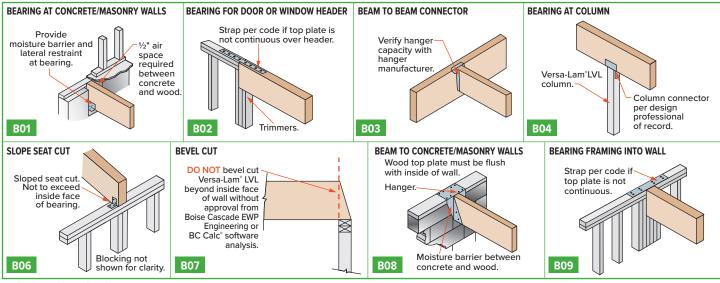


- 1. Square and rectangular holes are not permitted.
- 2. Round holes may be drilled or cut with a hole saw anywhere within the shaded area of the beam.
- 3. The horizontal distance between adjacent holes must be at least two times the size of the larger hole.
- 4. Do not drill more than three access holes in any four foot long section of beam.

5. The maximum round hole diameter permitted is:

Beam Depth	Max. Hole Diameter
51/2"	3/4"
71⁄4"	1"
9¼" and greater	2"

- 6. These limitations apply to holes drilled for plumbing or wiring access only. The size and location of holes drilled for fasteners are governed by the provisions of the National Design Specification* for Wood Construction.
- 7. Beams deflect under load. Size holes to provide clearance where required.
- 8. This hole chart is valid for beams supporting uniform load only. For beams supporting concentrated loads or for beams with larger holes, use BC Calc® sizing software (www.BCCalc.com) or contact Boise Cascade EWP Engineering.



INSTALLATION NOTES

- Minimum of ½" air space between beam and wall pocket or adequate barrier must be provided between beam and concrete/masonry.
- Adequate bearing shall be provided. If not shown on plans, please refer to load tables on pages 30-32 of this quide.
- Versa-Lam® LVL beams are intended for interior applications only and should be kept as dry
 as possible during construction.
- · Continuous lateral support of top of beam shall be provided (side or top bearing framing).

Multiple Member Connectors

		Sid	e-Loac	led Ap	plicati	ons		
			Maxim	um Unifor	m Side Lo	ad [plf]		
Number	Naile	ed ⁽³⁾	½" Di	a. Through	Bolt (1)	5⁄8" Di	a. Through	Bolt ⁽¹⁾
of Members	2 rows 16d Sinkers @ 12" o.c.	3 rows 16d Sinkers @ 12" o.c.	2 rows @ 24" o.c. staggered	2 rows @ 12" o.c. staggered	2 rows @ 6" o.c. staggered	2 rows @ 24" o.c. staggered	2 rows @ 12" o.c. staggered	2 rows @ 6" o.c. staggered
		1¾" Vei	rsa-Lam® L	.VL (Depth		nd less)		
2	470	705	505	1010	2020	560	1120	2245
3 (2)	350	525	375	755	1515	420	840	1685
4 (3)	use bolt	schedule	335	670	1345	370	745	1495
			31/2" \	Versa-Lam	° LVL			
2(3)	use bolt	schedule	855	1715	N/A	1125	2250	N/A
		1¾" Ve	rsa-Lam® L	VL (Depth	s of 24" aı	nd less)		
Number	Naile	ed ⁽³⁾	½" Di	a. Through	Bolt (1)	5⁄8" Di	a. Through	Bolt ⁽¹⁾
of Members	3 rows 16d Sinkers @ 12" o.c.	4 rows 16d Sinkers @ 12" o.c.	3 rows @ 24" o.c. 8" staggered	3 rows @ 18" o.c. 6" staggered	3 rows @ 12" o.c. 4" staggered	3 rows @ 24" o.c. 8" staggered	3 rows @ 18" o.c. 6" staggered	3 rows @ 12" o.c. 4" staggered
2	705	940	755	1010	1515	840	1120	1685
3 (2)	525	705	565	755	1135	630	840	1260
4 (4)	use bolt	schedule	505	670	1010	560	745	1120

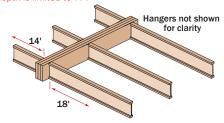
- Design values apply to common bolts that conform to ANSI/ASME standard B18.21-1981 (ASTM A307 Grades A&B, SAE J429 Grades 10 r 2, or higher). A washer not less than a standard cut washer shall be between the wood and the bolt head and between the
- wood and the nut. The distance from the edge of the beam to the bolt holes must be at least 2" for $\frac{1}{2}$ " bolts and $2\frac{1}{2}$ " for $\frac{1}{2}$ " bolts. Bolt holes shall be the same diameter as the bolt.
- The nail schedules shown apply to both sides of a 3-member beam.
- 3. 16d box nails = 0.135" diameter x 3.5" length, 16d sinker nails = 0.148" diameter x 3.25" length.
 4. 7" wide beams must be top-loaded
- 7" wide beams must be top-loaded or loaded from both sides (lesser side shall be no less than 25% of opposite side)

Multiple Versa-Lam® LVL Members When using multiple ply Versa-Lam® LVL beams to create a wider

Designing Connections For

When using multiple ply Versa-Lam* LVL beams to create a wider member, the connection of the plies is as critical as determining the beam size. When side loaded beams are not connected properly, the inside plies do not support their share of the load and thus the load-carrying capacity of the full member decreases significantly. The following is an example of how to size and connect a multiple-ply Versa-Lam* LVL floor beam.

Given: Beam shown below is supporting residential floor load (40 psf live load, 10 psf dead load) and is spanning 16'-0". Beam depth is limited to 14".



Find: A multiple 1¾" ply Versa-Lam® LVL that is adequate to support the design loads and the member's proper connection schedule.

- 1. Calculate the tributary width that beam is supporting: 14' / 2 + 18' / 2 = 16'
- Use PLF tables on pages 3-5 of this guide or BC Calc* to size beam.
 A Triple Versa-Lam* LVL 2.1 3100 1¾" x 14" is found to adequately support the design loads
- Calculate the maximum plf load from one side (the right side in this case).

Max. Side Load = $(18' / 2) \times (40 + 10 \text{ psf}) = 450 \text{ plf}$

- Go to the Multiple Member Connection Table, Side-Loaded Applications, 1¾" Versa-Lam[®] LVL, 3 members.
- 5. The proper connection schedule must have a capacity greater than the max. side load:

Nailed: 3 rows 16d sinkers @ 12" o.c: 525 plf is greater than 450 plf OK Bolts: ½" diameter 2 rows @ 12" staggered: 755 plf is greater than 450 plf OK

Top-Loaded Applications

		Toda de de la	
For top-loaded	beams and beams with	side loads less than shown in Side-Loaded Appl	ications table above:
Plies	Depth	Nailing ⁽²⁾	Maximum Uniform Load From One Side
	Depths 11%" & less	2 rows 16d box/sinker nails @ 12" o.c.	400 plf
(2) 1¾" plies	Depths 14" - 18"	3 rows 16d box/sinker nails @ 12" o.c.	600 plf
	Depth = 24"	4 rows 16d box/sinker nails @ 12" o.c.	800 plf
	Depths 11%" & less	2 rows 16d box/sinker nails @ 12" o.c.	300 plf
(3) 1¾" plies (1)	Depths 14" - 18"	3 rows 16d box/sinker nails @ 12" o.c.	450 plf
	Depth = 24"	4 rows 16d box/sinker nails @ 12" o.c.	600 plf
/4\ 43/!! mlinn	Depths 18" & less	2 rows ½" bolts @ 24" o.c., staggered	335 plf
(4) 1¾" plies	Depth = 24"	3 rows 1/2" bolts @ 24" o.c., staggered every 8"	505 plf
(2) 21/" plies	Depths 18" & less	2 rows ½" bolts @ 24" o.c., staggered	855 plf
(2) 3½" plies	Depth 20" - 24"	3 rows 1/2" bolts @ 24" o.c., staggered every 8"	1285 plf

- The nail schedules shown apply to both sides of a 3-member beam.
- 16d box nails = 0.135" diameter x 3.5" length, 16d sinker nails = 0.148" diameter x 3.25" length.
- Beams wider than 7" must be designed by the engineer of record.
- 4. All values in these tables may be increased
- by 15% for snow-load roofs and by 25% for non-snow load roofs where the building code allows.
- 5. Use allowable load tables or BC Calconsoftware to size beams.6. An equivalent specific gravity of 0.5
- may be used when designing specific connections with Versa-Lam*.
- Connection values are based upon the NDS, 2018 Edition.
- FastenMaster TrussLOK®, Simpson Strong-Tie SDW or SDS, and USP WS screws may also be used to connect multiple member Versa-Lam® LVL beams, contact Boise Cascade EWP Engineering for further information.

