## ALLJOIST® COMMERCIAL GUIDE

for ALLJOIST® products manufactured in St. Jacques, New Brunswick Canada \& VERSA-LAM ${ }^{\circledR}$ products manufactured in Alexandria [Lena], Louisiana



Boise Cascade Engineered Wood Products for Light Commercial Applications

Boise Cascade Engineered Wood Products are readily available through experienced distributors and retailers who provide valuable assistance in selecting and specifying the most economical joist and beam system. By specifying the products shown in the guide, time delays associated with so-called custom products from other manufacturers are minimized. While those products may be structurally adequate, they are only available on a "build to order" basis. That means your clients have automatically just added a minimum of 2-3 weeks to their production cycle: an eternity in today's tight construction schedules. Add that same amount of time for a single mis-cut joist and you can certainly see how quickly a well-intended specification can turn into a logistical nightmare.

This design guide is intended to provide information for the preliminary sizing of Boise Cascade Engineered Wood Products. While the products and applications shown in this guide are similar to our residential applications guide, the building code provisions and design requirements of the light commercial projects are significantly different. Concentrated floor load provisions, partition loads, wind and seismic provisions, and fire protection systems are all examples of these differences. This guide helps identify these issues and provides corresponding product application details. Professional guidelines in most areas require the project's design professional of record to be responsible for the overall design
of light commercial structures. This guide is intended to assist the design professional in specifying engineered wood products in dryuse condition of such structures.

ALLJOIST® ${ }^{\circledR}$ and $B C I{ }^{\circledR}$ joists and VERSALAM ${ }^{\circledR}$ beams are manufactured per corresponding International Code Council building code evaluation reports. Testing and quality control is certified by an independent inspection agency. Boise Cascade Engineered Wood Products are warranted for the life of the structure (see back cover of this guide). For the location of the nearest Boise Cascade representative or supplier of Boise Cascade Engineered Wood Products, please call 800-232-0788.

For complete information on fire resistance detailing and design with the entire Boise Cascade EWP product line, please refer to the US Fire Design \& Installation Guide, located at www.bcewp.com.


# ALLJOIST® Product Architectural Specifications 

EVALUATION SUBJECT: AJS® Series Prefabricated Wood I-Joists
1.0 Evaluation Scope:

Compliance with the following codes:

- International Building Code ${ }^{\circledR}$ (IBC)
- International Residential Code ${ }^{\circledR}$ (IRC) Properties Evaluated: Structural. 2.0 Uses: The AJS® Joists are prefabricated wood I-joists used as floor joists, roof rafters and blocking panels, to support code-required loads. Prefabricated wood I-joists described in this report comply with Section 2303.1.2 of the IBC and Section R502.1.4 of the IRC, for allowable stress design.
3.0 Description:
3.1 General: The AJS® Series prefabricated wood I-joists have solid-sawn lumber or composite lumber flanges and oriented strand board (OSB) webs. The top and bottom flanges are parallel, creating constantdepth joists. The web-to-web joints of the I-joists are square butt joints and conform to the specifications in the approved quality control manuals. The web-toflange connection is a proprietary grooved connection, also conforming to the approved quality control manuals. The I-joists are available in various lengths and depths. See ESR-1144 Table 1 for full description of the AJS® 1 -Joists.
3.2 Material Specifications:
3.2.1 Flanges: The flanges of the I-joists are sawn lumber or composite lumber conforming to the specifications in the approved quality control manuals. The lumber flanges are $11 / 2$ inch $x$ either $21 / 2$ inch or $31 / 2$ inch $(38 \mathrm{~mm} x$ either 64 mm or

89 mm ) spruce-pine-fir (SPF) and are used interchangeably with any of the sawn lumber flanges of the same dimensions. The sawn lumber flange material, grade, width and depth are noted in ESR-1144, Table 1.
3.2.2 Web: Web material for the I-Joists is $3 / 8$-inchthick ( 10 mm ) or $7 / 16$-inch-thick ( 11 mm ) OSB conforming to Exposure 1 requirements of DOC PS-2, with further requirements set forth in the approved quality control manuals and manufacturing standards.
3.2.3 Adhesive: Adhesives used in the fabrication of the I-joists are exterior-type, heat durable adhesives complying with ASTM D 2559 and ASTM D 5055, and are specified in the quality control manuals and the manufacturing standards.
4.0 Design and Installation: Design of the prefabricated wood I-joists described in this report shall be in accordance with the applicable code. Additionally, the design and installation of the prefabricated wood I-joists shall comply with Sections 4.1 through 4.12 listed in ESR-1144 which include 4.1 Allowable Structural Capacity, 4.2 Fasteners, 4.3 Web Stiffeners, 4.4 Lateral Support, 4.5 Holes in I-Joist Web, 4.6 Duration of Load, 4.7 In -Service Moisture Conditions, 4.8 RepetetiveMember Use, 4.9 Member Spans, 4.10 Deflection, 4.11 Blocking Panels, \& 4.12 Cantilevered Joists, and the manufacturer's installation instructions.
5.0 Conditions of Use: The AJS® Series I-joists described in this report comply with, or are suitable alternatives to what is specified in, those codes listed under ESR-1144, Section 1.0 Evaluation Scope of these specifications, subject to the following conditions:
5.1 AJS® joists must be installed in accordance with this report and the manufacturer's installation instructions.
5.2 Drawings and design details verifying compliance with this report must be submitted to the code official when requested. The drawings and calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.
5.3 Flanges of the I-joist may not be cut or notched, unless an engineered design prepared by a registered design professional is submitted to the code official for approval.
5.4 The AJS® joists are manufactured by Boise Cascade Wood Products, L.L.C. at their plant in St. Jacques, New Brunswick, Canada under an approved quality control program with inspections by APA - The Engineered Wood Association (AA-649).
6.0 Evidence Submitted:

Data in accordance with the ICC-ES Acceptance Criteria for Prefabricated Wood I-joists (AC14), dated February 2013.
7.0 Identification:

AJS® I-joists are identified by a stamp indicating the joist model; company name (Boise Cascade Wood Products, L.L.C.); manufacturing location; evaluation report number (ESR-1144); and the name and logo of the inspection agency (APA).
AJS® Joists in Commercial Projects: The 18" and deeper depth AJS® joists are intended for commercial projects with heavier design loads and longer spans. All commercial projects utilizing AJS® joists shall have an engineer or architect of record.

Boise Cascade Engineered Wood Products

## 40 PSF Live Load Span Tables

Multi-Family Dwellings and Hotels: Private Rooms No Web Stiffeners Required for Joist Depths of 16" and Less. Web Stiffeners Required at All Bearing Locations for 18" and Deeper Joists

40 PSF Live Load + 25 PSF Dead Load

| Joist Depth | Joist Series | 3½" Interior Bearing |  |  |  | 5¼" Interior Bearing |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 12" о.c. | 16" o.c. | 19.2" o.c. | 24" о.c. | 12" о.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| 91⁄2" | AJS® 20 | 19'-1" | 17'-5" | 16'-0" | 14'-4" | 19'-1" | 17'-5" | 16'-0" | 14'-4" |
|  | AJS® 190 | 19'-4" | 17'-8" | 16'-8" | 14'-4" | 19'-4" | 17'-8" | 16'-8" | 14'-4" |
|  | AJS® 25 | 21'-0" | 19'-1" | 18'-0" | 15'-9" | 21'-0" | 19'-1" | 18'-0" | 15'-9" |
| 117/8" | AJS ${ }^{\text {® }} 20$ | 22'-8" | 20'-0" | $18^{\prime}-3 "$ | 14'-7" | 22'-8" | 20'-0" | 18'-3" | 14'-7" |
|  | AJS® 190 | 23'-0" | 21'-0" | 18'-3" | 14'-7" | 23'-0" | 21'-0" | 18'-3" | 14'-7" |
|  | AJS® 25 | 24'-11" | 22'-9" | 20'-1" | 16'-1" | 24'-11" | 22'-9" | 20'-1" | 16'-1" |
| 14" | AJS ${ }^{\text {® }} 20$ | 25'-5" | 22'-0" | 18'-7" | 14'-10" | 25'-5" | 22'-0" | 18'-7" | 14'-10" |
|  | AJS® 190 | $26^{\prime}-1{ }^{\prime \prime}$ | 22'-4" | 18'-7" | 14'-10" | 26'-1" | 22'-4" | 18'-7" | 14'-10" |
|  | AJS® 25 | 28'-4" | 24'-6" | 20'-5" | 16'-4" | 28'-4" | 24'-6" | 20'-5" | 16'-4" |
| 16" | AJS ${ }^{\text {® }} 20$ | 27'-4" | 22'-8" | 18'-10" | 15'-1" | 27'-4" | 22'-8" | 18'-10" | 15'-1" |
|  | AJS® 190 | 28'-11" | 22'-8" | 18'-10" | 15'-1" | 28'-11" | 22'-8" | 18'-10" | 15'-1" |
|  | AJS® 25 | 31'-4" | 25'-0" | 20'-10" | 16'-7" | 31'-4" | 25'-0" | 20'-10" | 16'-7" |
| 18" | AJS® 25 | 34'-6" | $31^{\prime}-5 "$ | 28'-11" | 25'-10" | 34'-6" | $31^{\prime}-5 "$ | 28'-11" | 25'-10" |
|  | AJS ${ }^{\circledR} 30$ | 35'-6" | 32'-4" | 30'-5" | 28'-4" | 35'-6" | 32'-4" | 30'-5" | 28'-4" |
| $20 "$ | AJS® 25 | 37'-4" | $33^{\prime}-6 "$ | 30'-7" | 27'-4" | 37'-4" | 33'-6" | 30'-7" | 27'-4" |
|  | AJS® 30 | 38'-5" | 35'-0" | 33'-0" | 30'-8" | 38'-5" | 35'-0" | 33'-0" | 30'-8' |
| 22 | AJS ${ }^{\circledR} 25$ | 40'-1" | 35'-1" | 32'-1" | 28'-8" | 40'-1" | 35'-1" | 32'-1" | 28'-8" |
|  | AJS® 30 | 41'-3" | 37'-7" | 35'-5" | 32'-1" | 41'-3' | 37'-7" | $35^{\prime}-5{ }^{\prime \prime}$ | 32'-3" |
| 24 " | AJS® 25 | 42'-4" | 36'-7" | 33'-5" | 29'-11" | 42'-4" | 36'-7" | $33^{\prime \prime}-5$ | 29'-11" |
|  | AJS® 30 | 44'-1" | 40'-2" | 37'-7" | 32'-9" | 44'-1" | 40'-2" | $37^{\prime}-7{ }^{\prime \prime}$ | $33 '-8{ }^{\prime \prime}$ |

NOTES

- Loading based upon Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/480.
- Table values assume that $23 / 32$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table assumes minimum $21 / 4^{\prime \prime}$ end bearing, no web stiffeners required for joist depths of 16 "and less, web stiffeners required at all bearing locations for 18 " and deeper joists.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® sizing software.


## Medical/Dental: Private Rooms

Web Stiffeners Required at All Bearing Locations for Table Below
Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BC CALC® sizing software.

| [Worst Case 40 PSF or 1000 LB Concentrated Live Load] + 25 PSF Dead Load |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Joist <br> Depth | Joist Series | 3½" Interior Bearing / 5¼" Interior Bearing |  |  |  |
|  |  | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| 9½" | AJS® 20 | 17'-7" | 16'-1" | 15'-0" | 12'-9" |
|  | AJS® 190 | 17'-10" | 16'-4" | 15'-4" | 14'-1" |
|  | AJS® 25 | 19'-4" | 17'-8" | 16'-7" | 15'-5" |
| 117/8" | AJS® 20 | 20'-11" | 19'-1" | 18'-0" | 15'-4" |
|  | AJS® 190 | 21'-3" | 19'-5" | 18'-4" | 17'-0" |
|  | AJS® 25 | 23'-1" | 21'-0" | 19'-9" | 18'-5" |
| 14" | AJS® 20 | 23'-9" | 21'-8" | 20'-1" | 17'-7" |
|  | AJS® 190 | 24'-2" | 22'-0" | 20'-9" | 19'-2" |
|  | AJS® 25 | 26'-2" | 23'-10" | 22'-5" | 20'-10" |
| 16" | AJS® 20 | 26'-4" | 23'-8" | 21'-7" | 19'-4" |
|  | AJS® 190 | 26'-9" | 24'-5" | 23'-0" | 20'-8" |
|  | AJS® 25 | 29'-0" | 26'-4" | 24'-10" | 23'-1" |
| 18" | AJS® 25 | 31'-11" | 29'-1" | 27'-5" | 25'-6" |
|  | AJS® 30 | 32'-10" | 29'-10" | 28'-2" | 26'-2" |
| $20 "$ | AJS® 25 | 34'-6" | 31'-6" | 29'-8" | 27'-4" |
|  | AJS® 30 | 35'-6" | 32'-4" | 30'-6" | 28'-4" |
| $22 "$ | AJS® 25 | 37'-1" | 33'-10" | 31'-11" | 28'-8" |
|  | AJS® 30 | 38'-2" | 34'-9" | 32'-9" | 30'-5" |
| 24" | AJS® 25 | 39'-7" | 36'-1" | 33'-5" | 29'-11" |
|  | AJS® 30 | 40'-10" | 37'-2" | 35'-0" | 32'-7" |

## NOTES

- Loading based upon Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of $\mathrm{L} / 240$. Live load deflection limited to L/480 with 40 psf .
- Table values assume that $23 / 32$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4^{\prime \prime}$ end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® ${ }^{\text {® }}$ sizing software.


## Concentrated Live Loads ("Safe" Loads)

The International Building Code requires certain types of commercial floors to be designed with a concentrated live load, in addition to the prescribed uniform loads. This concentrated live load is applied to the floor without the uniform live load but with all dead load, creating a separate live load analysis. The concentrated load is either 2000 lb or 1000 lb , depending upon the floor type, and is applied to a $21 / 2$ foot $\times 21 / 2$ foot square area. This $21 / 2$-foot square is applied on the floor such that would result in the highest shear and bending stress, and deflections. Since this load is intended to create maximum stress and deflection conditions, it is centered over a specific floor joist. Floor joists at 24 " oncenter will receive more load from the concentrated live load than joists spaced at smaller increments. For example, a joist at 24"
on-center will receive $2 / 2.5$ or $80 \%$ of the concentrated load over a distance of 2 ' -6 " along the joist length.

There have been some misconceptions in the design community on how the concentrated live load should be applied to floors. Boise Cascade EWP Engineering has consulted directly with ICC on this matter. The loads prescribed in section 1607.4 of the 2009/2012 IBC have been properly considered in the development of the relative floor joist tables in this guide.

## 6 <br> 50 PSF Live, 15 PSF Partition \& 2000 lb Conc. Load Span Tables

[Worst Case 50 PSF or 1000 LB Concentrated Live Load]

+ 15 PSF Partition Load + 25 PSF Dead Load
Office Buildings: Office Area


## Web Stiffeners Required at All Bearing Locations for Table Below

Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BC CALC® sizing software.

| [Worst Case 50 PSF or 1000 LB Concentrated Live Load] + 15 PSF Partition Load + 25 PSF Dead Load |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Joist Depth | Joist Series | 3½" Interior Bearing |  |  |  | 5¼" Interior Bearing |  |  |  |
|  |  | 12 " o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| 91⁄2" | AJS® 20 | 14'-7" | 11'-7" | 10'-0" | 4'-11" | 14'-7" | 11'-7" | 10'-0" | 4'-11" |
|  | AJS® 190 | 16'-3' | 12'-11" | 10'-2" | 4'-11" | 16'-3' | 12'-11" | 10'-2" | 4'-11" |
|  | AJS® 25 | 17'-11" | 16'-3" | 10'-2" | 4'-11" | 17'-11" | 16'-3" | 10'-2" | 4'-11" |
| 117/8" | AJS® 20 | 17'-10" | 14'-3" | 12'-4" | 6'-1" | 17'-10" | 14'-3" | 12'-4" | 6'-1" |
|  | AJS® 190 | 19'-8" | 15'-10" | 13'-5" | 6'-1" | 19'-8" | 15'-10" | 13'-5" | 6'-1" |
|  | AJS® 25 | 21'-4" | 19'-4" | $13^{\prime}-5^{\prime \prime}$ | 6'-1" | 21-4" | 19'-4" | $13^{\prime}-5^{\prime \prime}$ | 6'-1" |
| $14 "$ | AJS® 20 | 20'-8" | 16'-6" | 14'-3" | 7'-5" | 20'-8" | 16'-6" | 14'-3" | 7'-5" |
|  | AJS® 190 | 22'-4" | 18'-4" | 15'-11" | 7'-5" | 22'-4" | 18'-4" | 15'-11" | 7'-5" |
|  | AJS® 25 | 24'-2" | 22'-0" | 16'-10" | 7'-5" | 24'-2" | 22'-0" | 16'-10" | 7'-5" |
| $16 "$ | AJS® 20 | 23'-2" | 18'-6" | 16'-0" | 9'-1" | 23'-2" | 18'-6" | 16'-0" | 9'-1" |
|  | AJS® 190 | 24'-9" | 20'-7" | 17'-10" | 9'-1" | 24'-9" | 20'-7" | 17'-10" | 9'-1" |
|  | AJS® 25 | 26'-9" | 24'-4" | 20'-5" | 9'-1" | 26'-9" | 24'-4" | 20'-5" | 9'-1" |
| 18" | AJS® 25 | 29'-6" | 26'-10" | 24'-7" | 20'-10" | 29'-6" | 26'-10" | 24'-7" | 20'-10" |
|  | AJS ${ }^{\text {® }} 30$ | $30^{\prime}-4{ }^{\prime \prime}$ | 27'-7" | 26'-0" | 20'-10" | 30'-4" | 27'-7" | 26'-0" | 20'-10" |
| $20 "$ | AJS® 25 | $31^{\prime}-11^{\prime \prime}$ | 28'-6" | 26'-0" | 22'-7" | 31'-11" | 28'-6" | 26'-0" | 22'-7" |
|  | AJS® 30 | 32'-10" | 29'-11" | 28'-2" | 22'-7" | 32'-10" | 29'-11" | 28'-2" | 22'-7" |
| 22 " | AJS® 25 | $34{ }^{\prime}-4{ }^{\prime \prime}$ | 29'-10" | 27'-3' | 23'-1" | $34{ }^{\prime}-4{ }^{\prime \prime}$ | 29'-10" | 27'-3" | 24'-4" |
|  | AJS® 30 | $35^{\prime}-4 "$ | 32'-1" | 28'-11" | 23'-1" | $35^{\prime}-4 "$ | 32'-1" | 30'-3" | 24'-4" |
| $24 "$ | AJS® 25 | 35'-11" | 31'-1" | 28'-5" | 23'-8" | 35'-11" | 31'-1" | 28'-5" | 25'-4" |
|  | AJS® 30 | 37'-9" | $34{ }^{\prime}-4{ }^{\prime \prime}$ | 29'-7" | 23'-8" | 37'-9" | $34{ }^{\prime}-4{ }^{\prime \prime}$ | 31'-11" | 26'-1" |

## NOTES

- Loading based upon Table 1607.1 of 2009/2012 International Building Code
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/600 with 50 psf , L/360 with $50+15 \mathrm{psf}$.
- Table values assume that $23 / 32$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4$ " end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® sizing software.


## Deflection Criteria for Commercial Floors

The minimum deflection criterion for commercial floors is the same as for residential floors, L/360 for live load deflection and L/240 for total load deflection. Since deflection controls a significant amount of I-joist designs, an additional deflection limit has been considered in the development of the AJS® joist span tables shown in this guide. In addition to the minimum limits at full uniform and concentrated ("safe") live loads, deflection is limited to L/600 with a uniform live load of 50 psf in those tables where the code specified uniform live load is 50 psf or greater. As is the case with all joist designs, floor performance should always be considered by the design professional of record in each specific condition.

## 60 / 75 PSF Live \& 1000 lb Conc. Load Span Tables

## Medical/Dental: Operating Rooms \& Laboratory Library: Reading Rooms

| [Worst Case 60 PSF or 1000 LB Concentrated Live Load] + 25 PSF Dead Load |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Joist <br> Depth | Joist <br> Series | $31122^{\prime \prime}$ Interior Bearing |  |  |  | 51⁄4" Interior Bearing |  |  |  |
|  |  | 12 " | 16" | 19.2" | $24 "$ | 12" | 16" | 19.2" | 24" |
|  |  | o.c. | o.c. | o.c. | o.c. | o.c. | o.c. | o.c. | o.c. |
| 91⁄2" | AJS ${ }^{\text {® }} 20$ | 16'-3" | 14'-10" | 14'-0" | 11'-5" | 16'-3" | 14'-10" | 14'-0" | 11'-5" |
|  | AJS® 190 | 16'-6" | 15'-0" | 14'-2" | 11'-5" | 16'-6" | 15'-0" | 14'-2" | 11-5" |
|  | AJS® 25 | 17'-11" | 16'-3" | 15'-4" | 13'-3" | 17'-11" | 16'-3" | 15'-4" | 13'-3" |
| 117/8" | AJS® 20 | 19'-4" | 17'-6" | 16'-0" | 13'-1" | 19'-4" | 17'-6" | $16^{\prime}-0{ }^{\prime \prime}$ | 13'-1" |
|  | AJS® 190 | 19'-8" | 17'-11" | 16'-4" | 13'-1" | 19'-8" | 17'-11" | 16'-4" | 13'-1" |
|  | AJS® 25 | 21'-4" | 19'-4" | 18'-3" | 14'-11" | 21-4" | 19'-4" | 18'-3" | 14'-11" |
| $14 "$ | AJS® 20 | 22'-0" | 19'-2" | 17'-6" | 14'-7" | 22'-0" | 19-2" | 17'-6" | 14'-7" |
|  | AJS® 190 | 22'-4" | 20'-4" | 18'-3" | 14'-7" | 22-4" | 20'-4" | 18'-3" | 14'-7" |
|  | AJS® 25 | 24'-2" | 22'-0" | 20'-6" | 16'-4" | 24'-2" | 22'-0" | 20'-6" | 16'-4" |
| 16" | AJS® 20 | 23'-11" | 20'-8" | 18'-11" | 16'-0" | 23'-11" | 20'-8" | 18'-11" | 16'-0" |
|  | AJS® 190 | 24'-9" | 22'-2" | 20'-1" | 16'-0" | 24'-9" | 22'-2" | 20'-1" | 16'-0" |
|  | AJS® 25 | 26'-9" | 24'-4" | 22'-3" | 17'-9" | 26'-9" | 24'-4" | 22'-3" | 17'-9" |
| 18" | AJS® 25 | 29'-6" | 26'-10" | 25'-3" | 22'-1" | 29'6" | 26'-10" | 25'-3" | 22'1" |
|  | AJS® 30 | 30'-4" | 27'-7" | 26'-0" | 22'-1" | 30'-4" | 27'-7" | 26'-0" | 22'-1" |
| 20 | AJS® 25 | 31'-11" | 29'-1" | 26'-9" | 23'-11" | 31'-11" | 29'-1" | 26'-9" | 23'-11" |
|  | AJS® 30 | 32'-10" | 29'-11" | 28'-2" | 23'-11" | 32'-10" | 29'-11" | 28'-2" | 23'-11" |
| 22" | AJS® 25 | 34'-4" | 30'-8" | 28'-0" | 24'-6" | 34'-4" | 30'-8" | 28'-0" | 25'-0" |
|  | AJS® 30 | 35'-4" | 32'-1" | 30'-3" | 24'-6" | 35'-4" | 32'-1" | 30'-3" | 25'-9" |
| $24 "$ | AJS® 25 | 36'-8" | 32'-0" | 29'-2" | 25'-0" | 36'-8" | 32'-0" | 29'-2" | 26'-1" |
|  | AJS® 30 | 37'-9" | 34'-4" | 31'-4" | 25'-0" | 37'-9" | $34^{\prime}-4{ }^{\prime \prime}$ | 32'-4" | 27'-8" |

## Retail: Upper Floors

[Worst Case 75 PSF or 1000 LB Concentrated Live Load] + 25 PSF Dead Load

| Joist Depth | Joist Series | 3½" Interior Bearing |  |  |  | 5¼" Interior Bearing |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{aligned} & 12 \text { " } \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & 16 " \\ & \text { o.c. } \end{aligned}$ | $\begin{gathered} \text { 19.2" } \\ \text { o.c. } \end{gathered}$ | $\begin{aligned} & 24 " \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & 12 " \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & 16 " \\ & \text { o.c. } \end{aligned}$ | $\begin{gathered} \text { 19.2" } \\ \text { o.c. } \end{gathered}$ | $\begin{aligned} & 24 " \\ & \text { o.c. } \end{aligned}$ |
| 91⁄2" | AJS® 20 | 16'-3' | 14'-2" | 12'-2" | 9'-8" | 16'-3" | 14'-2" | 12'-2" | 9'-8" |
|  | AJS® 190 | 16'-6" | 14'-7" | 12'-2" | 9'-8' | 16'-6" | 14'-7" | 12'-2" | 9'-8" |
|  | AJS® 25 | 17'-11" | 16'-3" | 14'-2" | 11'-3" | 17'-11" | 16'-3" | 14'-2" | 11'-3" |
| 117/8" | AJS® 20 | 18'-8" | 16'-1" | 13'-11" | 11'-1" | 18'-8" | 16'-1" | 13'-11" | 11'-1" |
|  | AJS® 190 | 19'-8" | 16'-8" | 13'-11" | 11'-1" | 19'-8" | 16'-8" | 13'-11" | 11'-1" |
|  | AJS® 25 | 21'-4" | 19'-0" | 15'-10" | 12'-8" | 21'-4" | 19'-0" | 15'-10" | 12'-8" |
| 14" | AJS® 20 | 20'-5" | 17'-8" | 15'-6" | 12'-5" | 20'-5" | 17'-8" | 15'-6" | 12'-5" |
|  | AJS® 190 | 21'-11" | 18'-8" | 15'-6" | 12'-5" | 21'-11" | 18'-8" | 15'-6" | 12'-5" |
|  | AJS® 25 | 24'-2" | 20'-11" | 17'-5" | 13'-11" | 24'-2" | 20'-11" | 17'-5" | 13'-11" |
| 16" | AJS® 20 | 22'-0" | 19'-1" | 17'-1" | 13'-7" | 22'-0" | 19'-1" | 17'-1" | 13'-7" |
|  | AJS® 190 | 23'-7" | 20'-5" | 17'-1" | 13'-7" | 23'-7" | 20'-5" | 17'-1" | 13'-7" |
|  | AJS® 25 | 26'-9" | 22'-8" | 18'-11" | 15'-1" | 26'-9" | 22'-8" | 18'-11" | 15'-1" |
| 18" | AJS® 25 | 29'-6" | 25'-6" | 23'-4" | 18'-9" | 29'-6" | 25'-6" | 23'-4" | 18'-9" |
|  | AJS® 30 | 30'-4" | 27'-7" | 23'-6" | 18'-9" | 30'-4" | 27'-7" | 23'-6" | 18'-9" |
| 20 | AJS® 25 | 31'-2" | 27'-0" | 24'-8" | 20'-4" | 31'-2" | 27'-0" | 24'-8" | 20'-4" |
|  | AJS® 30 | 32'-10" | 29'-11" | 25'-5" | 20'-4" | 32'-10" | 29'-11" | 25'-5" | 20'-4" |
| 22 | AJS® 25 | 32'-8" | 28'-3" | 25'-10" | 20'-10" | 32'-8" | 28'-3" | 25'-10" | 21'-11" |
|  | AJS® 30 | 35'-4" | 31'-3" | 26'-0" | 20'-10" | 35'-4" | 31'-10" | 27'-5" | 21'-11" |
| 24" | AJS® 25 | 34'-1" | 29'-6" | 26'-7" | 21'-3" | 34'-1" | 29'-6" | 26'-11" | 23'-6" |
|  | AJS® 30 | 37'-9" | 31'-11" | 26'-7" | 21'-3" | 37'-9" | $33^{\prime}-3 "$ | 29'-5" | 23'-6" |

Web Stiffeners Required at All Bearing Locations for Table Below
Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BC CALC® sizing software.