Versa-Lam® LVL 2.1E 3100

(100% Load Duration)

KEY TO TABLE:

Top Figure - Allowable Total Load [plf] Middle Figure - Allowable Live Load [plf]

Bottom Figure - Minimum Required Bearing Length at End / Intermediate Supports [inches]

				Bot	tom I	Figure	. - I/	<u>/linim</u>	ium F	≀equii	rea B	earınç	g Len	gtn a	t Ena	/ Inte						
SPAN		ersa-La	am° 2.1I	3100		ble Ply or 3½"							¾" Vers Versa-l				Quadr		/ 1¾" V /ersa-L			
(ft)	71/4"	91/2"	11%"	14"	71/4"	91/2"	11%"	14"	16"	18"	91/2"	11%"	14"	16"	18"	20"	11%"	14"	16"	18"	20"	24"
	763	1063	1424	1795	1525	2126	2849	3590	4387	4794	3189	4273	5384	6580	7191	7188	5697	7179	8773	9588	9584	9576
6	693	-	-	-	1385	-	-	-		-	-	-	-		-	-	-	-		-	-	-
	1.8 / 4.4 636	2.4 / 6.1 877	3.3 / 8.2 1160	4.1 / 10.3 1444	1.8 / 4.4 1271	2.4 / 6.1 1753	3.3 / 8.2 2321	4.1 / 10.3 2888	3482	5.5 / 13.8 4107	2.4 / 6.1 2630	3.3 / 8.2	4.1 / 10.3	5/12.6	6160	5.5 / 13.8 6157	3.3 / 8.2 4641	4.1 / 10.3 5775	5 / 12.6 6964	8213	8209	5.5 / 13.8 8201
7	452	-	-	-	905	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	1.7 / 4.3	2.4 / 5.9	3.1 / 7.8	3.9 / 9.7	1.7 / 4.3	2.4 / 5.9	3.1 / 7.8	3.9 / 9.7	4.7 / 11.7	5.5 / 13.8	2.4 / 5.9	3.1 / 7.8	3.9 / 9.7	4.7 / 11.7		5.5 / 13.8	3.1 / 7.8	3.9 / 9.7	4.7 / 11.7	5.5 / 13.8	5.5 / 13.8	5.5 / 13.8
_	462	746	979	1207	924	1492	1957	2414	2886	3402	2237	2936	3622	4328	5103	5384	3914	4829	5771	6803	7178	7170
8	310 1.5 / 3.5	660 2.3 / 5.7	3 / 7.5	3.7 / 9.3	621 1.5 / 3.5	1321	- 3 / 7.5	3.7 / 9.3	4.4 / 11.1	5.2 / 13	1981 2.3 / 5.7	3 / 7.5	3.7 / 9.3	4.4 / 11.1	5.2 / 13	5.5 / 13.8	- 3 / 7.5	3.7 / 9.3	4.4 / 11.1	5.2 / 13	- 55/138	5.5 / 13.8
	329	649	846	1037	658	1297	1692	2074	2463	2884	1946	2537	3111	3694	4325	4782	3383	4148	4926	5767	6376	6368
9	222	477	-	-	444	954	-	-	-	-	1431	-	-	-	-	-	-	-	-	-	-	-
	1.5 / 3	2.2 / 5.6	2.9 / 7.3	3.6 / 8.9	1.5 / 3	2.2 / 5.6	2.9 / 7.3	3.6 / 8.9			2.2 / 5.6		3.6 / 8.9	4.3 / 10.6		5.5 / 13.8		3.6 / 8.9	4.3 / 10.6			5.5 / 13.8
10	242 164	527 355	745 660	909	484 327	1055 710	1489 1321	1817	2148	2502	1582 1065	2234 1981	2726	3222	3753	4301	2978 2642	3635	4296	5003	5734	5726
10	1.5 / 3	2 / 5.1	2.9 / 7.1	3.5 / 8.7	1.5 / 3	2 / 5.1	2.9 / 7.1	3.5 / 8.7	4.1 / 10.3	4.8 / 12	2/5.1	2.9 / 7.1	3.5 / 8.7	4.1 / 10.3	4.8 / 12	5.5 / 13.8	2.9 / 7.1	3.5 / 8.7	4.1 / 10.3	4.8 / 12	5.5 / 13.8	5.5 / 13.8
	183	401	665	808	365	803	1330	1617	1904	2209	1204	1995	2425	2856	3313	3800	2659	3233	3807	4417	5067	5201
11	124	271	508	798	248	541	1015	1595	-	-	812	1523	2393	-	-	-	2031	3190	-	-	-	-
	1.5 / 3	1.7 / 4.3	2.8 / 7	3.4 / 8.5	1.5 / 3	1.7 / 4.3	2.8/7	3.4 / 8.5	4 / 10.1	4.7 / 11.7	1.7 / 4.3	2.8 / 7	3.4 / 8.5	4 / 10.1	4.7 / 11.7	5.4 / 13.4	2.8 / 7	3.4 / 8.5	4 / 10.1	4.7 / 11.7		5.5 / 13.8
12	141 96	312 211	585 398	728 629	282 193	623 422	1170 796	1456 1258	1709	1977	935 633	1755 1194	2184 1887	2564	2965	3390	2340 1592	2912 2517	3418	3953	4519	4764
12	1.5 / 3	1.5 / 3.6	2.7 / 6.8	3.4 / 8.4	1.5 / 3	1.5 / 3.6	2.7 / 6.8	3.4 / 8.4	3.9 / 9.9	4.6 / 11.4	1.5 / 3.6	2.7 / 6.8	3.4 / 8.4	3.9 / 9.9	4.6 / 11.4	5.2 / 13	2.7 / 6.8	3.4 / 8.4	3.9 / 9.9	4.6 / 11.4	5.2 / 13	5.5 / 13.8
	111	246	470	662	221	493	941	1324	1550	1789	739	1411	1986	2326	2683	3059	1881	2647	3101	3577	4078	4394
13	76	168	318	504	152	335	635	1009	1456	-	503	953	1513	2185	-	-	1270	2017	2913	-	-	-
	1.5 / 3	1.5 / 3.1	2.4 / 5.9	3.3 / 8.3	1.5 / 3	1.5 / 3.1	2.4 / 5.9	3.3 / 8.3	3.9 / 9.7	4.5 / 11.2	1.5 / 3.1	2.4 / 5.9		3.9 / 9.7	4.5 / 11.2	5.1 / 12.7	2.4 / 5.9	3.3 / 8.3	3.9 / 9.7	4.5 / 11.2		5.5 / 13.8
14	88 61	198 135	380 257	585 410	176 123	396 270	759 514	1171 820	1418 1189	1633	594 405	1139 771	1756 1230	2128 1783	2449	2786	1519 1029	2342 1640	2837 2378	3265	3715	4076
14	1.5 / 3	1.5 / 3	2.1 / 5.1	3.2 / 7.9	1.5 / 3	1.5 / 3	2.1 / 5.1	3.2 / 7.9	3.8 / 9.6	4.4 / 11	1.5 / 3	2.1 / 5.1	3.2 / 7.9	3.8 / 9.6	4.4 / 11	5 / 12.5	2.1 / 5.1	3.2 / 7.9	3.8 / 9.6	4.4 / 11	5 / 12.5	5.5 / 13.8
	71	161	310	499	143	322	621	998	1307	1502	483	931	1497	1960	2253	2558	1242	1997	2614	3003	3410	3801
15	50	111	211	338	100	221	422	675	982	1359	332	633	1013	1473	2039	-	844	1350	1964	2718	-	-
	1.5 / 3 58	1.5 / 3	1.8 / 4.5 257	2.9 / 7.2 414	1.5 / 3 117	1.5 / 3 265	1.8 / 4.5 514	2.9 / 7.2 829	3.8 / 9.5 1151	4.3 / 10.9 1390	1.5 / 3 397	1.8 / 4.5 770	2.9 / 7.2 1243	3.8 / 9.5 1727	4.3 / 10.9 2085	4.9 / 12.3 2364	1.8 / 4.5 1027	2.9 / 7.2 1658	3.8 / 9.5 2303	4.3 / 10.9 2780	4.9 / 12.3 3151	5.5 / 13.8 3561
16	41	92	175	281	83	183	350	562	820	1138	275	526	843	1230	1707	2279	701	1124	1640	2277	3038	3301
10	1.5 / 3	1.5 / 3	1.6 / 4	2.6 / 6.4	1.5 / 3	1.5 / 3	1.6 / 4	2.6 / 6.4	3.6 / 8.9		1.5 / 3	1.6 / 4	2.6 / 6.4		4.3 / 10.7	4.9 / 12.2	1.6 / 4	2.6 / 6.4	3.6 / 8.9	4.3 / 10.7		5.5 / 13.8
		110	214	347	96	220	429	695	1018	1274	330	643	1042	1527	1911	2196	858	1389	2036	2547	2929	3348
17		77	147	236	69	153	294	473	691	962	230	441	709	1037	1443	1931	588	945	1382	1924	2575	-
		1.5 / 3 92	1.5 / 3.6 181	2.3 / 5.7	1.5 / 3 80	1.5 / 3 185	1.5 / 3.6 361	2.3 / 5.7 587	3.3 / 8.4 865	4.2 / 10.5 1134	1.5 / 3 277	1.5 / 3.6 542	2.3 / 5.7 881	3.3 / 8.4 1298	4.2 / 10.5 1701	4.8 / 12 2051	1.5 / 3.6 723	2.3 / 5.7	3.3 / 8.4 1731	4.2 / 10.5 2268	4.8 / 12 2735	5.5 / 13.8 3160
18		65	124	201	58	130	249	401	588	820	194	373	602	882	1230	1650	498	802	1176	1640	2200	-
		1.5 / 3	1.5 / 3.2	2.1 / 5.2	1.5 / 3	1.5 / 3	1.5 / 3.2	2.1 / 5.2	3 / 7.6	4/9.9	1.5 / 3	1.5 / 3.2	2.1 / 5.2	3 / 7.6	4/9.9	4.8 / 11.9	1.5 / 3.2	2.1 / 5.2	3 / 7.6	4/9.9	4.8 / 11.9	5.5 / 13.8
		78	153	250	67	156	307	500	739	1016	234	460	751	1109	1524	1863	614	1001	1479	2032	2484	2991
19		55 1.5 / 3	106 1.5 / 3	172 1.9 / 4.7	50 1.5 / 3	110	213 1.5 / 3	343 1.9 / 4.7	504 2.7 / 6.8	704 3.7 / 9.4	166 1.5 / 3	319 1.5 / 3	515 1.9 / 4.7	756 2.7 / 6.8	1056 3.7 / 9.4	1420 4.6 / 11.4	425 1.5 / 3	686 1.9 / 4.7	1008	1408 3.7 / 9.4	1893	5.5 / 13.8
		66	131	215	57	133	263	429	636	895	199	394	644	954	1343	1678	525	859	1272	1790	2237	2839
20		47	92	148	43	95	183	296	435	609	142	275	444	652	913	1230	366	592	870	1218	1640	2718
		1.5 / 3	1.5 / 3	1.7 / 4.2	1.5 / 3	1.5 / 3	1.5 / 3	1.7 / 4.2	2.5 / 6.2	3.5 / 8.7	1.5 / 3	1.5 / 3	1.7 / 4.2		3.5 / 8.7	4.3 / 10.8	1.5 / 3	1.7 / 4.2	•	3.5 / 8.7	4.3 / 10.8	5.5 / 13.8
22			98	161		98	196	322	479	678	147	293	483	719	1016	1379	391	644	959	1355	1839	2576
22			69 1.5 / 3	112		72 1.5 / 3	138 1.5 / 3	224 1.5 / 3.5	330 2.1 / 5.2	464 2.9 / 7.3	107 1.5 / 3	208 1.5 / 3	336 1.5 / 3.5	496 2.1 / 5.2	696 2.9 / 7.3	940	277 1.5 / 3	448 1.5 / 3.5	661 2.1 / 5.2	928	1253	2091 5.5 / 13.8
			74	123		73	1.5 / 3	246	369	523	110	223	370	553	785	1070	297	493	738	1047	1426	2184
24			54	87		55	107	174	257	361	83	161	261	385	542	733	214	348	513	722	978	1640
			1.5 / 3	1.5 / 3		1.5 / 3	1.5 / 3	1.5 / 3		2.5 / 6.2	1.5 / 3	1.5 / 3	1.5 / 3		2.5 / 6.2		1.5 / 3	1.5 / 3	1.8 / 4.4			5.1 / 12.8
20			57	96		56	115	192	289	411	84	172	288	433	617	844	230	384	577	823	1125	1853
26			42 1.5 / 3	69 1.5 / 3		44 1.5 / 3	85 1.5 / 3	137 1.5 / 3	203	286	65 1.5 / 3	127 1.5 / 3	206 1.5 / 3	305 1.5 / 3.8	430 2.1 / 5.3	583 2.9 / 7.2	169 1.5 / 3	275 1.5 / 3	407 1.5 / 3.8	573 2.1 / 5.3	777 2.9 / 7.2	1308 4.7 / 11.8
				76			90	151	229	328	64	135	227	344	492	675	180	303	458	656	900	1541
28				55			68	110	164	231	53	102	166	245	346	470	136	221	327	462	627	1060
				1.5 / 3			1.5 / 3	1.5 / 3	1.5 / 3.3		1.5 / 3	1.5 / 3	1.5 / 3	1.5 / 3.3		2.5 / 6.3	1.5 / 3	1.5 / 3	1.5 / 3.3	1.8 / 4.6		4.2 / 10.6
20				60			71 55	121	184	265	50	106	181	276	397	547	142	242	368	530	729 512	1256
30				45 1.5 / 3			55 1.5 / 3	90	134	189	43 1.5 / 3	83 1.5 / 3	135	200 1.5 / 3	283 1.6 / 4	385 2.2 / 5.5	111 1.5 / 3	180 1.5 / 3	267 1.5 / 3	378 1.6 / 4	513 2.2 / 5.5	870 3.7 / 9.3
Totali		ies are lin			<u> </u>				*	'						'	·					