NOTES FOR BOTH TABLES

- Loading based upon Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/600 with 50 psf and L/360 with listed uniform live load.
- Table values assume that 23/32" min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4$ " end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® sizing software.

| Medical, Schools, Libraries: Upper Floor Corridors |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| [Worst Case 80 PSF or 1000 LB Concentrated Live Load] + 25 PSF Dead Load |  |  |  |  |  |  |  |  |  |
| Joist <br> Depth | Joist Series | 3½" Interior Bearing |  |  |  | 5¼" Interior Bearing |  |  |  |
|  |  | $\begin{aligned} & 12 \text { " } \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & 16 " \\ & \text { o.c. } \end{aligned}$ | $\begin{gathered} \text { 19.2" } \\ \text { o.c. } \end{gathered}$ | $\begin{aligned} & 24 " \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & 12 " \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & 16 " \\ & \text { o.c. } \end{aligned}$ | $\begin{gathered} \text { 19.2" } \\ \text { o.c. } \end{gathered}$ | $\begin{aligned} & 24 " \\ & \text { o.c. } \end{aligned}$ |
| 91⁄2" | AJS® 20 | 15'-11" | 13'-10" | 11'-6" | 9'-3" | 15'-11" | 13'-10" | 11'-6" | 9'-3' |
|  | AJS® 190 | 16'-6" | 13'-11" | 11'-6" | 9'-3' | 16'-6" | 13'-11" | 11'-6" | 9'-3' |
|  | AJS ${ }^{\text {® }} 25$ | 17'-11" | 16'-2" | 13'-5" | 10'-9" | 17'-11" | 16'-2" | 13'-5" | 10'-9" |
| 117/8" | AJS ${ }^{\text {® }} 20$ | 18'-2" | 15'-9" | 13'-3" | 10'-6" | 18'-2" | 15'-9" | 13'-3" | 10'-6" |
|  | AJS® 190 | 19'-6" | 15'-11" | 13'-3" | 10'-6" | 19'-6" | 15'-11" | 13'-3" | 10'-6" |
|  | AJS® 25 | 21'-4" | 18'-1" | 15'-1" | 12'-0" | 21'-4" | 18'-1" | 15'-1" | 12'-0" |
| 14" | AJS® 20 | 20'-0" | 17'-3" | 14'-9" | 11'-10" | 20'-0" | 17'-3" | 14'-9" | 11'-10" |
|  | AJS® 190 | 21'-5" | 17'-9" | 14'-9" | 11'-10" | 21'-5" | 17'-9" | 14'-9" | 11'-10" |
|  | AJS® 25 | 24'-2" | 19'-11" | 16'-7" | 13'-3" | 24'-2" | 19'-11" | 16'-7" | 13'-3" |
| 16" | AJS ${ }^{\text {® }} 20$ | 21'-6" | 18'-7" | 16'-3" | 13'-0" | 21'-6" | 18'-7" | 16'-3" | 13'-0" |
|  | AJS® 190 | 23'-0" | 19'-6" | 16'-3" | 13'-0" | 23'-0" | 19'-6" | 16'-3" | 13'-0" |
|  | AJS® 25 | 26'-9" | 21'-7" | 18'-0" | 14'-4" | 26'-9" | 21'-7" | 18'-0" | 14'-4" |
| 18" | AJS® 25 | 28'-10" | 24'-11" | 22'-4" | 17'-10" | 28'-10" | 24'-11" | 22'-4" | 17'-10" |
|  | AJS® 30 | 30'-4" | 26'-10" | 22'-4" | 17'-10" | 30'-4" | 26'-10" | 22'-4" | 17'-10" |
| 20 | AJS® 25 | 30'-5" | 26'-4" | 24'-1" | 19'-4" | 30'-5" | 26'-4" | 24'-1" | 19'-4" |
|  | AJS® 30 | 32'-10" | 29'-1" | 24'-2" | 19'-4" | 32'-10" | 29'-1" | 24'-2" | 19'-4" |
| 22 | AJS® 25 | 31'-11" | 27'-7" | 24'-9" | 19'-10" | 31'-11" | 27'-7" | 25'-2" | 20'-10" |
|  | AJS® 30 | 35'-4" | 29'-9" | 24'-9" | 19'-10" | 35'-4" | 31'-1" | 26'-1" | 20'-10" |
| 24" | AJS ${ }^{\text {® }} 25$ | $33^{\prime}-3 "$ | 28'-9" | 25'-4" | 20'-3" | 33'-3" | 28'-9" | 26'-3" | 22'-4" |
|  | AJS ${ }^{\circledR} 30$ | 37'-5" | 30'-5" | 25'-4" | 20'-3" | 37'-5" | 32'-5" | 28'-0" | 22'-4" |


| Retail - 1st Floor / Schools - 1st Floor Corridors |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| [Worst Case 100 PSF or 1000 LB Concentrated Live Load] + 25 PSF Dead Load |  |  |  |  |  |  |  |  |  |
| Joist <br> Depth | Joist Series | 31/2" Interior Bearing |  |  |  | 51/4" Interior Bearing |  |  |  |
|  |  | 12 " o.c. | $\begin{aligned} & 16 " \\ & \text { o.c. } \end{aligned}$ | $\begin{gathered} \text { 19.2" } \\ \text { o.c. } \end{gathered}$ | $\begin{aligned} & 24 " \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & 12 " \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & 16 " \\ & \text { o.c. } \end{aligned}$ | $\begin{gathered} \text { 19.2" } \\ \text { o.c. } \end{gathered}$ | $\begin{aligned} & 24 " \\ & \text { o.c. } \end{aligned}$ |
| 91⁄2" | AJS® 20 | 14'-7" | 11'-8" | 9'-8" | 7'-9" | 14'-7" | 11'-8" | 9'-8" | 7'-9" |
|  | AJS® 190 | 15'-5" | 11'-8" | 9'-8" | 7'-9" | 15'-5" | 11'-8" | 9'-8" | 7'-9" |
|  | AJS® 25 | 16'-9" | 13'-7" | 11'-3" | 9'-0" | 16'-9" | 13'-7" | 11'-3" | 9'-0" |
| 117/8" | AJS® 20 | 16'-8" | 13'-4" | 11'-1" | 8'-10" | 16'-8" | 13'-4" | 11'-1" | 8'-10" |
|  | AJS® 190 | 17'-10" | 13'-4" | 11'-1" | 8'-10" | 17'-10" | 13'-4" | 11'-1" | 8'-10" |
|  | AJS ${ }^{\text {® }} 25$ | 19'-11" | 15'-2" | 12'-8" | 10'-1" | 19'-11" | 15'-2" | 12'-8" | 10'-1" |
| 14" | AJS ${ }^{\text {® }} 20$ | 18'-3" | 14'-11" | 12'-5" | 9'-11" | 18'-3" | 14'-11" | 12'-5" | 9'-11" |
|  | AJS® 190 | 19'-7" | 14'-11" | 12'-5" | 9'-11" | 19'-7" | 14'-11" | 12'-5" | 9'-11" |
|  | AJS® 25 | 22'-3" | 16'-8" | 13'-11" | 11'-1" | 22'-3" | 16'-8" | 13'-11" | 11'-1" |
| 16" | AJS ${ }^{\text {2 }} 20$ | 19'-8" | 16'-4" | 13'-7" | 10'-10" | 19'-8" | 16'-4" | 13'-7" | 10'-10" |
|  | AJS® 190 | 21'-1" | 16'-4" | 13'-7" | 10'-10" | 21'-1" | 16'-4" | 13'-7" | 10'-10" |
|  | AJS® 25 | 24'-2" | 18'-1" | 15'-1" | 12'-0" | 24'-2" | 18'-1" | 15'-1" | 12'-0" |
| 18" | AJS® 25 | 26'-5" | 22'-6" | 18'-9" | 15'-0" | 26'-5' | 22'-6" | 18'-9" | 15'-0" |
|  | AJS® 30 | 28'-5" | 22'-6" | 18'-9" | 15'-0" | 28'-5" | 22'-6" | 18'-9" | 15'-0" |
| 20" | AJS® 25 | 27'-11" | 24'-2" | 20'-4" | 16'-3" | 27'-11" | 24'-2" | 20'-4" | 16'-3" |
|  | AJS® 30 | 30'-10" | 24'-5" | 20'-4" | 16'-3' | 30'-10" | 24'-5" | 20'-4" | 16'-3" |
| 22" | AJS® 25 | 29'-3" | 25'-0" | 20'-10" | 16'-7" | 29'-3" | 25'-3" | 21'-11" | 17'-6" |
|  | AJS® 30 | 32'-11" | 25'-0" | 20'-10" | 16'-7" | 32'-11" | 26'-4" | 21'-11" | 17'-6" |
| 24" | AJS® 25 | 30'-6" | 25'-6" | 21'-3" | 17'-0" | 30'-6" | 26'-4" | 23'-6" | 18'-9" |
|  | AJS ${ }^{\text {® }} 30$ | $34{ }^{\prime}-1{ }^{\prime \prime}$ | 25'-6" | 21'-3" | 17'-0" | $34^{\prime}-4{ }^{\prime \prime}$ | 28'-2" | 23'-6" | 18'-9" |

## Web Stiffeners

Required at All Bearing Locations for Table Below
Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BC CALC® sizing software.

NOTES FOR BOTH TABLES

- Loading based upon Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/600 with 50 psf and L/360 with listed uniform live load.
- Table values assume that 23/32" min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum 2¼" end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ sizing software.


# 80 PSF Live \& 2000 lb Conc. Load Span Tables 

## Office: Upper Floor Corridors

Web Stiffeners Required at All Bearing Locations for Table Below
Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BC CALC ${ }^{\circledR}$ sizing software.
[Worst Case 80 PSF or 2000 LB Concentrated Live Load] + 25 PSF Dead Load

| Joist <br> Depth | Joist Series | 3½" Interior Bearing |  |  |  | 5¼" Interior Bearing |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 12 " o.c. | 16" o.c. | 19.2" o.c. | 24" о.c. | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| 91⁄2" | AJS® 20 | 14'-7" | 11'-7" | 10'-0" | 4'-11" | 14'-7" | 11'-7" | 10'-0" | 4'-11" |
|  | AJS® 190 | 16'-3" | 12'-11" | 10'-2" | 4'-11" | 16'-3" | 12'-11" | 10'-2" | 4'-11" |
|  | AJS ${ }^{\circledR} 25$ | 17'-11" | 16'-2" | 10'-2" | 4'-11" | 17'-11" | 16'-2" | 10'-2" | 4'-11" |
| 117/8" | AJS® 20 | 17'-10" | 14'-3" | 12'-4" | 6'-1" | 17'-10" | 14'-3" | 12'-4" | 6'-1" |
|  | AJS® 190 | 19'-6" | 15'-10" | $13^{\prime}-3^{\prime \prime}$ | 6'-1' | 19'-6" | 15'-10" | $13^{\prime}-3^{\prime \prime}$ | 6'-1' |
|  | AJS® 25 | $21^{\prime}-4 "$ | 18'-1" | $13^{\prime}-5{ }^{\prime \prime}$ | 6'-1" | $21^{\prime}-4{ }^{\prime \prime}$ | 18'-1" | $13^{\prime}-5^{\prime \prime}$ | 6'-1" |
| 14" | AJS® 20 | 20'-0" | 16'-6" | 14'-3" | 7'-5" | 20'-0" | 16'-6" | 14'-3" | 7'-5" |
|  | AJS® 190 | $21^{\prime}-5^{\prime \prime}$ | 17'-9" | 14'-9" | 7'-5' | $21^{\prime}-5^{\prime \prime}$ | 17'-9" | 14'-9" | 7'-5' |
|  | AJS® 25 | 24'-2" | 19'-11" | 16'-7" | 7'-5' | 24'-2" | 19'-11" | 16'-7" | 7'-5' |
| 16" | AJS® 20 | 21'-6" | 18'-6" | 16'-0" | 9'-1" | 21'-6" | 18'-6" | 16'-0" | 9'-1" |
|  | AJS® 190 | 23'-0" | 19'-6" | $16^{\prime}-3 "$ | 9'-1" | 23'-0" | 19'-6" | 16'-3" | 9'-1" |
|  | AJS® 25 | 26'-9" | 21'-7" | 18'-0" | 9'-1" | 26'-9" | 21'-7" | 18'-0" | 9'-1" |
| 18" | AJS® 25 | 28'-10" | 24'-11" | 22'-4" | 17'-10" | 28'-10" | 24'-11" | 22'-4" | 17'-10" |
|  | AJS® 30 | 30'-4" | 26'-10" | 22'-4" | 17'-10" | 30'-4" | 26'-10" | 22'-4" | 17'-10" |
| $20 "$ | AJS® 25 | $30^{\prime}-5{ }^{\prime \prime}$ | 26'-4" | 24'-1" | 19'-4" | $30^{\prime}-5^{\prime \prime}$ | 26'-4" | 24'-1" | 19'-4" |
|  | AJS® 30 | 32'-10" | 29'-1" | 24'-2" | 19'-4" | 32'-10" | 29'-1" | 24'-2" | 19'-4" |
| 22" | AJS® 25 | 31'-11" | 27'-7" | 24'-9" | 19'-10" | 31'-11" | 27'-7" | 25'-2" | 20'-10" |
|  | AJS® 30 | $35^{\prime}-4 "$ | 29'-9" | 24'-9" | 19'-10" | $35^{\prime}-4 "$ | 31'-1" | 26'-1" | 20'-10" |
| 24" | AJS® 25 | $33^{\prime}-3^{\prime \prime}$ | 28'-9" | $25^{\prime}-4 "$ | 20'-3' | $33^{\prime}-3^{\prime \prime}$ | 28'-9" | 26'-3' | 22'-4" |
|  | AJS ${ }^{\text {® }} 30$ | $37{ }^{\prime}-5$ | $30^{\prime}-5{ }^{\prime \prime}$ | 25'-4" | 20'-3' | $37^{\prime}-5{ }^{\prime \prime}$ | $32 '-5 "$ | 28'-0" | 22'-4" |

## NOTES

- Loading based upon Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/600 with 50 psf and L/360 with 80 psf.
- Table values assume that $23 / 32$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4^{\prime \prime}$ end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® ${ }^{\circledR}$ sizing software.