- Total Load values are limited by shear, moment or deflection equal to L/240. Total Load values are the capacity of the beam in addition to its own weight.

 Live Load values are limited by deflection equal to L/360. Check the local building code for other
- deflection limits that may apply.
- Where a Live Load value is not shown, the Total Load value will control.
- Table values represent the most restrictive of simple or multiple span applications. Span is measured center to center of the supports. Analyze multiple span beams with BC Calc software if the length of any span is less than half the length of an adjacent span.
- Table values assume that lateral support is provided at each support and continuously along the top edge and applicable compression edges of the beam.
- Table values for Minimum Required Bearing Lengths are based on the allowable compression design value perpendicular to grain for the beam and the Total Load value shown. Other design considerations, such as a weaker support material, may warrant longer bearing lengths. Table values assume that support is provided across the full width of the beam.

 For 2-ply, 3-ply or 4-ply beams; double, triple or quadruple Allowable Total Load and Allowable Live Load values. Minimum Required Bearing Lengths remain the same for any number of plies.
- 1¾ inch members deeper than 14 inches are to be used as multiple-member beams only.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with BC Calc* software.

Versa-Lam® LVL 2.1E 3100

(115% Load Duration)

KEY TO TABLE:

Top Figure Middle Figure - Allowable Live Load [plf] Rottom Figure

- Allowable Total Load [plf]

- Minimum Required Bearing Length at End / Intermediate Supports (inches)

				Bot	tom F	Figure	e - N	⁄linim	ium F	≀equii	red Be	earing	g Len	gth a	t End	/Inte	ermed	diate :	Supp	orts [inche	s]
SPAN	1¾" V	'ersa-La	am® 2.1E	3100	Dou	uble Ply or 3½"					Triț		I¾" Ver Versa-l				Quadrı			rsa-Lar m° 2.1E		3100 or
(ft)	71/4"	91/2"	11%"	14"	71/4"	91/2"	11%"	14"	16"	18"	91/2"	11%"	14"	16"	18"	20"	11%"	14"	16"	18"	20"	24"
	878	1223	1639	2065	1755	2446	3278	4130	4796	4794	3669	4917	6195	7194	7191	7188	6556	8260	9592	9588	9584	9576
6		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	2 / 5 731	2.8 / 7	3.8 / 9.4 1335	4.7 / 11.8 1661	2 / 5 1463	2.8 / 7	3.8 / 9.4 2670	4.7 / 11.8 3323	5.5 / 13.8 4007	5.5 / 13.8 4107	2.8 / 7 3027	3.8 / 9.4 4006	4.7 / 11.8 4984	5.5 / 13.8 6010		5.5 / 13.8 6157	3.8 / 9.4 5341		5.5 / 13.8 8013			5.5 / 13.8
7	678	1009	1335	- 1001	1357	2018	2670	3323	4007	4107	3027	4006	4984	5010	6160	6157	5341	6646	8013	8213	8209	8201
′	2 / 4.9	2.7 / 6.8	3.6 / 8.9		2/4.9	2.7 / 6.8	3.6 / 8.9	4.4 / 11.1	5.4 / 13.4	5.5 / 13.8	2.7 / 6.8	3.6 / 8.9	4.4 / 11.1	5.4 / 13.4	5.5 / 13.8	5.5 / 13.8	3.6 / 8.9	4.4 / 11.1	5.4 / 13.4		5.5 / 13.8	5.5 / 13.8
	598	858	1126	1389	1197	1717	2252	2779	3321	3591	2575	3379	4168	4981	5387	5384	4505	5558	6642	7182	7178	7170
8	466	-	-	- 4 2 /40 C	931	-	-	- 4.0.740.0		-	-	-	- 4 2 /40 C	-	-	-	-	- 4.0.740.0	-	-	-	-
	1.8 / 4.6 440	2.6 / 6.6 747	3.5 / 8.6 974	4.3 / 10.6 1194	1.8 / 4.6 880	2.6 / 6.6	3.5 / 8.6 1947	4.3 / 10.6 2387	2835	5.5 / 13.8 3190	2.6 / 6.6	2921	4.3 / 10.6 3581	4252	4785	5.5 / 13.8 4782	3.5 / 8.6	4.3 / 10.6 4774	5.1 / 12.7 5670	6380	6376	5.5 / 13.8 6368
9	333	715	-	-	665	1431	-	-	-	-	2146	-	-	-	-	-	-	-	-	-	-	-
	1.5 / 3.8	2.6 / 6.4	3.4 / 8.4		1.5 / 3.8	2.6 / 6.4	3.4 / 8.4	4.1 / 10.3	4.9 / 12.2	5.5 / 13.8	2.6 / 6.4	3.4 / 8.4	4.1 / 10.3	4.9 / 12.2	5.5 / 13.8	5.5 / 13.8	3.4 / 8.4	4.1 / 10.3	4.9 / 12.2	5.5 / 13.8	5.5 / 13.8	5.5 / 13.8
	324	637	857	1046	648	1274	1714	2092	2472	2869	1912	2571	3138	3709	4304	4301	3429	4184	4945	5738	5734	5726
10	246 1.5 / 3.1	532 2.4 / 6.1	3.3 / 8.2	4 / 10	491 1.5 / 3.1	1065	3.3 / 8.2	- 4 / 10	4.7 / 11.9	5.5 / 13.8	1597 2.4 / 6.1	3.3 / 8.2	4/10	47/110	- E E / 12 0	- 5.5 / 13.8	- 3.3 / 8.2	- 4 / 10	47/110	- E E / 12 0	- E E / 12 0	5.5 / 13.8
	245	526	765	931	489	1052	1531	1861	2192	2543	1577	2296	2792	3288	3814	3907	3062	3723	4.77 11.9	5085	5209	5201
11	186	406	762	-	372	812	1523	-	-	-	1218	2285	-	-	-	-	3046	-	-	-	-	-
	1.5 / 3	2.2 / 5.6	3.2 / 8.1	3.9 / 9.8	1.5 / 3	2.2 / 5.6	3.2 / 8.1	3.9 / 9.8	4.6 / 11.6	5.4 / 13.4	2.2 / 5.6	3.2 / 8.1	3.9 / 9.8	4.6 / 11.6	5.4 / 13.4	5.5 / 13.8	3.2 / 8.1	3.9 / 9.8	4.6 / 11.6	5.4 / 13.4	5.5 / 13.8	5.5 / 13.8
	189	417	674	838	378	834	1347	1676	1968	2276	1252	2021	2514	2952	3414	3579	2694	3353	3936	4552	4772	4764
12	144	317	597	20/07	289	633	1194	20/07	4.5 / 11.3	- F 2 / 12 1	950	1791	20/07	- 4 E / 44 O		- F F /42 0	2389	20/07	- 4 E / 44 2		-	- F F /12 0
	1.5 / 3 149	1.9 / 4.8	3.1 / 7.8 573	3.9 / 9.7 762	1.5 / 3 297	1.9 / 4.8	3.1 / 7.8 1146	3.9 / 9.7 1524	1785	2060	1.9 / 4.8 991	3.1 / 7.8 1719	3.9 / 9.7 2287	2678	3089	5.5 / 13.8 3301	3.1 / 7.8 2292	3.9 / 9.7	3571	4119	4402	5.5 / 13.8 4394
13	114	251	476	756	229	503	953	1513	-	-	754	1429	2269	-	-	-	1905	3026	-	-	-	-
	1.5 / 3	1.7 / 4.1	2.9 / 7.2	3.8 / 9.5	1.5 / 3	1.7 / 4.1	2.9 / 7.2	3.8 / 9.5	4.5 / 11.2	5.1 / 12.9	1.7 / 4.1	2.9 / 7.2	3.8 / 9.5	4.5 / 11.2	5.1 / 12.9	5.5 / 13.8	2.9 / 7.2	3.8 / 9.5	4.5 / 11.2	5.1 / 12.9	5.5 / 13.8	5.5 / 13.8
	119	265	493	674	238	531	987	1349	1634	1880	796	1480	2023	2450	2821	3063	1973	2697	3267	3761	4084	4076
14	92	203	386	615	184	405	771	1230	-	-	608	1157	1845	-	-	-	1543	2460	-	-	-	-
	1.5 / 3 96	1.5 / 3.6 216	2.7 / 6.7 416	3.6 / 9.1 586	1.5 / 3 193	1.5 / 3.6 432	2.7 / 6.7 832	3.6 / 9.1 1173	4.4 / 11 1505	5.1 / 12.7 1730	1.5 / 3.6 649	2.7 / 6.7 1248	3.6 / 9.1 1759	4.4 / 11 2258	5.1 / 12.7 2595	5.5 / 13.8 2857	2.7 / 6.7 1664	3.6 / 9.1 2346	4.4 / 11 3011	3459	3809	5.5 / 13.8 3801
15	75	166	317	506	150	332	633	1013	1473	-	497	950	1519	2210	-	-	1266	2025	2946	-	-	- 3001
	1.5 / 3	1.5 / 3.2	2.4/6	3.4 / 8.5	1.5 / 3	1.5 / 3.2	2.4/6	3.4 / 8.5	4.3 / 10.9	5 / 12.5	1.5 / 3.2	2.4/6	3.4 / 8.5	4.3 / 10.9	5 / 12.5	5.5 / 13.8	2.4/6	3.4 / 8.5	4.3 / 10.9	5 / 12.5	5.5 / 13.8	5.5 / 13.8
	79	178	344	515	158	356	689	1029	1327	1601	535	1033	1544	1990	2402	2677	1377	2058	2653	3202	3569	3561
16	62	137	263	421	124	275	526	843	1230	-	412	788	1264	1845	-	-	1051	1686	2460	-	-	-
	1.5 / 3 65	1.5 / 3 148	2.1 / 5.3	3.2 / 7.9 455	1.5 / 3 131	1.5 / 3 297	2.1 / 5.3 576	3.2 / 7.9 910	4.1 / 10.2 1173	4.9 / 12.3 1468	1.5 / 3 445	2.1 / 5.3 864	3.2 / 7.9 1365	4.1 / 10.2 1760	2201	5.5 / 13.8 2517	2.1 / 5.3 1152	3.2 / 7.9 1820	4.1 / 10.2 2346	2935	3356	5.5 / 13.8 3348
17	52	115	220	354	104	230	441	709	1037	1443	345	661	1063	1555	2165	-	882	1418	2074	2886	-	-
"	1.5 / 3	1.5 / 3	1.9 / 4.8	3 / 7.5	1.5 / 3	1.5 / 3	1.9 / 4.8	3 / 7.5	3.9 / 9.6	4.8 / 12	1.5 / 3	1.9 / 4.8	3 / 7.5	3.9 / 9.6	4.8 / 12	5.5 / 13.8	1.9 / 4.8	3 / 7.5	3.9 / 9.6	4.8 / 12	5.5 / 13.8	5.5 / 13.8
	55	125	243	394	109	249	486	788	1045	1307	374	729	1182	1567	1961	2364	972	1576	2089	2614	3151	3160
18	44	97	187	301	87	194	373	602	882	1230	291	560	902	1322	1845	-	747	1203	1763	2460	-	-
	1.5 / 3 46	1.5 / 3	1.7 / 4.3	2.8 / 6.9	1.5 / 3 92	1.5 / 3 211	1.7 / 4.3 413	2.8 / 6.9 672	3.6 / 9.1 936	4.5 / 11.4 1171	1.5 / 3 317	1.7 / 4.3 620	2.8 / 6.9 1008	3.6 / 9.1 1404	4.5 / 11.4 1757	5.5 / 13.7 2147	1.7 / 4.3 827	2.8 / 6.9	3.6 / 9.1 1872	4.5 / 11.4 2342	2862	5.5 / 13.8 2991
19	37	83	160	257	74	166	319	515	756	1056	249	479	772	1133	1584	2130	638	1029	1511	2112	2839	-
	1.5 / 3	1.5 / 3	1.5 / 3.8	2.5 / 6.2	1.5 / 3	1.5 / 3	1.5 / 3.8	2.5 / 6.2	3.4 / 8.6	_	1.5 / 3	1.5 / 3.8	2.5 / 6.2		4.3 / 10.8	5.3 / 13.1	1.5 / 3.8	2.5 / 6.2	3.4 / 8.6	4.3 / 10.8		5.5 / 13.8
		90	177	289	78	180	354	577	843	1055	270	531	866	1265	1583	1934	708	1155	1686	2110	2579	2839
20		71	137	222	64	142	275	444	652	913	214	412	666	979	1370	1845	549	887	1305	1827	2460	-
		1.5 / 3 67	1.5 / 3.5	2.3 / 5.6	1.5 / 3 57	1.5 / 3	1.5 / 3.5	2.3 / 5.6	3.3 / 8.2 645	4.1 / 10.2 869	1.5 / 3 200	1.5 / 3.5 397	2.3 / 5.6 651	967	1303	5 / 12.5 1593	1.5 / 3.5 529	2.3 / 5.6 868	1289	4.1 / 10.2 1738	5 / 12.5 2124	5.5 / 13.8 2576
22		54	104	168	48	107	203	336	496	696	161	311	504	743	1044	1410	415	672	991	1392	1880	-
		1.5 / 3	1.5 / 3	1.9 / 4.7	1.5 / 3	1.5 / 3	1.5 / 3	1.9 / 4.7		3.7 / 9.3	1.5 / 3	1.5 / 3	1.9 / 4.7			4.5 / 11.3	1.5 / 3	1.9 / 4.7			_	5.5 / 13.8
		51	101	167	42	101	202	333	497	704	152	303	500	746	1056	1334	404	667	994	1408	1779	2357
24		42	80	130	37	83	161	261	385	542	125	241	391	578	813	1100	321	521	770	1083	1467	-
		1.5 / 3	1.5 / 3 79	1.6 / 4	1.5 / 3	1.5 / 3 78	1.5 / 3 157	1.6 / 4 261	390	3.3 / 8.3 555	1.5 / 3 116	1.5 / 3 236	1.6 / 4 391	2.3 / 5.9 585	3.3 / 8.3 832	4.2 / 10.4 1132	1.5 / 3 314	1.6 / 4 521	2.3 / 5.9 781	3.3 / 8.3 1109	4.2 / 10.4 1510	5.5 / 13.8 2139
26			63	103		65	127	206	305	430	98	190	309	457	645	874	254	412	610	859	1166	1963
			1.5 / 3	1.5 / 3.4		1.5 / 3	1.5 / 3	1.5 / 3.4	2/5	2.8 / 7.1	1.5 / 3	1.5 / 3	1.5 / 3.4	2/5	2.8 / 7.1	3.8 / 9.6	1.5 / 3	1.5 / 3.4	2/5	2.8 / 7.1		5.4 / 13.5
			62	103		60	124	207	311	443	91	186	310	466	665	910	248	413	622	887	1214	1837
28			51	83		53	102	166	245	346	79	153	249	368	520	706	204	331	491	693	941	1590
			1.5 / 3	1.5 / 3		1.5 / 3	1.5 / 3	1.5 / 3	1	2.5 / 6.2	1.5 / 3	1.5 / 3	1.5 / 3			3.3 / 8.4	1.5 / 3	1.5 / 3				
30			49 42	83 68		47	99 83	166 135	251 200	359 283	71 64	148 125	249	376 301	539 425	740 578	197 166	332 270	502 401	718 566	986 770	1594 1305
30			1.5 / 3			1.5 / 3	1.5 / 3			2.2 / 5.4		1.5 / 3				2.9 / 7.3						4.7 / 11.7
	-																					

- Total Load values are limited by shear, moment or deflection equal to L/180. Total Load values are
- the capacity of the beam in addition to its own weight.

 Live Load values are limited by deflection equal to L/240. Check the local building code for other deflection limits that may apply. Flat and low slope roofs may require more restrictive deflection limits, consult project's design professional of record.
- Where a Live Load value is not shown, the Total Load value will control.
- Table values represent the most restrictive of simple or multiple span applications. Span is measured center to center of the supports. Analyze multiple span beams with BC Calc® software if the length of any span is less than half the length of an adjacent span.

 Table values assume that lateral support is provided at each support and continuously along the
- top edge and applicable compression edges of the beam.

 Boise Cascade EWP ALLJOIST® Specifier Guide 09/30/2014 r 04/10/2020
- Table values for Minimum Required Bearing Lengths are based on the allowable compression design value perpendicular to grain for the beam and the Total Load value shown. Other design considerations, such as a weaker support material, may warrant longer bearing lengths. Table values assume that support is provided across the full width of the beam.
- Values assume that Support is provided across the run width of the Beath.

 For 2-ply, 3-ply or 4-ply beams; double, triple or quadruple Allowable Total Load and Allowable
 Live Load values. Minimum Required Bearing Lengths remain the same for any number of plies.

 134 inch members deeper than 14 inches are to be used as multiple-member beams only.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with BC Calc® software.