Hotels: Public Rooms \& Corridors
Retail - All Corridors; Medical - 1st Floor Corridors
Restaurants \& Dining Rooms
Assembly Areas \& Theatres: Lobbies, Movable Seat Areas
Web Stiffeners Required at All Bearing Locations for Table Below
Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BC CALC® ${ }^{\circledR}$ sizing software.

| 100 PSF Live Load + 25 PSF Dead Load |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Joist Depth | Joist Series | 3112" Interior Bearing |  |  |  | 511/4" Interior Bearing |  |  |  |
|  |  | 12 " o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| 91/2" | AJS® 20 | 14'-7" | 11'-8" | 9'-8" | 7'-9" | 14'-7" | 11'-8" | 9'-8" | 7'-9" |
|  | AJS® 190 | 15'-5" | 11'-8" | 9'-8" | 7'-9" | 15'-5" | 11'-8" | 9'-8" | 7'-9" |
|  | AJS® 25 | 16'-9" | 13'-7" | 11'-3" | 9'-0' | 16'-9" | 13'-7" | 11'-3" | 9'-0" |
| 117/8" | AJS® 20 | 16'-8" | 13'-4" | 11'-1" | 8'-10" | 16'-8" | 13'-4" | 11'-1" | 8'-10" |
|  | AJS® 190 | 17'-10" | 13'-4" | 11'-1" | 8'-10" | 17'-10" | $13^{\prime}-4 "$ | 11'-1" | 8'-10" |
|  | AJS® 25 | 19'-11" | 15'-2" | 12'-8" | 10'-1" | 19'-11" | 15'-2" | 12'-8" | 10'-1" |
| 14" | AJS® 20 | 18'-3" | 14'-11" | 12'-5" | 9'-11" | 18'-3" | 14'-11" | 12'-5" | 9'-11" |
|  | AJS® 190 | 19'-7" | 14'-11" | 12'-5" | 9'-11" | 19'-7" | 14'-11" | 12'-5" | 9'-11" |
|  | AJS® 25 | 22'-3" | 16'-8" | 13'-11" | 11'-1" | 22 -3" | 16'-8" | 13'-11" | 11'-1" |
| $16 "$ | AJS® 20 | 19'-8" | $16^{\prime}-4 "$ | $13^{\prime}-7{ }^{\prime \prime}$ | 10'-10" | 19'-8" | $16^{\prime}-4 "$ | 13'-7" | 10'-10" |
|  | AJS® 190 | 21'-1" | 16'-4" | 13'-7" | 10'-10" | 21'-1" | 16'-4" | 13'-7" | 10'-10" |
|  | AJS® 25 | 24'-2" | 18'-1" | 15'-1" | 12'-0" | 24'-2" | 18'-1" | 15'-1" | 12'-0" |
| 18" | AJS® 25 | 26'-5" | 22'-6" | 18'-9" | 15'-0" | 26'-5" | 22'-6" | 18'-9" | 15'-0" |
|  | AJS® 30 | $28^{\prime}-5^{\prime \prime}$ | 22'-6" | 18'-9" | 15'-0" | 28'-5" | 22'-6" | 18'-9" | 15'-0" |
| $20 "$ | AJS® 25 | 27'-11" | 24'-2" | 20'-4" | 16'-3" | 27'-11" | 24'-2" | 20'-4" | 16'-3" |
|  | AJS® 30 | 30'-10" | 24'-5" | 20'-4" | $16^{\prime}-3 "$ | 30'-10" | 24'-5" | 20'-4" | 16'-3" |
| 22 " | AJS® 25 | 29'-3" | 25'-0" | 20'-10" | 16'-7" | 29'-3" | 25'-3" | 21'-11" | 17'-6" |
|  | AJS® 30 | 32'-11" | 25'-0" | 20'-10" | 16'-7" | 32'-11" | 26'-4" | 21'-11" | 17'-6" |
| $24 "$ | AJS® 25 | 30'-6" | 25'-6" | $21^{\prime}-3{ }^{\prime \prime}$ | 17'-0" | 30'-6" | 26'-4" | 23'-6" | 18'-9" |
|  | AJS® 30 | $34^{\prime}-1{ }^{\prime \prime}$ | 25'-6" | $21^{\prime}-3 "$ | 17'-0" | $34^{\prime}-4 "$ | 28'-2" | 23'-6" | 18'-9" |

## NOTES

- Loading based upon Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/600 with 50 psf and L/360 with 100 psf.
- Table values assume that $23 / 32$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4^{\prime \prime}$ end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ sizing software.


## Office: Lobbies \& 1st Floor Corridors

## Web Stiffeners Required at All Bearing Locations for Table Below

Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BCCALC® sizing software.

| [Worst Case 100 PSF or 2000 LB Concentrated Live Load] + 25 PSF Dead Load |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $31122^{\prime \prime}$ Interior Bearing |  |  |  | 51⁄4" Interior Bearing |  |  |  |
| Depth | Joist Series | 12 " o.c. | 16" о.c. | 19.2" o.c. | 24" о.c. | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| $91 / 2{ }^{\prime \prime}$ | AJS® 20 | 14'-7" | 11'-7" | 9'-8" | 4'-11" | 14'-7" | 11'-7" | 9'-8" | 4'-11" |
|  | AJS® 190 | 15'-5" | 11'-8" | 9'-8" | 4'-11" | $15^{\prime}-5{ }^{\prime \prime}$ | 11'-8" | $99^{\prime \prime} \mathbf{8 ' ~}^{\prime}$ | 4'-11" |
|  | AJS® 25 | 16'-9" | $13^{\prime}-7{ }^{\prime \prime}$ | 10'-2" | 4'-11" | 16'-9" | 13 -7" | 10'-2" | 4'-11" |
| 117/8" | AJS® 20 | 16'-8" | $13^{\prime}-4{ }^{\prime \prime}$ | 11'-1" | $6^{\prime}-1{ }^{\prime \prime}$ | 16'-8" | $13^{\prime}-4{ }^{\prime \prime}$ | 11'-1" | $6^{\prime}-1 "$ |
|  | AJS® 190 | 17'-10" | $13^{\prime}-4 "$ | 11'-1" | $6^{\prime}-1{ }^{\prime \prime}$ | 17'-10" | $13^{\prime}-4 "$ | 11'-1" | 6'-1" |
|  | AJS® 25 | 19'-11" | $15^{\prime}-2{ }^{\prime \prime}$ | 12'-8" | $6^{\prime}-1 "$ | 19'-11" | $15^{\prime}-2{ }^{\prime \prime}$ | $12^{\prime}-8{ }^{\prime \prime}$ | 6'-1" |
| 14 " | AJS® 20 | 18'-3" | 14'-11" | $12^{\prime 2}$ " | 7'-5" | 18'-3" | 14'-11" | $12^{\prime}-5{ }^{\prime \prime}$ | 7'-5" |
|  | AJS® 190 | 19'-7" | 14'-11" | 12'-5" | 7'-5" | 19'-7" | 14'-11" | $12^{\prime}-5{ }^{\prime \prime}$ | 7'-5" |
|  | AJS® 25 | $22^{\prime}-3$ " | 16'-8" | 13'-11" | 7'-5" | $22^{\prime}-3$ " | 16'-8" | 13'-11" | 7'-5" |
| 16" | AJS® 20 | 19'-8" | 16'-4" | $13^{\prime}-7{ }^{\prime \prime}$ | $9 ' 1{ }^{\prime \prime}$ | 19'-8" | $16^{\prime}-4 "$ | $13^{\prime}-7{ }^{\prime \prime}$ | 9'-1" |
|  | AJS® 190 | 21'-1" | 16'-4" | 13'-7" | 9'-1" | 21'-1" | 16'-4" | $13^{\prime \prime}$-7" | 9'-1" |
|  | AJS® 25 | $24^{\prime}-2$ " | 18'-1" | 15'-1" | 9'-1" | $24^{\prime}-2{ }^{\prime \prime}$ | 18'-1" | 15'-1" | 9'-1" |
| 18" | AJS® 25 | $26^{\prime}-5{ }^{\prime \prime}$ | 22'-6" | 18'-9" | 15'-0" | 26'-5" | 22'-6" | 18'-9" | 15'-0" |
|  | AJS® 30 | $28^{\prime}-5{ }^{\prime \prime}$ | 22'-6" | 18'-9" | 15'-0" | 28'-5" | 22'-6" | 18'-9" | 15'-0" |
| 20" | AJS® 25 | 27'-11" | 24'-2" | 20'-4" | 16'-3" | $27^{\prime}-11^{\prime \prime}$ | 24'-2" | 20'-4" | 16'-3" |
|  | AJS® 30 | 30'-10" | 24'-5" | 20'-4" | 16'-3" | 30'-10" | $24^{\prime}-5{ }^{\prime \prime}$ | 20'-4" | 16'-3" |
| 22" | AJS® 25 | 29'-3" | 25'-0" | 20'-10" | 16'-7" | $29^{\prime}-3 "$ | $25^{\prime}-3 "$ | 21'-11" | 17'-6" |
|  | AJS® 30 | $32^{\prime \prime} 11{ }^{\prime \prime}$ | $25^{\prime \prime}$-0" | 20'-10" | 16'-7" | 32'-11" | $26^{\prime \prime} \mathbf{4 "}^{\prime \prime}$ | $21^{\prime \prime} 11^{\prime \prime}$ | 17'-6" |
| 24" | AJS® 25 | 30'-6" | $25^{\prime \prime}$-6" | $21^{\prime \prime} 3^{\prime \prime}$ | 17'-0" | 30'-6" | $26^{\prime \prime} \mathbf{4 "}^{\prime \prime}$ | $23^{\prime \prime}$ 6" | 18'-9" |
|  | AJS® 30 | $34^{\prime \prime}{ }^{\prime \prime}$ | $25^{\prime \prime}$ " ${ }^{\prime \prime}$ | $21^{\prime \prime} \mathbf{3 '}^{\prime \prime}$ | $17^{\prime \prime}-0{ }^{\prime \prime}$ | $34^{\prime \prime}-4^{\prime \prime}$ | $28^{\prime \prime} 2^{\prime \prime}$ | $23^{\prime \prime}-6 "$ | 18'-9" |

## NOTES

- Loading based upon Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/600 with 50 psf and $\mathrm{L} / 360$ with 100 psf .
- Table values assume that $23 / 32$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4^{\prime \prime}$ end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ sizing software.

Web Stiffeners Required at All Bearing Locations for Table Below
Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BC CALC ${ }^{\circledR}$ sizing software.
125 PSF Live Load + 25 PSF Dead Load

| Joist Depth | Joist Series | 3½" Interior Bearing |  |  |  | 5¼" Interior Bearing |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 12 " o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| 91⁄2" | AJS® 20 | 12'-11" | 9'-8" | 8'-0" | 6'-5" | 12'-11" | 9'-8" | 8'-0" | 6'-5" |
|  | AJS® 190 | 12'-11" | 9'-8" | 8'-0" | 6'-5" | 12'-11" | 9'-8" | 8'-0" | 6'-5" |
|  | AJS® 25 | 15'-1" | 11'-3" | 9'-5" | 7'-6" | 15'-1" | 11'-3" | 9'-5" | 7'-6" |
| 117/8" | AJS® 20 | 14'-10" | 11'-1" | 9'-3' | 7'-4" | 14'-10" | 11'-1" | 9'-3' | 7'-4" |
|  | AJS® 190 | 14'-10" | 11'-1" | 9'-3' | 7'-4" | 14'-10" | 11'-1" | 9'-3' | 7'-4' |
|  | AJS® 25 | 16'-11" | 12'-8" | 10'-6" | 8'-5" | 16'-11" | 12'-8" | 10'-6" | 8'-5" |
| $14 "$ | AJS® 20 | 16'-7" | 12'-5" | 10'-4" | 8'-3' | 16'-7" | 12'-5" | 10'-4" | 8'-3' |
|  | AJS® 190 | 16'-7" | 12'-5" | 10'-4" | 8'-3' | 16'-7" | 12'-5" | 10'-4" | 8'-3' |
|  | AJS® 25 | 18'-7" | 13'-11" | 11'-6" | 9'-3' | 18'-7" | 13'-11" | 11'-6" | 9'-3' |
| $16 "$ | AJS® 20 | 18'-0" | 13'-7" | 11'-4" | 9'-0' | 18'-0" | 13'-7" | 11'-4" | 9'-0' |
|  | AJS® 190 | 18'-2" | 13'-7" | 11'-4" | 9'-0' | 18'-2" | 13'-7" | 11'-4" | 9'-0" |
|  | AJS® 25 | 20'-2" | 15'-1" | 12'-6" | 10'-0" | 20'-2" | 15'-1" | 12'-6" | 10'-0" |
| 18" | AJS® 25 | 24'-1" | 18'-9" | 15'-7" | 12'-6" | 24'-1" | 18'-9" | 15'-7" | 12'-6" |
|  | AJS® 30 | 25'-1" | 18'-9" | 15'-7" | 12'-6" | 25'-1" | 18'-9" | 15'-7" | 12'-6" |
| $20 "$ | AJS® 25 | 25'-5" | 20'-4" | 16'-11" | 13'-6" | $25^{\prime}-5^{\prime \prime}$ | 20'-4" | 16'-11" | 13'-6" |
|  | AJS® 30 | 27'-2" | 20'-4" | 16'-11" | 13'-6" | 27'-2" | 20'-4" | 16'-11" | 13'-6" |
| 22" | AJS® 25 | 26'-8" | 20'-10" | 17'-4" | 13'-10" | 26'-8' | 21'-11" | 18'-3" | 14'-7" |
|  | AJS® 30 | 27'-9" | 20'-10" | 17'-4" | 13'-10" | 29'-3' | 21'-11" | 18'-3" | 14'-7" |
| $24 "$ | AJS® 25 | 27'-10" | $21^{\prime \prime}-3$ ' | 17'-8" | 14'-2" | 27'-10" | 23'-6" | 19'-7" | 15'-7" |
|  | AJS® 30 | 28'-5" | $21^{\prime}-3{ }^{\prime \prime}$ | 17'-8" | 14'-2" | $31^{\prime}-4{ }^{\prime \prime}$ | 23'-6" | 19'-7" | 15'-7" |

## NOTES

- Loading based upon Light Storage - Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/600 with 50 psf and L/360 with 125 psf.
- Table values assume that $23 / 32$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4$ " end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ sizing software.


## 250 PSF Live Load (Heavy Storage) Span Tables

| Web Stiffeners Required at All Bearing Locations for Table Below <br> Spans without web stiffeners and/or shorter bearings are possible, analyze such conditions with BC CALC® sizing software. 250 PSF Live Load + 25 PSF Dead Load |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Joist Depth | Joist Series | 3½" Interior Bearing |  |  | 51⁄4" Interior Bearing |  |  |
|  |  | 12" o.c. | 16" o.c. | 19.2" о.c. | 12" о.c. | 16" o.c. | 19.2" o.c. |
| $91 / 2 "$ | AJS® 20 | 7'-0' | 5'-3" | 4'-4" | 7'-0' | 5'-3" | 4'-4" |
|  | AJS® 190 | 7'-0" | 5'-3' | 4'-4" | 7'-0" | 5'-3' | 4'-4" |
|  | AJS® 25 | 8'-2" | 6'-1' | 5'-1" | 8'-2" | 6'-1' | 5'-1" |
| 117/8" | AJS® 20 | 8'-0" | 6'-0' | 5'-0" | 8'-0" | 6'-0' | 5'-0" |
|  | AJS® 190 | 8'-0" | 6'-0' | 5'-0" | 8'-0" | 6'-0" | 5'-0" |
|  | AJS® 25 | 9'-2" | 6'-10" | 5'-8" | 9'-2" | 6'-10" | 5'-8" |
| 14" | AJS® 20 | 9'-0' | 6'-8" | 5'-7" | 9'-0" | 6'-8" | 5'-7" |
|  | AJS® 190 | 9'-0' | 6'-8" | 5'-7" | 9'-0' | 6'-8" | 5'-7" |
|  | AJS® 25 | 10'-1" | 7'-6" | 6'-3' | 10'-1" | 7'-6" | 6'-3' |
| 16" | AJS® 20 | 9'-10" | 7'-4" | 6'-1" | 9'-10" | 7'-4" | 6'-1" |
|  | AJS® 190 | 9'-10" | 7'-4" | 6'-1' | 9'-10" | 7'-4" | 6'-1' |
|  | AJS® 25 | 10'-11" | 8'-2' | 6'-9" | 10'-11" | 8'-2" | 6'-9" |
| 18" | AJS® 25 | 13'-7" | 10'-2" | 8'-5" | 13'-7" | 10'-2' | 8'-5" |
|  | AJS® 30 | 13'-7" | 10'-2" | 8'-5" | 13'-7" | 10'-2" | 8'-5" |
| $20 "$ | AJS® 25 | 14'-9" | 11'-0" | 9'-2' | 14'-9" | 11'-0" | 9'-2' |
|  | AJS® 30 | 14'-9" | 11'-0" | 9'-2' | 14'-9" | 11'-0" | 9'-2' |
| 22" | AJS® 25 | 15'-1" | 11'-3" | 9'-5" | 15'-11" | 11'-11" | 9'-11" |
|  | AJS® 30 | 15'-1" | 11'-3" | 9'-5" | 15'-11" | 11'-11" | 9'-11" |
| 24" | AJS® 25 | 15'-5" | 11'-6" | 9'-7" | 17'-0" | 12'-9" | 10'-7" |
|  | AJS® 30 | 15'-5" | 11'-6" | 9'-7" | 17'-0" | 12'-9"" | 10'-7" |

## NOTES

- Loading based upon Heavy Storage - Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of $\mathrm{L} / 240$. Live load deflection limited to L/600 with 50 psf and L/360 with 250 psf.
- Table values assume that $23 / 32$ " min. plywood/OSB rated sheathing is glued and nailed to joists.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4^{\prime \prime}$ end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® ${ }^{\text {® }}$ sizing software.


## AJS® Joists

NOTE
The illustration below is showing several suggested applications for the Boise Cascade EWP products. It is not intended to show an actual house under construction.

See also Intermediate Bearing details, page 16

NO MIDSPAN BRIDGING IS REQUIRED FOR ALLJOIST ${ }^{\circledR}$ PRODUCT

FOR INSTALLATION STABILITY,
Temporary strut lines ( $1 \times 4$ min.) $8^{\prime}$ on center max. Fasten at each joist with 2-8d nails minimum.

Dimension lumber is not suitable for use as a rim board in ALLJOIST ${ }^{\circledR}$ floor systems.

BOISE CASCADE® Rimboard See pages 15 and 34.

When installing Boise Cascade EWP products with treated wood, use only connectors/fasteners that are approved for use with the corresponding wood treatment.


BCI® Joists, VERSA-LAM ${ }^{\circledR}$ and ALLJOIST® must be stored, installed and used in accordance with the Boise Cascade EWP Installation Guide, building codes, and to the extent not inconsistent with the Boise Cascade EWP Installation Guide, usual and customary building practices and standards. VERSA-LAM ${ }^{\circledR}$, ALLJOIST®, and $\mathrm{BCI®}$ Joists must be wrapped, covered, and stored off of the ground on stickers at all times prior to installation. VERSA-LAM ${ }^{\circledR}$, ALLJOIST® ${ }^{\circledR}$ and $\mathrm{BCI}{ }^{\circledR}$ Joists are intended only for applications
that assure no exposure to weather or the elements and an environment that is free from moisture from any source, or any pest, organism or substance which degrades or damages wood or glue bonds. Failure to correctly store, use or install VERSA-LAM ${ }^{\circledR}$, ALLJOIST®, and $\mathrm{BCI}{ }^{\circledR}$ Joist in accordance with the Boise Cascade EWP Installation Guide will void the limited warranty.

## SAFETY WARNING

DO NOT ALLOW WORKERS ON AJS® JOISTS UNTIL ALL HANGERS, AJS® RIM JOISTS, RIM BOARDS, AJS® BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW. SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

- Build a braced end wall at the end of the bay, or permanently install the first eight feet of AJS® Joist and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of AJS® ${ }^{\circledR}$ Joist at the end of the bay.
- All hangers, AJS® rim joists, rim boards, AJS® blocking panels, and x-bracing must be completely installed and properly nailed as each AJS® Joist is set.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional AJS ${ }^{\circledR}$ Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS® Joist with two 8d nails.
- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS ${ }^{\circledR}$ Joist to within $1 / 2$ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or rollover under light construction loads.
- Do not stack construction materials (sheathing, drywall, etc) in the middle of AJS ${ }^{\circledR}$ Joist spans, contact Boise Cascade EWP Engineering for proper storage and shoring information.


## PRODUCT HANDLING TO AND AT JOB SITES

There are some differences between engineered wood products and traditional lumber products in terms of product handling: AJS ${ }^{\circledR}$ joists are significantly weaker in the flat orientation. VERSA-LAM ${ }^{\circledR}$ is denser and due to the coating applied to the surface, can be more apt to sliding. Please consider these differences when transporting and handling engineered wood products.


| END BEARING DETAILS |  |  |
| :---: | :---: | :---: |
| F07 | F07A <br> Dimension lumber is not suitable for use as rimboard with AJS ${ }^{\circledR}$ Joists. <br> Blocking may be required perpendicular to wall, consult design professional of record and/or local building official. | F02 <br> Use of AJS ${ }^{\circledR}$ rimjoist requires $2 \times 6$ wall for minimum joist bearing. |
| F01 | F18 | F52 |
| F08 | F03 <br> Note: AJS ${ }^{\circledR}$ floor joist must be designed to carry wall above when not stacked over wall below. <br> Blocking required underneath braced wall panels and shear walls, consult design professional of record. | Flat $2 \times 4$ blocking with clips may substitute for AJS ${ }^{\circledR}$ blocking. <br> Note: AJS ${ }^{\circledR}$ joist must be designed to support vertical loads from wall above. |

## LATERAL SUPPORT

- AJS ${ }^{\circledR}$ Joists must be laterally supported at end supports (including supports adjacent to overhangs) with hangers, rimboard, or blocking (VERSA-LAM ${ }^{\circledR}$, BOISE CASCADE ${ }^{\circledR}$ Rimboard or AJS ${ }^{\circledR}$ Joist). Metal cross bracing or other x-bracing provides adequate lateral support for AJS® Joists. Consult governing building code for floor diaphragm connection provisions.
- Blocking may be required at intermediate bearings for floor diaphragm per project's structural engineer of record.


## MINIMUM BEARING LENGTH FOR AJS® JOISTS

- Minimum end bearing: $1 \frac{1}{2} 2^{\prime \prime}$ for $91 / 2$ " -16 " deep joists, $13 / 4$ " for 18 " and deeper joists. $31 / 2^{1 "}$ minimum bearing length at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values. Refer to the building code evaluation report or the BC CALC ${ }^{\circledR}$ software.


## NAILING REQUIREMENTS

- AJS ${ }^{\circledR}$ rim joist, rim board or closure panel to AJS ${ }^{\circledR}$ joist:
- Rims or closure panel $111 / 4$ inches thick and less:

2-8d nails, one each in the top and bottom flange.

- AJS® 20/190 rim joist: 2-16d box nails, one each in the top and bottom flange.
- AJS® 25/30 rim joist: Toe-nail top flange to rim joist with 2-10d box nails, one each side of flange.
Boise Cascade EWP • ALLJOIST® Commercial Specifier Guide 01/09/2015 r9/29/2017
- AJS ${ }^{\circledR}$ rim joist, rim board or AJS ${ }^{\circledR}$ blocking panel to support:
- Min. 8d nails @ 6" o.c. per IBC.
- Connection per design professional of record's specification for shear transfer.
- AJS ${ }^{\circledR}$ joist to support:
- 2-8d nails, one on each side of the web, placed $11 / 2$ inches minimum from the end of the AJS ${ }^{\circledR}$ Joist to limit splitting.
- Sheathing to AJS ${ }^{\circledR}$ joist:
- Minimum nailing schedule 8d common nails @ 6" o.c. on edges and 12" o.c. in the field, see IBC Table 2304.9.1 for other fastener options.
- See closest allowable nail spacing limits on page 24 for floor diaphragm nailing specified at closer spacing than IBC.
- Maximum bracing spacing for full lateral stability: 18" for AJS® 190 \& 20, 24" for larger AJS® joist series."
- 14 gauge staples may be substituted for 8 d nails if the staples penetrate at least 1 inch into the joist.
- Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.


## Additional floor framing details available with BC FRAMER ${ }^{\circledR}$ software

(s)

## BACKER AND FILLER BLOCK DIMENSIONS

| Series | Backer Block Thickness | Filler Block Thickness |
| :---: | :---: | :---: |
| 190 <br> 20 | $11 / 8^{\prime \prime}$ or two $1 / 2{ }^{\prime \prime}$ wood panels | $2 x_{\_}+5 / 8{ }^{\prime \prime}$ wood panel |
| 25 | $2 x$ lumber | Double $2 x$ lumber |
| 30 |  |  |

- Cut backer and filler blocks to a maximum depth equal to the web depth minus $1 / 4$ " to avoid a forced fit.
- For deeper AJS® Joists, stack $2 x$ lumber or use multiple pieces of $3 / 4$ " wood panels.


## WEB STIFFENER REQUIREMENTS

- See Web Stiffener Requirements on page 19.

PROTECT AJS® JOISTS FROM THE WEATHER

- AJS ${ }^{\circledR}$ Joists are intended only for applications that provide permanent protection from the weather. Bundles of product should be covered and stored off of the ground on stickers.

AJS ${ }^{\circledR}$ RIM JOISTS AND BLOCKING

| Joist Depth | Vertical Load <br> Transfer Capacity <br> (pIf) |
| :---: | :---: |
| $9 \frac{1}{2 \prime \prime}$ | 1875 |
| $111^{\prime \prime \prime}$ | 1680 |
| $14^{\prime \prime}$ | 1500 |
| $16^{\prime \prime}$ | 1340 |
| $18^{\prime \prime} \& 20^{\prime \prime}(1)$ | 3200 |
| $22^{\prime \prime} \& 24 "$ (1) | 2700 |

1) Web stiffeners required at each end of blocking panel. Distance between stiffeners must be less than 24 ".