Versa-Lam® LVL 2.1E 3100

(125% Load Duration)

KEY TO TABLE:

Top Figure - Allowable Total Load [plf] Middle Figure - Allowable Live Load [plf]

Bottom Figure - Minimum Required Bearing Length at End / Intermediate Supports [inches]

					Bot	tom I	Figure	. - L	/linim	ium F	≀equii	red B	earınç	g Len	gth a	t End	/ Inte	erme	diate	Supp	orts [inche	sj
Part	SPAN	1¾" V	'ersa-La	am® 2.1E	3100	Doı						Tri						Quadrı					3100 or
	(π)	71/4"	91/2"	11%"	14"	71/4"	91/2"	11%"	14"	16"	18"	91/2"	11%"	14"	16"	18"	20"	11%"	14"	16"	18"	20"	24"
Part		954	1330	1782	2245	1908	2660	3564	4491	4796	4794	3990	5346	6736	7194	7191	7188	7128	8981	9592	9588	9584	9576
Page 1989	6	-	-	-					-	-	-	-		-	-	-	-	-	-	-	-	-	-
Page											_							_					
Mathematical Math	7		1097	1452	1807		2194		3613	4109	4107	3291		5420	6163	6160	6157		/226	821/	8213	8209	8201
14 15 15 15 15 15 15 15	′		2.9 / 7.3	3.9 / 9.7	4.8 / 12.1		2.9 / 7.3		4.8 / 12.1	5.5 / 13.8	5.5 / 13.8	2.9 / 7.3		4.8 / 12.1	5.5 / 13.8	5.5 / 13.8	5.5 / 13.8		4.8 / 12.1	5.5 / 13.8	5.5 / 13.8	5.5 / 13.8	5.5 / 13.8
May		-																					
Math	8	466	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
1	_			1059					2596	3083				3894	4624	4785	4782	4235	5192	6166	6380	6376	6368
14 15 15 15 15 15 15 15	9			37/91		_			- 45/112	5 3 / 13 3				45/112	53/133	55/138	55/138	- 37/91	- 45/112	5 3 / 13 3	55/138	5 5 / 13 8	- 55/138
14																							
148 158	10				-					-	-			-	-	-	-	-	-	-	-	-	-
14 15 15 15 15 15 15 15		1.5 / 3.1	2.7 / 6.6	3.6 / 8.9	4.4 / 10.9	1.5 / 3.1	2.7 / 6.6	3.6 / 8.9	4.4 / 10.9	5.2 / 12.9	5.5 / 13.8	2.7 / 6.6	3.6 / 8.9	4.4 / 10.9	5.2 / 12.9	5.5 / 13.8	5.5 / 13.8	3.6 / 8.9	4.4 / 10.9	5.2 / 12.9	5.5 / 13.8	5.5 / 13.8	5.5 / 13.8
18									2024	2384	2607			3037	3576	3910	3907	_	4049	4767	5213	5209	5201
189	11						-		-		-			- 40.407		-	-		-		-	-	-
14																		_					
15/3 15/3 15/4 34/6 42/105 15/3 15/4 34/6 42/105 15/3 15/13 15	12			_					1023	- 2141	2300			-	- 3211	3302	- 3373		- 3047	4201	4//0	4//2	-
14	'-		-		4.2 / 10.5				4.2 / 10.5	4.9 / 12.3	5.5 / 13.8			4.2 / 10.5	4.9 / 12.3	5.5 / 13.8	5.5 / 13.8		4.2 / 10.5	4.9 / 12.3	5.5 / 13.8	5.5 / 13.8	5.5 / 13.8
14 15/3 7/41 31/78 41/704 51/3 17/41 31/78 41/704 18/72 51/33		149	330	623	829	297	660	1247	1658	1942	2203	991	1870	2487	2913	3304	3301	2494	3316	3884	4406	4402	4394
14	13		_							-	-				-	-	-			-	-	-	-
14 92 203 336 615 184 405 71 1230 688 1157 1845 1843 2460										_													
15/3 15/3 15/3 15/3 15/4 15/3	1.1							-		1777	2044				2666	3066	3063			3554	4088	4084	4076
Part	14			_						48/12	55/138				48/12	55/138	55/138			48/12	- 5 5 / 13 8	5 5 / 13 8	55/138
15/73 15/73 24/6 37/92 47/118 54/736 55/738 24/6 37/92 47/118 54/736 55/738 24/6 37/92 47/118 54/736 55/738 54/736 55/738 55/																							
19 178 344 555 188 356 689 110 1443 1742 535 133 1665 2165 2613 2677 1377 220 2887 3484 3569 3561 1573 1573 1573 21753 34786 44711 547134 1573 21753 34786 44711 547134 1573 21753 34786 44711 547134 557138 21753 34786 44711 547134 557138 21753 34786 44711 547134 557138 21753 34786 44711 547134 557138 21753 34786 44711 547134	15	75	166	317	506	150	332	633	1013	1473	-	497	950	1519	2210	-	-	1266	2025	2946	-	-	-
16				_							-										_	_	_
1.5/3	40																					3569	3561
Part	16													-								- E E / 12 0	- E E / 12 O
15																							
18	17			_		_	-										_						-
18		1.5 / 3	1.5 / 3	1.9 / 4.8	3.1 / 7.7	1.5 / 3	1.5 / 3	1.9 / 4.8	3.1 / 7.7	4.2 / 10.5	5.2 / 13.1	1.5 / 3	1.9 / 4.8	3.1 / 7.7	4.2 / 10.5	5.2 / 13.1	5.5 / 13.8	1.9 / 4.8	3.1 / 7.7	4.2 / 10.5	5.2 / 13.1	5.5 / 13.8	5.5 / 13.8
15/3 15/3 17/43 28/69 15/3 15/3 17/43 28/69 15/3 15/3 17/43 28/69 4/99 49/123 15/3 17/43 28/69 4/99 49/123 15/3 17/43 28/69 4/99 4/9/123 15/3																	2376					3168	3160
Heat	18		-	-													-					-	-
19 37 83 160 257 74 166 319 515 756 1056 249 479 772 1133 1584 2130 638 1029 1511 2112 2839 15/3 15/3 15/3 15/3 25/62 15/3 15/3 15/3 25/62 36/91 47/117 15/3 15/3 25/62 36/91 47/117 55/138 15/38 15/38 15/38 25/62 36/91 47/117 55/138 15/38																							
15/3 15/3 15/3 15/3 25/62 15/3 15/3 15/3 25/62 15/3 15/3 15/3 15/3 25/62 36/91 47/117 15/3 15/3 25/62 36/91 47/117 55/138 55/138 25/62 36/91 25/62 33/83 44/111 54/318 55/138 25/62 36/91 25/62 33/83 44/111 54/318 55/138 25/62 36/91 25/62 33/83 44/111 54/318 55/138 25/62 36/91 25/62 33/83 44/111 54/318 55/138 25/62 36/91 25/62 33/83 44/111 54/318 55/138 25/62 36/91 25/62 33/83 44/111 54/318 55/138 25/62 36/91 25/62 33/83 44/111 54/318 55/138 25/62 36/91 25/62 33/83 44/111 54/318 55/138 25/62 36/91 25/62 33/83 45/112 55/138 25/62 33/83 45/112 55/1	19							-															2991
20 32 71 137 222 64 142 275 444 652 913 214 412 666 979 1370 1845 549 887 1305 1827 2460 -1	13												_										5.5 / 13.8
1.5/3		39	90	177	289	78	180	354	577	854	1149	270	531	866	1280	1723	2105	708	1155	1707	2297	2807	2839
22	20			-																			-
22		1.5 / 3																					
24	22																						
24 51 101 167 42 101 202 333 497 704 152 303 500 746 1056 1436 404 667 994 1408 1915 2357	22																	_					
24 42 80 130 37 83 161 261 385 542 125 241 391 578 813 1100 321 521 770 1083 1467 - 1.5/3 1.5/3 1.6/4 1.5/3																				1		1	
26 39 79 130 78 157 261 390 555 116 236 391 585 832 1135 314 521 781 1109 1513 2172 33 63 103 65 127 206 305 430 98 190 309 457 645 874 254 412 610 859 1166 1963 1.5/3 1.	24																						-
28 33 63 103 65 127 206 305 430 98 190 309 457 645 874 254 412 610 859 1166 1963						1.5 / 3				1												_	
28 1.5/3 1	20																						
28 62 103 60 124 207 311 443 91 186 310 466 665 910 248 413 622 887 1214 2001	26									_													
28 51 83 53 102 166 245 346 79 153 249 368 520 706 204 331 491 693 941 1590 1590 1590 1590 1590 1590 1590 159			1.5/5																				
30 1.5/3 1	28																	_					_
30 49 83 47 99 166 251 359 71 148 249 376 539 740 197 332 502 718 986 1691 42 68 43 83 135 200 283 64 125 203 301 425 578 166 270 401 566 770 1305 1.5/3 1.5																						_	
1.5/3 1.5/3 1.5/3 1.5/3 1.5/3 1.5/3 1.5/3.8 2.2/5.4 1.5/3 1.5/3 1.5/3 1.5/3 1.5/3 1.5/3 1.5/3 1.5/3 5/12.4				49	83				166	251								197		502			
	30										-												
Total Load values are limited by shear, moment or deflection equal to L/180. Total Load values are • Table values for Minimum Required Bearing Lengths are based on the allowable compression									'		'		·							*		*	

- Total Load values are limited by shear, moment or deflection equal to L/180. Total Load values are the capacity of the beam in addition to its own weight.

 Live Load values are limited by deflection equal to L/240. Check the local building code for other
- deflection limits that may apply. Flat and low slope roofs may require more restrictive deflection limits, consult project's design professional of record.
- Where a Live Load value is not shown, the Total Load value will control.
- Table values represent the most restrictive of simple or multiple span applications. Span is measured center to center of the supports. Analyze multiple span beams with BC Calc® software if the length of any span is less than half the length of an adjacent span.

 Table values assume that lateral support is provided at each support and continuously along the
- top edge and applicable compression edges of the beam.
- Table values for Minimum Required Bearing Lengths are based on the allowable compression design value perpendicular to grain for the beam and the Total Load value shown. Other design considerations, such as a weaker support material, may warrant longer bearing lengths. Table values assume that support is provided across the full width of the beam. For 2-ply, 3-ply or 4-ply beams; double, triple or quadruple Allowable Total Load and Allowable Live Load values. Minimum Required Bearing Lengths remain the same for any number of plies. 13/4 inch members deeper than 14 inches are to be used as multiple-member beams only.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with BC Calc® software.

Nailing Parallel to Glue Lines (Narrow Face)

	Close	est Al	lowa	ble N	lail S _l	oacin	g		
Versa-Lam	[®] LVL Products	Na	ailing Para	llel to Glue	e Lines (Na	irrow Face) ⁽¹⁾	to Ġlue	ling Idicular E Lines Face)
Na	il Size	Versa-L 1½	am® LVL ⁄2"	Versa-L 13		Versa-L 3½" &	am [®] LVL Wider	All Pro	oducts
		0.C. [in]	End [in]	O.C. [in]	End [in]	O.C. [in]	End [in]	0.C. [in]	End [in]
8d Box	(0.113"ø x 2.5")	3	11/2	2	1	2	1/2	2	1/2
8d Common	(0.131"ø x 2.5")	3	2	3	2	2	1	2	1
10d & 12d Box	(0.128"ø x 3", 3.25")	3	2	3	2	2	1	2	1
16d Box	(0.135"ø x 3.5")	3	2	3	2	2	1	2	1
10d & 12d Commo 16d Sinker	on & (0.148"ø x 3", 3.25")	4	3	4	3	2	2	2	2
16d Common	(0.162"ø x 3.5")	6	4	6	3	2	2	2	2

Nailing
Perpendicular to
Glue Lines
(Wide Face)

1) For 13/4" thickness and greater, 2 rows

of nails (such as for a metal strap) are allowed (use ½" minimum offset between rows and stagger nails).

- Offset and stagger nail rows from floor sheathing and wall sole plate.
- Simpson Strong-Tie A35 and LPT4 connectors may be attached to the side Versa-Lam[®] LVL/Versa-Rim[®] LVL.
 Use nails as specified by Simpson Strong-Tie.

Versa-Lam® LVL Design Values

Grade	Width [in]	Depth [in]	Weight [lb/ft]	Allowable Shear [lb]	Allowable Moment [ft-lb]	Moment of Inertia [in⁴]	Grade	Width [in]	Depth [in]	Weight [lb/ft]	Allowable Shear [lb]	Allowable Moment [ft-lb]	Moment of Inertia [in⁴]
Versa-Stud [®] 1.8E 2650		31/2	1.5	998	776	5.4			51/4	8.0	5,237	6,830	63.3
sa-Si	11/2	51/2	2.4	1,568	1,821	20.8			5½	8.4	5,486	7,457	72.8
Vers		71⁄4	3.2	2,066	3,069	47.6			71/4	11.0	7,232	12,566	166.7
		31/2	1.8	1,164	1,058	6.3			91/4	14.1	9,227	19,908	346.3
		51/2	2.8	1,829	2,486	24.3			9½	14.5	9,476	20,937	375.1
		71/4	3.7	2,411	4,189	55.6			111/4	17.1	11,222	28,814	622.9
		91/4	4.7	3,076	6,636	115.4		5¼	117/8	18.1	11,845	31,913	732.6
		91/2	4.8	3,159	6,979	125.0	0				-		
	13⁄4	111/4	5.7	3,741	9,605	207.6	310		14	21.3	13,965	43,552	1200.5
0		11%	6.0	3,948	10,638	244.2	1		16	24.4	15,960	56,046	1792.0
3100		14	7.1	4,655	14,517	400.2	L 2.		18	27.4	17,955	70,011	2551.5
2.1E		16	8.1	5,320	18,682	597.3	Versa-Lam® LVL 2.1E 3100		20	30.4	19,950	85,428	3500.0
		18	9.1	5,985	23,337	850.5	, E		24	36.5	23,940	120,549	6048.0
Versa-Lam® LVL		24	12.2	7,980	40,183	2016.0	-F		91/4	16.6	12,303	26.544	461.7
am.		5½	5.6	3,658	4,971	48.5	ersa					- , -	
a-L		71/4	7.4	4,821	8,377	111.1	>		9½	17.1	12,635	27,916	500.1
ers		91/4	9.4	6,151	13,272	230.8			111/4	20.2	14,963	38,419	830.6
>		91/2	9.6	6,318	13,958	250.1			11%	21.4	15,794	42,550	976.8
	3½	111/4	11.4	7,481	19,210	415.3		7	14	25.2	18,620	58,069	1600.7
	3/2	11%	12.1	7,897	21,275	488.4			16	28.8	21,280	74,728	2389.3
		14	14.2	9,310	29,035	800.3			18	32.4	23,940	93,348	3402.0
		16	16.2	10,640	37,364	1194.7							
		18	18.3	11,970	46,674	1701.0			20	36.0	26,600	113,904	4666.7
		20	20.3	13,300	56,952	2333.3			24	43.2	31,920	160,732	8064.0

Versa-Lam® LVL Allowable Stress Values

		Modulus of Elasticity True (Shear-Free)	Modulus of Elasticity Apparent	Modulus of Elasticity for Stability	Bending	Horizontal Shear	Tension Parallel to Grain	Compression Parallel to Grain	Compression Perpendicular to Grain	Equivalent Specific Gravity for Fastener Design
Design Property	Grade	E (x 10 ₆ psi) (1) (7)	E (x 10 ⁶ psi) ⁽¹⁾	E _{min} (x 10 ⁶ psi) (1) (8)	F _b (psi) (2)(3)	F _v (psi) (2)(4)	F _t (psi) (2)(5)	F _{cII} (psi) ⁽²⁾	F _c ⊥ (psi) ⁽¹⁾⁽⁶⁾	(SG)
Versa-Lam® LVL Beams	2.1E 3100	2.1	2.0	1.1	3100	285	1950	3000	750	0.5
Versa-Lam® LVL Studs	1.8E 2650	1.8	1.7	0.9	2650	285	1500	3000	750	0.5
Versa-Lam® LVL Columns	1.8E 2650	1.8	1.7	0.9	2650	285	1650	3000	750	0.5

- 1. This value cannot be adjusted for load duration.
- 2. This value is based upon a load duration of 100% and may be adjusted for other load durations.
- 3. Fiber stress bending value shall be multiplied by the depth factor, $(12/d)^{1/9}$ where d = member depth [in].
- 4. Stress applied perpendicular to the gluelines.
- 5. Tension value shall be multiplied by a length factor, (4/L)^{vs} where L = member length [ft]. Use L = 4 for members less than four feet long.
- ${\hbox{\bf 6. Stress applied parallel to the gluelines.}}\\$
- 7. True or shear-free modulus of elasticity does not account for shear deformation.
- 8. $E_{\rm min}$ is the reference modulus of elasticity for beam and column stability calculations. It is calculated using $E_{\rm apparent}$ in accordance with Appendix D of the 2018 NDS. When calculating $E_{\rm min}$, the coefficient of modulus of elasticity, ${\rm COV}_{\rm e}$, may be taken as 0.10, and the adjustment factor to convert E to a pure bending basis may be taken as 1.05.
- Design properties are limited to dry conditions of use where the maximum moisture content of the material will not exceed 16%.

Column							Allowal	ble Axial Lo	oad (lb)						
Length	3	3½" x 3½"			3½" x 4%"			3½" x 5¼"			3½" x 5½"			3½" x 7"	
[ft]	100%	115%	125%	100%	115%	125%	100%	115%	125%	100%	115%	125%	100%	115%	125%
4	14,700	16,090	16,930	18,390	20,130	21,180	22,070	24,165	25,430	23,130	25,320	26,640	29,450	32,240	33,920
5	12,270	13,150	13,660	15,350	16,440	17,090	18,425	19,740	20,515	19,300	20,680	21,490	24,580	26,330	27,365
6	10,080	10,650	10,980	12,610	13,320	13,740	15,140	15,995	16,495	15,860	16,750	17,280	20,195	21,335	22,000
7	8,310	8,705	8,930	10,400	10,890	11,170	12,480	13,075	13,415	13,080	13,700	14,050	16,650	17,435	17,890
8	6,930	7,205	7,370	8,660	9,010	9,210	10,405	10,825	11,070	10,900	11,340	11,600	13,880	14,440	14,760
9	5,840	6,050	6,160	7,300	7,560	7,710	8,770	9,080	9,260	9,190	9,510	9,700	11,700	12,115	12,350
10	4,980	5,135	5,225	6,230	6,420	6,540	7,480	7,715	7,850	7,830	8,080	8,220	9,975	10,290	10,470
11	4,290	4,410	4,480	5,360	5,520	5,600	6,445	6,625	6,730	6,750	6,940	7,050	8,595	8,835	8,975
12	3,730	3,825	3,880	4,660	4,780	4,850	5,600	5,745	5,830	5,870	6,020	6,100	7,475	7,665	7,775
13	3,270	3,350	3,390	4,090	4,190	4,240	4,915	5,030	5,095	5,150	5,270	5,340	6,555	6,710	6,795
14	2,890	2,950	2,990	3,610	3,690	3,740	4,340	4,435	4,490	4,550	4,650	4,700	5,790	5,915	5,990
Column Length	;	3½" x 7¼"	' x 7¼" 5¼" x 5¼" 15% 125% 100% 115%					5¼" x 5½"			5¼" x 7"			5¼" x 7¼"	
[ft]	100%	115%	125%	100%	115%	125%	100%	115%	125%	100%	115%	125%	100%	115%	125%
4	30,500	33,390	35,130												
5	25,460	27,270	28,340												
6	20,910	22,090	22,780	33,070	36,220	38,110	34,670	37,950	39,930						
7	17,250	18,060	18,530	29,420	31,730	33,085	30,830	33,240	34,660						
8	14,370	14,960	15,290	25,875	27,570	28,565	27,110	28,880	29,930	34,525	36,790	38,115	35,760	38,090	39,480
9	12,120	12,540	12,790	22,690	23,970	24,715	23,770	25,110	25,900	30,275	31,985	32,980	31,360	33,130	34,160
10	10,330	10,660	10,840	19,930	20,920	21,495	20,880	21,920	22,520	26,600	27,920	28,685	27,550	28,920	29,710
11	8,900	9,150	9,300	17,585	18,375	18,820	18,420	19,250	19,720	23,465	24,510	25,125	24,310	25,400	26,010
12	7,740	7,940	8,050	15,590	16,220	16,585	16,340	16,990	17,380	20,805	21,650	22,130	21,550	22,420	22,930
13	6,790	6,950	7,040	13,895	14,410	14,700	14,560	15,100	15,400	18,545	19,225	19,620	19,210	19,920	20,320
14	6,000	6,130	6,200	12,450	12,870	13,115	13,040	13,480	13,740	16,615	17,180	17,500	17,210	17,790	18,130
15				11,210	11,560	11,760	11,740	12,110	12,320	14,960	15,425	15,695	15,490	15,980	16,260
16				10,135	10,430	10,600	10,620	10,930	11,110	13,525	13,920	14,150	14,010	14,420	14,650
17				9,205	9,455	9,600	9,650	9,910	10,060	12,285	12,620	12,810	12,730	13,070	13,270
18				8,395	8,610	8,735	8,800	9,020	9,150	11,205	11,495	11,655	11,610	11,900	12,070
19				7,685	7,870	7,975	8,050	8,250	8,360	10,260	10,505	10,645	10,620	10,880	11,030
20				7,060	7,220	7,310	7,400	7,560	7,660	9,420	9,635	9,760	9,760	9,980	10,110
21				6,505	6,645	6,725	6,820	6,960	7,050	8,680	8,870	8,980	8,990	9,190	9,300
22															