## Filler Block Connection Specifications

Filler block connections are to be used in applications where the joists are not loaded evenly. Filler block connections shall be used for uniform and concentrated side load conditions. Top load conditions where the load is not centered or evenly distributed shall be evaluated by the project design professional of record (flange nailing is structurally adequate for non-load bearing partition walls).


| AJS® Series | Depth | Filler Block | Nailing Pattern |
| :---: | :---: | :---: | :---: |
| $\begin{array}{r} \text { 140, 150, } \\ 20,190 \end{array}$ | 91⁄2", 11¹/8", 14", 16" | 2 l lumber $+5 / 8$ " wood panel | 2 rows 10d nails @ 12" o.c., staggered \& clinched |
| 25, 30 | 11/8", 14", 16" | Double 2x lumber | 2 rows 10d nails @ 12" o.c., staggered, nailed from both sides |
| 25, 30 | 18", 20" | Double 2x lumber | 3 rows 10d nails @ 12" o.c., staggered, nailed from both sides |
| 25, 30 | 22", 24" | Double 2x lumber | 4 rows 10d nails @ 12" o.c., staggered, nailed from both sides |

## Notes:

- Filler block is required between AJS ${ }^{\circledR}$ joists for the full length of span at uniform side loads, 24" minimum at concentrated side loads. Filler blocks do not need to be continuous.
- Filler block to be installed tight to bottom flange. Specified filler block material may leave gap between top of block and bottom of top flange. If receiving nails from face mount joist hangers, this gap must be filled to within $1 / 8$ " of top flange. The same filler block material may
be stacked to fill the gap. Provide one row of 10d nails @ 12" o.c., staggered \& clinched for stacked web-filler nailing in addition to nailing shown.
- Triple Members: Nail first two joists as shown, then attach third with same filler and nailing.
- Nail sheathing to all joists per diaphragm nailing schedule.

Concentrated Load Connections

## Concentrated Side Loads

The following detail shows the proper connection of concentrated side loads to single and multiple AJS® members:


| TOP FLANGE HANGER | FACE MOUNT HANGER Single AJS ${ }^{\circledR}$ Header (typical) | FACE MOUNT HANGER Double AJS ${ }^{\circledR}$ Header (typical) |
| :---: | :---: | :---: |
|  |  | FACE MOUNT Double AJS - Header (typical) <br> Hanger Max Load $=2500 \mathrm{lb}$ |

## Notes:

- Use filler block thickness for corresponding AJS® series as specified in filler block connection table, page 3 . For single concentrated side loads, minimum filler block length is 24 " long with $6-10 \mathrm{~d}$ connecting nails.
- Nail sheathing to all joists per diaphragm nailing schedule.
- Refer to hanger manufacturer literature for reductions in hanger capacity due to l-joist headers.
- Max loads shown are for connections only. The structural adequacy of all headers, beams, girders and hangers to be determined by the design professional of record.
- All backer blocks to be nailed with 10-10d nails.
- All nails protruding more than $1 / 4^{\prime \prime}$ to be clinched.


## AJS® Header Blocking at Side Loaded Conditions

Required for triple members, recommended best practices for double members.


## Plan

NOTE: Nail floor sheathing to all joists, blocking and headers per diaphragm nailing schedule.

AJS ${ }^{\circledR}$ Blocking @ end blocks. Fasten blocks with (2) 8 d nails or $21 / 2$ "screws (typical).


## Section A-A

AJS® Joists are manufactured with $11 / 2^{\prime \prime}$ round perforated knockouts in the web at approximately 12 on center

Minimum distance from support, listed in table below, is required for all holes greater than $11 / 2^{\prime \prime}$ MINIMUM DISTANCE (D) FROM ANY SUPPORT TO THE CENTERLINE OF THE HOLE

| Round Hole Diameter [in] |  |  | 2 | 3 | 4 | 5 | 6 | $61 / 2$ | 7 | 8 | $87 / 8$ | 9 | 10 | 11 | 12 | 13 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Rectangular Hole Side [in] |  |  | - | - | 2 | 4 | 6 | 6 | - | - | - | - | - | - | - | - |
| Any 91⁄2" Joist | Span [ft] | 8 | 2'-0" | 2'-5" | 2'-11" | 3'-5" | 3'-10" | 4'-0" |  |  |  |  |  |  |  |  |
|  |  | 12 | 3'-0" | 3'-8" | 4'-5" | 5'-1" | 5'-10" | 6'-0" |  |  |  |  |  |  |  |  |
|  |  | 16 | 4'-0" | 4'-11" | 5'-11" | 6'-10" | 7'-9" | 8'-0" |  |  |  |  |  |  |  |  |
| Round Hole Diameter [in] |  |  | 2 | 3 | 4 | 5 | 6 | 61/2 | 7 | 8 | 87/8 | 9 | 10 | 11 | 12 | 13 |
| Rectangular Hole Side [in] |  |  | - | - | - | 2 | 3 | 4 | 5 | 7 | 8 | - | - | - | - | - |
| Any 117/8" Joist | Span [ ft$]$ | 8 | 1'-0" | 1'-5" | 1'-10" | 2'-3" | 2'-8" | 2'-11" | 3'-1" | 3'-6" | 3'-11" |  |  |  |  |  |
|  |  | 12 | 1'-5" | 2'-1" | 2'-9" | 3'-5" | 4'-0" | 4'-4" | 4'-8" | 5'-4" | 5'-11" |  |  |  |  |  |
|  |  | 16 | 1'-11" | 2'-10" | 3'-8" | 4'-6" | 5'-5" | 5'-10" | 6'-3" | 7'-1" | 7'-10" |  |  |  |  |  |
|  |  | 20 | 2'-5" | 3'-6" | 4'-7" | 5'-8" | 6'-9" | 7'-3" | 7'-10" | 8'-11" | 9'-10" |  |  |  |  |  |
| Round Hole Diameter [in] |  |  | 2 | 3 | 4 | 5 | 6 | $61 / 2$ | 7 | 8 | $87 / 8$ | 9 | 10 | 11 | 12 | 13 |
| Rectangular Hole Side [in] |  |  | - | - | - | - | 2 | 3 | 3 | 5 | 6 | 6 | 8 | 9 | - | - |
| Any 14" Joist | Span [ft] | 8 | 1'-0" | 1'-1" | 1'-2" | 1'-4" | 1'-8" | 1'-11" | 2'-1" | 2'-6" | 2'-10" | 2'-11" | 3'-4" | 3'-9" |  |  |
|  |  | 12 | 1'-0" | 1'-1" | 1'-4" | 2'-0" | 2'-7" | 2'-11" | 3'-2" | 3'-10" | 4'-4" | 4'-5" | 5'-0" | 5'-7" |  |  |
|  |  | 16 | 1'-0" | 1'-1" | 1'-10" | 2'-8" | 3'-5" | 3'-10" | 4'-3" | 5'-1" | 5'-9" | 5'-11" | 6'-8" | 7'-6" |  |  |
|  |  | 20 | 1'-0" | 1'-3" | 2'-4" | 3'-4" | 4'-4" | 4'-10" | 5'-4" | 6'-4" | 7'-3" | 7'-4" | 8'-5" | 9'-5" |  |  |
|  |  | 24 | 1'-0" | 1'-7" | 2'-9" | 4'-0" | 5'-2' | 5'-10" | 6'-5" | 7'-8" | 8'-8" | 8'-10" | 10'-1" | 11'-3' |  |  |
| Round Hole Diameter [in] |  |  | 2 | 3 | 4 | 5 | 6 | $61 / 2$ | 7 | 8 | $87 / 8$ | 9 | 10 | 11 | 12 | 13 |
| Rectangular Hole Side [in] |  |  | - | - | - | - | - | - | 2 | 3 | 5 | 5 | 6 | 8 | 9 | 10 |
| Any 16" Joist | Span <br> [ft] | 8 | 1'-0" | 1'-1" | 1'-2" | 1'-2" | 1'-3" | 1'-3" | 1'-3" | 1'-8" | 2'-0" | 2'-1" | 2'-5" | 2'-10" | 3'-2" | 3'-7" |
|  |  | 12 | 1'-0" | 1'-1" | 1'-2" | 1'-2" | 1'-4" | 1'-8" | 1'-11" | 2'-6" | 3'-0" | 3'-1" | 3'-8" | 4'-3" | 4'-10" | 5'-5" |
|  |  | 16 | 1'-0" | 1'-1" | 1'-2" | 1'-2" | 1'-10" | 2'-2" | 2'-7" | 3'-4" | 4'-0" | 4'-2" | 4'-11" | 5'-8" | 6'-5" | 7'-2" |
|  |  | 20 | 1'-0" | 1'-1" | 1'-2" | 1'-4" | 2'-3" | 2'-9" | 3'-3" | 4'-3" | 5'-1" | 5'-2" | 6'-2" | 7'-1" | 8'-1" | 9'-0" |
|  |  | 24 | 1'-0" | $1^{\prime}-1$ " | 1'-2" | 1'-7" | 2'-9" | 3'-4" | 3'-11" | 5'-1" | 6'-1" | 6'-3" | 7'-4" | 8'-6" | 9'-8" | 10'-10" |

- Select a table row based on joist depth and the actual joist span rounded up to the nearest table span. Scan across the row to the column headed by the appropriate round hole diameter or rectangular hole side. Use the longest side of a rectangular hole. The table value is the closest that the centerline of the hole may be to the centerline of the nearest support.
The entire web may be cut out. DO NOT cut the flanges. Holes apply to either single or multiple joists in repetitive member conditions.
For multiple holes, the amount of uncut web between holes must equal at least twice the diameter (or longest side) of the largest hole.
- $11 / 2^{\prime \prime}$ round knockouts in the web may be removed by using a short piece of metal pipe and hammer.
Holes may be positioned vertically anywhere in the web. The joist may be set with the $1 \frac{1}{2} 2^{\prime \prime}$ knockout holes turned either up or down.
This table was designed to apply to the design conditions covered by tables elsewhere in this publication. Use the BC CALC® software to check other hole sizes or holes under other design conditions. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® software.


## Large Rectangular Holes in AJS® Joists

Hole size table based on maximum uniform load of 40 psf live load and 25 psf dead load, at maximum spacing of 24 " on-center.
Single Span Joist


Notes:
Additional holes may be cut in the web provided they meet the specifications as shown in the hole distance chart shown above or as allowed using BC CALC® sizing software.

| Joist Depth | Maximum Hole Size |  |
| :---: | :---: | :---: |
|  | Simple Span | Multiple Span |
| 91/2" | $6 " \times 12 "$ | $6 " \times 7$ " |
| 117/8" | 8" x 13" | $8 " \times 8 "$ |
| 14" | 9" x 16" | $8{ }^{\prime \prime} \times 13{ }^{\prime \prime}$ |
|  | $10 " \times 14 "$ | $9 " \times 11 "$ |
| 16" | $11^{\prime \prime} \times 16^{\prime \prime}$ | $10 "$ x 14" |
|  | $12^{\prime \prime} \times 15{ }^{\prime \prime}$ | $11{ }^{\prime \prime} \times 12$ |

Multiple Span Joist


Larger holes may be possible for either Single or Multiple span joists; use BC CALC ${ }^{\circledR}$ sizing software for specific analysis.


- The detail to the right on this page shows the installation of cantilever reinforcement for load-bearing cantilevers up to a maximum length of $2^{\prime}-0^{\prime \prime}$. Cantilevers longer than $2^{\prime}-0$ " cannot be reinforced. However, longer cantilevers with lower loads may be allowable without reinforcement. Analyze specific applications with the BC CALC® software.

PLYWOOD / OSB REINFORCEMENT (If Required per BC CALC ${ }^{\circledR}$ analysis)

Analyze AJS® Load Bearing Cantilevers in BC CALC ${ }^{\circledR}$ Software

- 23/32" Min. x 48" long plywood / OSB rated sheathing must match the full depth of the AJS® Joist. Nail to the AJS® Joist with 8d nails at 6" o.c. and nail with 4-8d nails into backer block. When reinforcing both sides, stagger nails to limit splitting. Install with horizontal face grain.
- These requirements assume a 100 PLF wall load and apply to AJS® Joists. Additional support may be required for other loadings. See BC CALC® ${ }^{\text {software. }}$
- Contact Boise Cascade EWP Engineering for reinforcement requirements on AJS® Joist depths greater than 16 ".



## Non-Load Bearing Wall Cantilever Details

AJS® Joists are intended only for applications that provide permanent protection from the weather.

Fasten the $2 \times 8$ minimum to the AJS® Joist by nailing through the backer block and joist web with 2 rows of 10 d nails at 6 " on center. Clinch all nails.


Not to exceed 3'-0" for 100 psf live load

- These details apply to cantilevers with uniform loads only.
- It may be possible to exceed the limitations of these details by analyzing a specific application with the BC CALC ${ }^{\circledR}$ software.