- Table assumes that the column is braced at column ends only. Effective column length is equal to actual column length.
- 2) Allowable loads are based upon one-piece (solid) column members used in dry service conditions. BC Calc® sizing software (www.BCCalc.com) may be used for multi-piece column design.
- Allowable loads are based on an eccentricity value equal to 0.167 multiplied by either the column thickness or width (worst case).
- 4) Allowable loads are based on axial loaded columns using the design provisions of the 2018 National Design Specification (NDS) for Wood Construction. Table capacity values based upon a buckling length coefficient, $K_{\rm e}$, equal to 1.0
- (rotation free, translation fixed at each column end per NDS Appendix G). A $\rm K_{\rm e}$ coefficient of 1.0 conservatively models typical wood column applications. For other end fixity conditions, contact Boise Cascade EWP Engineering. For side or other combined bending and axial loads, see provisions in 2018 NDS.
- Load values are not shown for short lengths due to loads exceeding common connector capacities. Load values are not shown for longer lengths if the controlling slenderness ratio exceeds 50 (per NDS).
- 6) Lateral loads (wind loading) are not considered in this table. BC Calc® sizing software (www.BCCalc.com) may be used for out of plane lateral load column application design.

Versa-Stud® LVL 1.8E 2650

	R	Reference Desig	n Values		
Product	Bending F _b [psi]	Compression Parallel to Grain F_c [psi]	Compression Perp to Grain F _{c⊥} [psi]	Modulus of Elasticity - Apparent E [psi]	Horizontal Shear F _v [psi]
Versa-Stud® 1.8E 2650 11/2" x 51/2"	2865	3000	450	1,700,000	285
Spruce Pine Fir (North) # 1/2 Grade 2 x 6	1138	1150	425	1,400,000	135
Hem-Fir # 2 Grade 2 x 6	1105	1300	405	1,300,000	150
Western Woods # 2 Grade 2 x 6	878	900	335	1,000,000	135

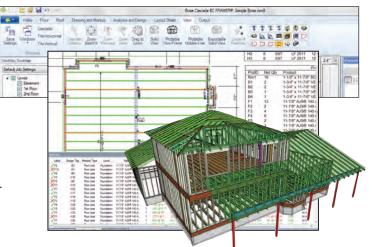
- Design values are for loads applied to the narrow face of the studs.
- Dimension lumber values per NDS Supplement, Design Values for Wood Construction, 2018 Edition.
- Repetitive member factors have not been applied to the bending values. Depth (size) factors per ICC-ES®/APA® ESR-1040 and 2018 NDS have been applied to the corresponding bending values.



BC Framer® helps customers create floor and roof framing layouts quickly. This easy-to-use computer-aided 3D drafting program frames layouts and creates piece and price reports. It also draws framing drawings that use Boise Cascade's engineered wood products (EWP) and develops schedules. BC Framer's editing and drawing tools allow flexibility when modifying framing layouts. You can also customize the layout drawing with framing details, notes, symbols, and accessories.

Information can also be obtained at 1-800-405-5969 or email us at EWPSupport@BC.com.

Technical Specs: This program is designed to work on standalone computers.



RECOMMENDED HARDWARE

- · CPU: 2.8GHz 6th Gen+ Core i7 or Xeon v4+
- L2 Cache: 3MB/Core
- RAM: 16GB
- Video: Full support for DirectX 9; Single monitor 512MB; Dual monitor 1GB (Resolution 1366×768 Minimum)
- Free Storage: 80GB (average 6,000-8,000 jobs)
- Operating Systems: Windows 10 (Pro or Enterprise Edition 64-bit)*, Windows 8.1 (Professional Edition 64-bit), Windows 7 (Professional Edition 64-bit),

*Apple Mac or Windows Emulator not supported

Actual specifications vary by user and will be assessed prior to installation.

BC Calc® Sizing Software

BC Calc® is now a web-based application available at www.bccalc.com and can be used on Windows or Apple operating systems via Internet Explorer, Edge, Chrome or Safari browsers as well as on iOS and Android tablets. An offline version is available to BC Connect® or registered BC Calc® users for use without an internet connection or in cases of limited connection availability. It can be downloaded once users have signed in to the application.

In addition to BCI® & AJS® Joists, Versa-Lam® LVL, and BOISE GLULAM®, BC Calc® also offers the analysis of solid sawn lumber and timber members. Thus BC Calc® is the only program needed to analyze structural wood members.



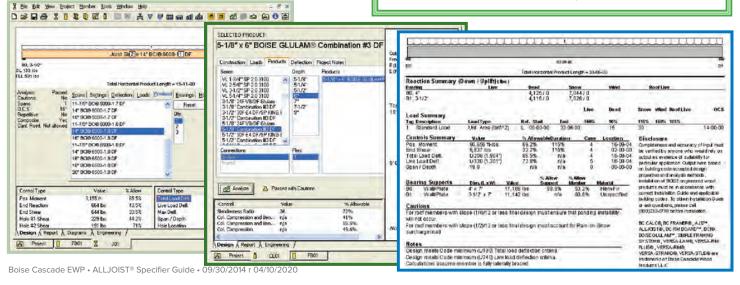


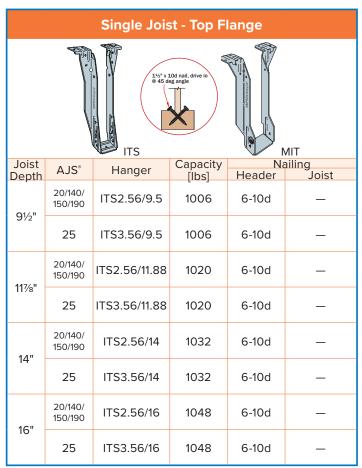
Boise Cascade has provided BC Calc[®] free of charge to the design community since 1994.

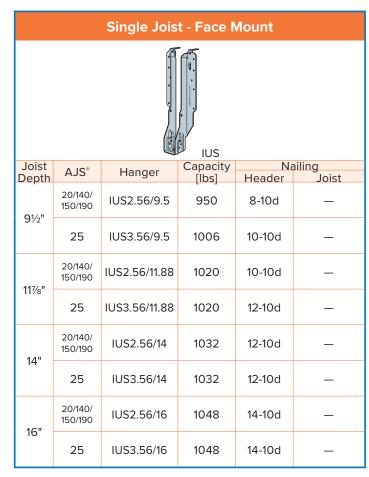
COMPUTER REQUIREMENTS

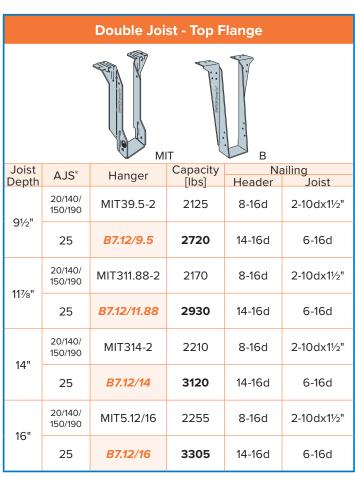
PC with any current version of MS Windows®, along with an internet connection. For questions regarding BC Calc®, call 1-800-405-5969 or email EWPSupport@BC.com.

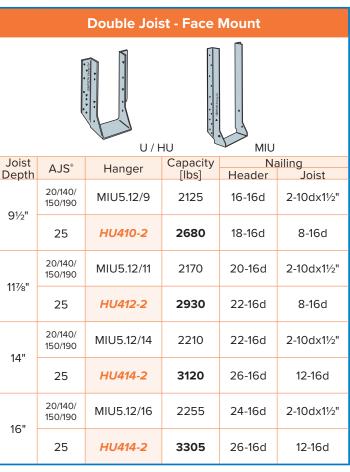
To Download BC CALC US, www.bccalc.com







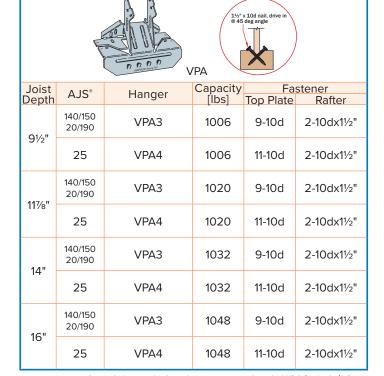




Face Mount Skewed 45° Joist Hanger Ď

		About to the state of the state	SUR/L					Activations (LSSU		
Joist	AJS*	Hanger	Capacity		ailing	Joist	AJS®	Hanger	Capacity	N	ailing
Depth	733	riariger	[lbs]	Header	Joist	Depth	733	rianger	[lbs]	Header	Joist
9½"	140/150 20/190	SUR/L2.56/9	1139	14-16d	2-10dx1½"	9½"	140/150 20/190	LSSUH310	1480	14-10d	12-10dx1½"
372	25	SUR/L410	1076	14-16d	6-16d	372	25	LSSU410	1480	14-10d	12-10dx1½"
11%"	140/150 20/190	SUR/L2.56/11	1174	16-16d	2-10dx1½"	117⁄8"	140/150 20/190	LSSUH310	1595	14-10d	12-10dx1½"
1178	25	SUR/L410	1101	14-16d	6-16d	1178	25	LSSU410	1595	14-10d	12-10dx1½"
14"	140/150 20/190	SUR/L2.56/14	1204	18-16d	2-10dx1½"	14"	140/150 20/190	LSSUH310	1600	14-10d	12-10dx1½"
14	25	SUR/L414	1123	18-16d	8-16d	14	25	LSSU410	1625	14-10d	12-10dx1½"
16"	140/150 20/190	SUR/L2.56/14	1235	18-16d	2-10dx1½"	16"	140/150 20/190	_	_	_	_
10	25	SUR/L414	1127	18-16d	8-16d	10	25	_	_	_	_
	Δ.	P							1-1-1-0-		
	Ac	djustable Heig	int Joist	Hangei			V	ariable Pitch .	Joist Co	nnector	

THAI Capacity Joist Nailing AJS[®] Hanger Depth [lbs] Header Joist 140/150 1330 2-10dx11/2" THAI322 6-10d 20/190 91/2" 25 THAI422 1330 6-10d 2-10dx11/2" 140/150 THAI322 1432 6-10d 2-10dx11/2" 20/190 111/8' 25 **THAI422** 1432 6-10d 2-10dx11/2" 140/150 **THAI322** 1525 6-10d 2-10dx11/2" 20/190 14" 2-10dx11/2" 1525 6-10d 25 **THAI422** 140/150 20/190 16" 25



Field Slope and Skew Joist Hanger

SIMPSON

Strong-Ti<u>e</u>

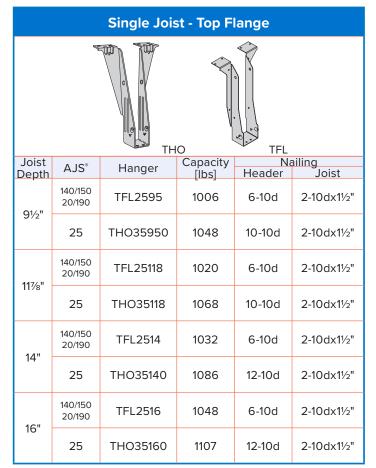
For more information call Simpson Strong-Tie at 1-800-999-5099 or visit their website at www.strongtie.com

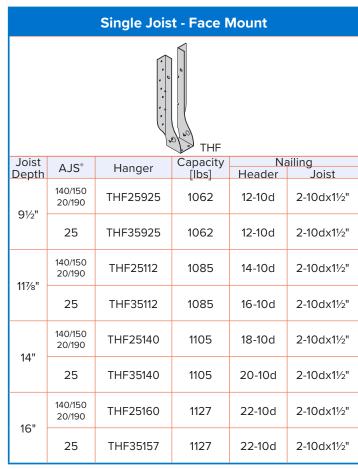
General Notes

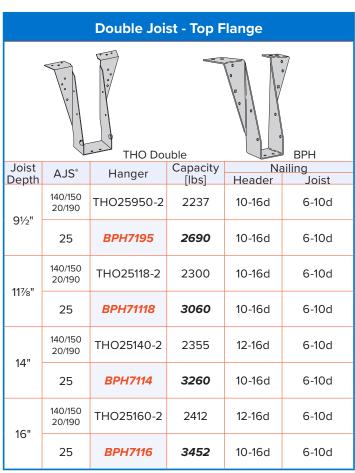
- **Bold Italic hangers require web stiffeners.**Capacities will vary with different nailing criteria and/or support conditions; contact supplier or Simpson Strong-Tie® for further information.
- Capacity values shown are either hanger capacity values (see support requirements below) or AJS® Joist end reaction capacities — whichever is less.
- All capacity values are downward loads at 100% load duration
- Use sloped seat hangers and beveled web stiffeners when AJS" Joist slope exceeds 1/4" per foot.
- Leave $^1\!/\!_{16}\!^{"}$ clearance (1/8" maximum) between the end of the supported joist and the head of the hanger.
- At max design capacity shown, hangers may exceed standard 1/8" deflection by 1/32".
- For proper installation of the VPA, the 2-10dx11/2" joist nails through the bend tabs must be installed at approximately a 45-degree angle.

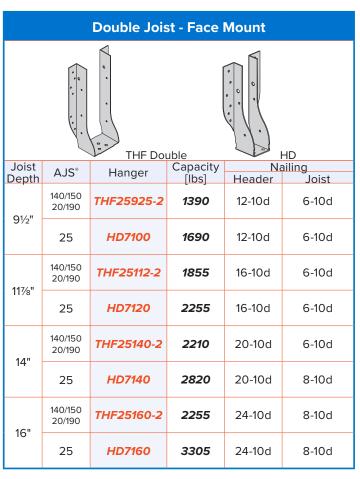
Support Requirements

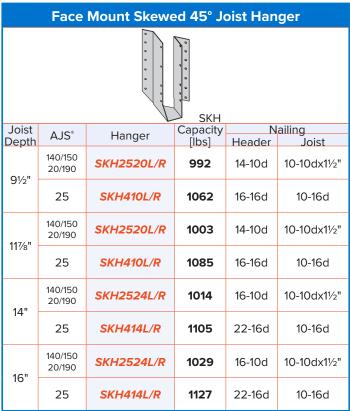
- Support material assumed to be Boise Cascade structural composite lumber or sawn lumber (Douglas fir or southern pine species).
- Minimum support width for single- and double-joist top mount hangers is 3".
- Minimum support width for face mount hangers with 10d and 16d nails is 13/4" and 2",





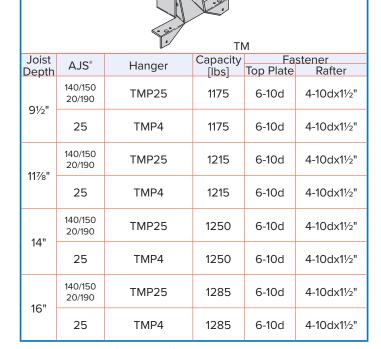






			SKH						LSSH		
Joist	AJS [®]	Hanger	Capacity		ailing	Joist	AJS®	Hanger	Capacity		ailing
Depth 9½"	140/150 20/190	SKH2520L/R	[lbs] 992	Header 14-10d	Joist 10-10dx1½"	Depth 9½"	140/150 20/190	LSSH25	[lbs] 1420	Header 14-16d	Joist 12-10dx1½"
9 72	25	SKH410L/R	1062	16-16d	10-16d	9 72	25	LSSH35	1420	14-16d	12-10dx1½"
111%"	140/150 20/190	SKH2520L/R	1003	14-10d	10-10dx1½"	111%"	140/150 20/190	LSSH25	1530	14-16d	12-10dx1½"
1178	25	SKH410L/R	1085	16-16d	10-16d	1178	25	LSSH35	1530	14-16d	12-10dx1½"
14"	140/150 20/190	SKH2524L/R	1014	16-10d	10-10dx1½"	14"	140/150 20/190	LSSH25	1630	14-16d	12-10dx1½"
14	25	SKH414L/R	1105	22-16d	10-16d	14	25	LSSH35	1630	14-16d	12-10dx1½"
16"	140/150 20/190	SKH2524L/R	1029	16-10d	10-10dx1½"	16"	140/150 20/190	LSSH35	1725	14-16d	12-10dx1½"
IU	25	SKH414L/R	1127	22-16d	10-16d	10	25	LSSH35	1725	14-16d	12-10dx1½"
	Ac	diustable Heio	aht Joist	Hange			V	ariable Pitch .	Joist Co	nnector	

MSH Joist Capacity Nailing AJS[®] Hanger Depth [lbs] Header Joist 140/150 **MSH322** 1270 16-10d 4-10dx11/2" 20/190 91/2" 25 MSH422IF 22-10d 1270 4-10d 140/150 **MSH322** 1367 16-10d 4-10dx11/2" 20/190 111/8" 25 MSH422IF 1367 22-10d 4-10d 140/150 **MSH322** 1455 4-10dx11/2" 16-10d 20/190 14" 4-10d 25 MSH422IF 1455 22-10d 140/150 **MSH322** 1413 16-10d 4-10dx11/2" 20/190 16" 25 MSH422IF 1413 22-10d 4-10d



Field Slope and Skew Joist Hanger

For more information. contact Mitek USP Structural Connectors at 1-800-328-5934 or MiTek-US.com

General Notes

- Shaded hangers required web stiffeners at joist ends. Web stiffeners may be equired for non-shaded hangers by Boise Cascade
- Capacities will vary with different nailing criteria and/or support conditions: contact supplier or USP Structural Connectors for further information.
- Capacity values shown are either hanger capacity values (see support requirements below) or AJS® Joist end reaction capacities - whichever is
- All capacity values are downward loads at 100% load duration.
- Use sloped seat hangers and beveled web stiffeners when AJS* Joist slope exceeds ¼" per foot.
- Leave 1/16" clearance (1/8" maximum) between the end of the supported joist and the head of the hanger.
- For AJS® Joist applications, consult Mitek USP for capacity reduction.

Support Requirements

- Support material assumed to be Boise Cascade structural composite *lumber or sawn lumber (Douglas fir or southern pine species).*Minimum support width for single- and double-joist top mount hangers is
- 3": (11/2" for THO hangers).
- Minimum support width for face mount hangers with 10d and 16d nails is 134" and 2", respectively.

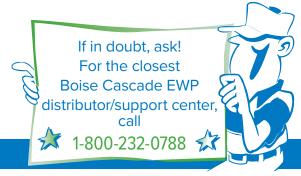
FASTER. STRONGER. EASIER.



Lifetime Guaranteed Quality and Performance

Boise Cascade warrants its BCI® Joist, Versa-Lam® LVL and ALLJOIST® products to comply with our specifications, to be free from defects in material and workmanship, and to meet or exceed our performance specifications for the normal and expected life of the structure when correctly stored, installed and used according to our Installation Guide.

The information in this document pertains to use in the UNITED STATES ONLY, Allowable Stress Design. Refer to the ALLJOIST Specifier Guide Canada for use in Canada, Limit States Design.



For information about

Boise Cascade's Engineered Wood Products, visit our website at

www.BC.com/ewp

Your Dealer is:



If no dealer is listed, call 1-800-232-0788