## Web Stiffener Requirements



Structural Panel Web Stiffener

| $\|c\|$ <br> AJS® <br> Series | In <br> Hanger | No <br> Hanger | Minimum <br> Width |
| :---: | :---: | :---: | :---: |
|  | $1 "$ | $11 / 2^{\prime \prime}$ | $25 / 16^{\prime \prime}$ |
| $25 / 30$ | $2 \times 4$ lumber (vertical) |  |  |

## NOTES

- Web stiffeners are optional except as noted below.
- Web stiffeners are always required in hangers that do not extend up to support the top flange of the AJS® Joist. Web stiffeners may be required with certain sloped or skewed hangers or to achieve uplift values. Refer to the hanger manufacturer's installation requirements.
- Web stiffeners are always required in certain roof applications. See Roof Framing Details on page 24.
- Web stiffeners are always required under concentrated loads that exceed 1000 pounds. Install the web stiffeners snug to the top flange in this situation. Follow the nailing schedule for intermediate bearings.
- Web stiffeners may be cut from structural rated wood panels, engineered rimboard or $2 x$ lumber (AJS® 25 and 30 series only).
- For Structural Capacity: Web stiffeners needed to increase the AJS® Joist's reaction capacity at a specific bearing location.
- Lateral Restraint in Hanger: Web stiffeners required when hanger does not laterally support the top flange (e.g., adjustable height hangers). Web stiffeners may be of multiple thickness (e.g., AJS ${ }^{\circledR} 20$ / 190: double $1 / 2^{\prime \prime}$ panel OK)
- Web stiffeners may be used to increase allowable reaction values. See AJS® Design Properties on page 24 or the $B C$ CALC ${ }^{\circledR}$ software.

| Web Stiffener Nailing Schedule |  |  |
| :---: | :---: | :---: |
| AJS® <br> Joist Series | Joist <br> Depth | Nailing |
| 20 | $91 / 2^{\prime \prime}-117 / 8^{\prime \prime}$ | 3-10d |
| 190 | $14 "-16^{\prime \prime}$ | 5-10d |
| 25 | $18 "-24 "$ | 5-10d |
| 25 |  |  |

## Allowable Uniform Floor Load (in pounds per lineal foot [PLF])

## 100\% Load Duration

| Span Length | AJS® 20 Series 2½" Flange Width |  |  |  |  |  |  |  | AJS® 190 Series 2½" Flange Width |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} 91 / 2 " 1 \\ \text { AJS® } 20 \end{gathered}$ |  | $\begin{gathered} 11^{77 / 8} \\ \text { AJS® } 20 \end{gathered}$ |  | $\begin{gathered} \text { 14" } \\ \text { AJS® } 20 \end{gathered}$ |  | $\begin{gathered} 16 " \\ \text { AJS® } 20 \end{gathered}$ |  | $\begin{gathered} 91 / 2 " \\ \text { AJS® }{ }^{\circledR} 190 \end{gathered}$ |  | $\begin{gathered} 11 / 8^{\prime \prime} \\ \text { AJS®190 } \end{gathered}$ |  | $\begin{gathered} 14 " 1 \\ \text { AJS®190 } \end{gathered}$ |  | $\begin{gathered} 16 " \\ \text { AJS®190 } \end{gathered}$ |  |
|  | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load |
| 6 | - | 313 | - | 318 | - | 320 | - | 323 | - | 313 | - | 318 | - | 320 | - | 323 |
| 7 | - | 268 | - | 272 | - | 274 | - | 277 | - | 268 | - | 272 | - | 274 | - | 277 |
| 8 | - | 235 | - | 238 | - | 240 | - | 242 | - | 235 | - | 238 | - | 240 | - | 242 |
| 9 | - | 208 | - | 212 | - | 213 | - | 215 | - | 208 | - | 212 | - | 213 | - | 215 |
| 10 | - | 188 | - | 191 | - | 192 | - | 194 | - | 188 | - | 191 | - | 192 | - | 194 |
| 11 | 161 | 170 | - | 173 | - | 174 | - | 176 | 168 | 170 | - | 173 | - | 174 | - | 176 |
| 12 | 128 | 156 | - | 159 | - | 160 | - | 161 | 133 | 156 | - | 159 | - | 160 | - | 161 |
| 13 | 102 | 144 | - | 146 | - | 147 | - | 149 | 107 | 144 | - | 146 | - | 147 | - | 149 |
| 14 | 83 | 134 | - | 136 | - | 137 | - | 138 | 87 | 134 | - | 136 | - | 137 | - | 138 |
| 15 | 69 | 120 | 113 | 127 | - | 128 | - | 129 | 72 | 125 | 118 | 127 | - | 128 | - | 129 |
| 16 | 57 | 106 | 95 | 119 | - | 120 | - | 121 | 60 | 117 | 99 | 119 | - | 120 | - | 121 |
| 17 | 48 | 93 | 80 | 112 | - | 112 | - | 114 | 50 | 101 | 83 | 112 | - | 112 | - | 114 |
| 18 | 41 | 82 | 68 | 106 | 98 | 106 | - | 107 | 43 | 86 | 71 | 106 | 102 | 106 | - | 107 |
| 19 |  |  | 58 | 97 | 84 | 101 | - | 102 |  |  | 61 | 100 | 88 | 101 | - | 102 |
| 20 |  |  | 50 | 88 | 73 | 96 | - | 97 |  |  | 53 | 95 | 76 | 96 | - | 97 |
| 21 |  |  | 44 | 79 | 63 | 91 | 85 | 92 |  |  | 46 | 90 | 66 | 91 | 89 | 92 |
| 22 |  |  |  |  | 55 | 87 | 74 | 88 |  |  | 40 | 80 | 58 | 87 | 78 | 88 |
| 23 |  |  |  |  | 49 | 80 | 65 | 84 |  |  |  |  | 51 | 83 | 69 | 84 |
| 24 |  |  |  |  | 43 | 73 | 58 | 80 |  |  |  |  | 45 | 80 | 61 | 80 |
| 25 |  |  |  |  |  |  | 51 | 77 |  |  |  |  | 40 | 76 | 54 | 77 |
| 26 |  |  |  |  |  |  | 46 | 72 |  |  |  |  |  |  | 48 | 74 |
| 27 |  |  |  |  |  |  | 41 | 67 |  |  |  |  |  |  | 43 | 71 |
| 28 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 29 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

- Total Load values are limited by shear, moment, or deflection equal to L/240.
- Live Load values are limited by deflection equal to L/480. For deflection limits of L/360 and L/960, multiply the Live Load values by 1.33 and 0.50 respectively.
- Both the Total Load and Live Load columns must be checked. Where a Live Load value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC CALC® ${ }^{\circledR}$ software if the length of any span is less than half the length of an adjacent span.
- Table values do not consider composite action from gluing and nailing floor sheathing (composite action is considered in floor span tables on page 4).
- Total Load values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- For assistance with floor design, consult the section About Floor Performance on page 4.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® ${ }^{\circledR}$ software.


## Allowable Uniform Floor Load <br> (in pounds per lineal foot [PLF])

| 100\% Load Duration |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AJS® 25 Series 3½" Flange Width |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | $\begin{gathered} 91 / 2 " \\ \text { AJS }{ }^{\text {® }} \end{gathered}$ |  | $\begin{gathered} 117 / 8 " \\ \text { AJS® } 25 \end{gathered}$ |  | AJS® 25 |  | $\begin{gathered} 16 " \\ \text { AJS® } 25 \end{gathered}$ |  | $\stackrel{18 \text { " }}{\text { AJS }}{ }^{\text {® }} 25$ |  | $\begin{gathered} 20 " \\ \text { AJS® } 25 \end{gathered}$ |  | $\stackrel{22^{\prime \prime}}{\text { AJS® }} 25$ |  | $\begin{gathered} \stackrel{24 "}{ } \\ \text { AJSS® } 25 \end{gathered}$ |  |
| Span Length | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load | $\begin{aligned} & \text { Live } \\ & \text { Load } \end{aligned}$ | Total Load | Live Load | Total Load | $\begin{array}{\|l\|} \hline \text { Live } \\ \text { Load } \end{array}$ | Total <br> Load | Live Load | Total Load |
| 6 | - | 316 | - | 318 | - | 320 | - | 323 | - | 629 | - | 681 | - | 697 | - | 712 |
| 7 | - | 271 | - | 272 | - | 274 | - | 277 | - | 539 | - | 584 | - | 597 | - | 610 |
| 8 | - | 237 | - | 238 | - | 240 | - | 242 | - | 472 | - | 511 | - | 523 | - | 534 |
| 9 | - | 211 | - | 212 | - | 213 | - | 215 | - | 419 | - | 454 | - | 464 | - | 475 |
| 10 | - | 190 | - | 191 | - | 192 | - | 194 | - | 377 | - | 408 | - | 418 | - | 427 |
| 11 | - | 172 | - | 173 | - | 174 | - | 176 | - | 343 | - | 371 | - | 380 | - | 388 |
| 12 | - | 158 | - | 159 | - | 160 | - | 161 | - | 314 | - | 340 | - | 348 | - | 356 |
| 13 | 136 | 146 | - | 146 | - | 147 | - | 149 | - | 290 | - | 314 | - | 321 | - | 328 |
| 14 | 111 | 135 | - | 136 | - | 137 | - | 138 | - | 269 | - | 292 | - | 298 | - | 305 |
| 15 | 92 | 126 | - | 127 | - | 128 | - | 129 | - | 251 | - | 272 | - | 278 | - | 285 |
| 16 | 77 | 118 | - | 119 | - | 120 | - | 121 | - | 236 | - | 255 | - | 261 | - | 267 |
| 17 | 65 | 111 | 107 | 112 | - | 112 | - | 114 | - | 222 | - | 240 | - | 246 | - | 251 |
| 18 | 55 | 105 | 91 | 106 | - | 106 | - | 107 | - | 209 | - | 227 | - | 232 | - | 237 |
| 19 | 47 | 95 | 78 | 100 | - | 101 | - | 102 | 197 | 198 | - | 215 | - | 220 | - | 225 |
| 20 | 41 | 82 | 68 | 95 | - | 96 | - | 97 | 171 | 188 | - | 204 | - | 209 | - | 213 |
| 21 |  |  | 59 | 90 | 85 | 91 | - | 92 | 150 | 179 | 187 | 194 | - | 199 | - | 203 |
| 22 |  |  | 52 | 86 | 74 | 87 | - | 88 | 132 | 171 | 165 | 185 | - | 190 | - | 194 |
| 23 |  |  | 46 | 83 | 66 | 83 | - | 84 | 116 | 164 | 146 | 177 | 178 | 181 | - | 185 |
| 24 |  |  | 40 | 79 | 58 | 80 | 78 | 80 | 103 | 152 | 129 | 170 | 158 | 174 | - | 178 |
| 25 |  |  |  |  | 52 | 76 | 69 | 77 | 92 | 140 | 115 | 157 | 141 | 167 | 170 | 171 |
| 26 |  |  |  |  | 46 | 73 | 62 | 74 | 82 | 129 | 103 | 145 | 127 | 159 | 153 | 164 |
| 27 |  |  |  |  | 41 | 71 | 56 | 71 | 74 | 120 | 93 | 134 | 114 | 147 | 137 | 158 |
| 28 |  |  |  |  |  |  | 50 | 69 | 66 | 111 | 84 | 125 | 103 | 137 | 124 | 149 |
| 29 |  |  |  |  |  |  | 45 | 66 | 60 | 104 | 76 | 116 | 93 | 127 | 112 | 139 |
| 30 |  |  |  |  |  |  | 41 | 64 | 54 | 97 | 69 | 109 | 84 | 119 | 102 | 130 |

## AJS® Floor Load Tables

Allowable Uniform Floor Load (in pounds per lineal foot [PLF])

## 100\% Load Duration

AJS® 30 Series
3½" Flange Width

| Span Length | $\begin{gathered} 18 " \\ \text { AJS® } 30 \end{gathered}$ |  | $\begin{gathered} 20 " \\ \text { AJS }{ }^{\circledR} 30 \\ \hline \end{gathered}$ |  | $\begin{gathered} 22^{\prime \prime} \\ \text { AJS } 30 \\ \hline \end{gathered}$ |  | $\begin{gathered} 24 " \\ \text { AJS® } 30 \end{gathered}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load | Live Load | Total Load |
| 10 | - | 377 | - | 408 | - | 418 | - | 427 |
| 11 | - | 343 | - | 371 | - | 380 | - | 388 |
| 12 | - | 314 | - | 340 | - | 348 | - | 356 |
| 13 | - | 290 | - | 314 | - | 321 | - | 328 |
| 14 | - | 269 | - | 292 | - | 298 | - | 305 |
| 15 | - | 251 | - | 272 | - | 278 | - | 285 |
| 16 | - | 236 | - | 255 | - | 261 | - | 267 |
| 17 | - | 222 | - | 240 | - | 246 | - | 251 |
| 18 | - | 209 | - | 227 | - | 232 | - | 237 |
| 19 | - | 198 | - | 215 | - | 220 | - | 225 |
| 20 | 186 | 188 | - | 204 | - | 209 | - | 213 |
| 21 | 163 | 179 | - | 194 | - | 199 | - | 203 |
| 22 | 144 | 171 | 179 | 185 | - | 190 | - | 194 |
| 23 | 127 | 164 | 159 | 177 | - | 181 | - | 185 |
| 24 | 113 | 157 | 141 | 170 | 172 | 174 | - | 178 |
| 25 | 100 | 151 | 126 | 163 | 154 | 167 | - | 171 |
| 26 | 90 | 145 | 113 | 157 | 138 | 160 | - | 164 |
| 27 | 81 | 139 | 101 | 151 | 124 | 154 | 149 | 158 |
| 28 | 73 | 134 | 92 | 146 | 112 | 149 | 135 | 152 |
| 29 | 66 | 130 | 83 | 140 | 102 | 144 | 122 | 147 |
| 30 | 60 | 120 | 75 | 136 | 92 | 139 | 111 | 142 |
| 32 | 50 | 100 | 62 | 121 | 77 | 130 | 93 | 133 |
| 34 | 42 | 84 | 52 | 105 | 65 | 117 | 78 | 125 |
| 36 |  |  | 44 | 89 | 55 | 105 | 66 | 114 |
| 38 |  |  |  |  | 47 | 94 | 57 | 102 |

## AJS® Roof Framing

## AJS ${ }^{\circledR}$ Rafters



## SAFETY WARNING

DO NOT ALLOW WORKERS ON AJS® JOISTS UNTIL ALL HANGERS, AJS® RIM JOISTS, RIM BOARDS, AJS® BLOCKING PANELS, X-BRACING AND TEMPORARY $1 x 4$ STRUT LINES ARE INSTALLED AS SPECIFIED BELOW.

SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

- Build a braced end wall at the end of the bay, or permanently install the first eight feet of AJS® Joists and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of AJS ${ }^{\circledR}$ Joists at the end of the bay.
- All hangers, AJS® ${ }^{\circledR}$ rim joists, rim boards, AJS ${ }^{\circledR}$ blocking panels, and x-bracing must be completely installed and properly nailed as each AJS® Joist is set.
- Install temporary $1 \times 4$ strut lines at no more than eight feet on center as additional AJS® Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS ${ }^{\circledR}$ Joist with two 8d nails.
- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS ${ }^{\circledR}$ Joist to within $1 / 2$ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.



Simpson VPA or USP TMP connectors or equal can be used in lieu of beveled plate for slopes from $3 / 12$ to $12 / 12$.


Double-beveled plate, connect to ridge with 2 rows 16 d nails at 12 " o.c.

23 of this guide.


R05 Simpson or USP LSTA24



Flange of AJS ${ }^{\circledR}$ Joists may be birdsmouth cut only at the low end of the joist. Birdsmouth cut AJS ${ }^{\oplus}$ joist must bear fully on plate, web stiffener required each side. Bottom flange shall be fully supported.

R03


R06 Simpson or USP LSTA24 strap where slope exceeds 7/12 (straps may be required for lower


## R07



Backer block required where top flange joist hanger load exceeds 250 lbs. Install tight to top flange.

## LATERAL SUPPORT

- AJS ${ }^{\circledR}$ Joists must be laterally supported at end supports (including supports adjacent to overhangs) with hangers, rimboard, or blocking (VERSA-LAM ${ }^{\circledR}$, BOISE CASCADE ${ }^{\circledR}$ Rimboard or AJS ${ }^{\circledR}$ Joist). Metal cross bracing or other x-bracing provides adequate lateral support for AJS® Joists, consult governing building code for roof diaphragm connection provisions.
- Blocking may be required at intermediate bearings for roof diaphragm per project's structural engineer of record.
MINIMUM BEARING LENGTH FOR AJS ${ }^{\circledR}$ JOISTS
- Minimum end bearing: $1 \frac{1}{2}$ " for $91 / 2^{\prime \prime}-16$ " deep joists, $13 / 4$ " for $18^{\prime \prime}$ and deeper joists. $3^{1 ⁄ 2} 2^{\prime \prime}$ minimum bearing length at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values. Refer to the building code evaluation report or the BC CALC ${ }^{\circledR}$ software.


## NAILING REQUIREMENTS

- AJS ${ }^{\circledR}$ rim joist, rim board or closure panel to AJS ${ }^{\circledR}$ joist:
- Rims or closure panel $1 \frac{1}{4}$ inches thick and less: 2-8d nails, one each in the top and bottom flange.
- AJS® 190/20 rim joist: 2-16d box nails, one each in the top and bottom flange.
- AJS® 25/30 rim joist: Toe-nail top flange to rim joist with 2-10d box nails, one each side of flange.
- AJS ${ }^{\circledR}$ rim joist, rim board or AJS ${ }^{\circledR}$ blocking panel to support:
- Min. 8d nails @ 6" o.c. per IBC.
- Connection per design professional of record's specification for shear transfer.
- AJS ${ }^{\circledR}$ joist to support:
- 2-8d nails, one on each side of the web, placed $11 / 2$ inches minimum from the end of the AJS ${ }^{\circledR}$ Joist to limit splitting.
- Sheathing to AJS ${ }^{\circledR}$ joist:
- Minimum nailing schedule 8d common nails @ 6" o.c. on edges and 12" 0.c. in the field, see IBC Table 2304.9.1 for other fastener options.
- See closest allowable nail spacing limits on page 24 for floor diaphragm nailing specified at closer spacing than IBC.
- Maximum bracing spacing for full lateral stability: 18 " for AJS ${ }^{\circledR} 190 \& 20,24 "$ for larger AJS ${ }^{\circledR}$ joist series.
- 14 gauge staples may be substituted for 8 d nails if the staples penetrate at least 1 inch into the joist.
- Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.
BACKER AND FILLER BLOCK DIMENSIONS

| AJS® Series | Backer Block Thickness | Filler Block Thickness |
| :---: | :---: | :---: |
| $\begin{array}{r} 190 \\ 20 \end{array}$ | $11 / 8^{\prime \prime}$ or two $1 / 22^{\prime \prime}$ wood panels | $2 x_{-}+5 / 8$ " wood panel |
| $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | 2 x _ lumber | Double $2 \mathrm{x}_{\text {_ }}$ lumber |

- Cut backer and filler blocks to a maximum depth equal to the web depth minus $1 / 4^{\prime \prime}$ to avoid a forced fit.


## WEB STIFFENER REQUIREMENTS

- See Web Stiffener Requirements on page 18.


## PROTECT AJS® JOISTS FROM THE WEATHER

- AJS ${ }^{\circledR}$ Joists are intended only for applications that provide permanent protection from the weather. Bundles of AJS ${ }^{\circledR}$ Joists should be covered and stored off of the ground on stickers.


## MAXIMUM SLOPE

- Unless otherwise noted, all roof details are valid for slopes of 12 in 12 or less.


## VENTILATION

- The $11 / 2$ inch, pre-stamped knock-out holes spaced at 12 inches on center along the AJS® Joist may all be knocked out and used for cross ventilation. Deeper joists than what is structurally needed may be advantageous in ventilation design. Consult local building official and/or ventilation specialist for specific ventilation requirements.


## BIRDSMOUTH CUTS

- AJS ${ }^{\circledR}$ Joists may be birdsmouth cut only at the low end support. AJS ${ }^{\circledR}$ joists with birdsmouth cuts may cantilever up to $2^{\prime}-6^{\prime \prime}$ past the low end support. The bottom flange must sit fully on the support and may not overhang the inside face of the support. High end supports and intermediate supports may not be birdsmouth cut.

Maximum clear span in feet and inches, based on horizontal spans.

| 115\% and 125\% Load Duration |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | AJS® 20 Series 2½" Flange Width |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | $\begin{gathered} 91 / 2 " \\ \text { AJS }{ }^{\circledR} 20 \end{gathered}$ |  |  | $\begin{gathered} 117 / 8 " \\ \text { AJS® } 20 \end{gathered}$ |  |  | $\begin{gathered} \stackrel{14 "}{ } \\ \text { AJS } \end{gathered}$ |  |  | $\begin{gathered} 16 " \\ \text { AJS }{ }^{\circledR} 20 \end{gathered}$ |  |  |
|  |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \\ & \text { [psf] } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { Dead } \\ & \text { Load } \\ & \text { [psf] } \\ & \hline \end{aligned}$ | $\begin{aligned} & 4 / 12 \\ & \text { or } \\ & \text { Less } \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{aligned} & 4 / 12 \\ & \text { or } \\ & \text { Less } \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ |
| 12" o.c. | $\begin{aligned} & \text { Non- } \\ & \text { Snow } \\ & \text { 125\% } \end{aligned}$ | 20 | 10 | 27'-4" | 25'-9" | 23'-11" | 32'-7" | 30'-9" | 28'-7" | 37'-1" | 35'-0" | 32'-6" | 41'-2" | 38'-10" | 36'-0" |
|  |  | 20 | 15 | 25'-10" | 24'-3" | 22'-5" | 30'-11" | 29'-0" | 26'-9" | 35'-2" | 33'-0" | 30'-5" | 39'-0" | 36'-7" | 33'-9" |
|  |  | 20 | 20 | 24'-8" | 23'-1" | 21'-3" | 29'-6" | 27'-7" | 25'-4" | 33'-6" | 31'-5" | 28'-10" | 37'-2" | 34'-10" | 32'-0" |
|  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | 25 | 10 | 25'-11" | 24'-6" | 22'-10" | 31'-0" | 29'-3" | 27'-3" | 35'-3" | $33^{\prime}-4 "$ | 31'-0" | 39'-1" | 36'-11" | 34'-4" |
|  |  | 25 | 15 | 24'-9" | 23'-3" | 21'-6" | 29'-7" | 27'-10" | 25'-9" | 33'-7" | 31'-8" | 29'-4" | 36'-11" | 35'-1" | 32'-6" |
|  |  | 30 | 10 | 24'-9" | 23'-6" | 21'-11" | 29'-7" | 28'-0" | 26'-2" | 33'-8" | 31'-11" | 29'-9" | 37'-1" | 35'-4" | $33^{\prime}-0{ }^{\prime \prime}$ |
|  |  | 30 | 15 | 23'-9" | 22'-5" | 20'-10" | 28'-5" | 26'-9" | 24'-10" | 32'-4" | 30'-6" | 28'-3" | 34'-10" | 33'-10' | 31'-4" |
|  |  | 40 | 10 | 22'-6" | 21'-7" | 20'-5" | 26'-11" | 25'-10" | 24'-5" | 30'-8" | 29'-5" | 27'-9" | 33'-2" | 32'-7" | 30'-9" |
|  |  | 40 | 15 | 22'-2" | 21'-0" | 19'-7" | 26'-6" | 25'-1" | 23'-5" | 29'-3" | 28'-7" | 26'-7" | 31'-7" | 30'-11" | 29'-6" |
|  |  | 50 | 10 | 20'-10" | 20'-0" | 19'-0" | 24'-11" | 23'-11" | 22'-9" | 28'-1" | 27'-3" | 25'-10" | 30'-3" | 29'-11" | 28'-8" |
|  |  | 50 | 15 | 20'-10" | 19'-11" | 18'-7" | 24'-6" | 23'-9" | 22'-3" | 26'-11" | 26'-6" | 25'-3" | 29'-0" | 28'-7" | 27'-11" |
| 16" o.c. | Non- <br> Snow <br> 125\% | 20 | 10 | 24'-9" | 23'-4" | 21'-8" | 29'-7" | 27'-11" | 25'-11" | 33'-8' | 31'-9" | 29'-5" | $37^{\prime}-4{ }^{\prime \prime}$ | 35'-2" | 32'-8" |
|  |  | 20 | 15 | 23'-5" | 22'-0" | 20'-4" | 28'-0" | 26'-4" | 24'-3" | 31'-10" | 29'-11" | 27'-7" | 35'-4" | 33'-2' | 30'-7" |
|  |  | 20 | 20 | 22'-4" | 20'-11" | 19'-3" | 26'-8" | 25'-0" | 23'-0" | 30'-5" | 28'-5" | 26'-2" | 33'-3" | 31'-7" | 29'-0" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 23'-6" | 22'-2" | 20'-8" | 28-1" | 26'-6" | 24'-8" | 31'-10" | 30'-2" | 28'-1" | $34^{\prime}-3^{\prime \prime}$ | 33'-6" | 31'-2" |
|  |  | 25 | 15 | 22'-5" | 21'-1" | 19'-6" | 26'-9" | 25'-2" | 23'-4" | 29'-8" | 28'-8" | 26'-7" | 32'-0" | 31'-1" | 29'-5" |
|  |  | 30 | 10 | 22'-5" | 21'-3" | 19'-10" | 26'-10" | 25'-5" | 23'-9" | 29'-9" | 28'-11" | 27'-0" | 32'-1" | 31'-6" | 29'-11" |
|  |  | 30 | 15 | 21'-6" | 20'-4" | 18'-10" | 25'-6" | 24'-3" | 22'-6" | 28'-0" | 27'-4" | 25'-8" | 30'-2" | 29'-5" | 28'-5" |
|  |  | 40 | 10 | 20'-5" | 19'-7" | 18'-6" | 24'-3" | 23'-5" | 22'-1" | 26'-8" | 26'-3" | 25'-2" | 28'-8" | 28'-3" | 27'-9" |
|  |  | 40 | 15 | 20'-1" | 19'-0" | 17'-9" | 23'-1" | 22'-7" | 21'-2" | 25'-4" | 24'-10" | 24'-1" | 27'-3" | 26'-9" | 26'-1" |
|  |  | 50 | 10 | 18'-11" | 18'-1" | 17'-2" | 22'-2" | 21'-8" | 20'-7" | 24'-4" | 24'-0" | 23'-5" | 26'-2' | 25'-11" | 25'-5" |
|  |  | 50 | 15 | 18'-7" | 18'-0" | 16'-10" | 21'-3" | 20'-10" | 20'-1" | 23'-4" | 22'-11" | 22'-5" | 25'-1" | 24'-8" | 23'-7" |
| 19.2" o.c. | Non- <br> Snow <br> 125\% | 20 | 10 | 23'-3" | 21'-11" | 20'-4" | 27'-9" | 26'-2" | 24'-4" | 31'-7" | 29'-10" | 27'-8" | 35'-1" | 33'-1" | 30'-8" |
|  |  | 20 | 15 | 22'-0" | 20'-8" | 19'-1" | 26'-4" | 24'-8" | 22'-10" | 29'-11" | 28'-1" | 25'-11" | 32'-6" | 31'-2" | 28'-9" |
|  |  | 20 | 20 | 21'-0" | 19'-8" | 18'-1" | 25'-1" | 23'-6" | 21'-7" | 28'-1" | 26'-9" | 24'-7" | $30^{\prime}-4 "$ | 29'-3" | 27'-3" |
|  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | 25 | 10 | 22'-1" | 20'-10" | 19'-5" | 26'-4" | 24'-11" | 23'-3" | 29'-0" | 28'-4" | 26'-5" | 31'-3" | 30'-7" | 29'-3" |
|  |  | 25 | 15 | 21'-0" | 19'-10" | 18'-4" | 24'-8" | 23'-8" | 21'-11" | 27'-1" | 26'-4" | 24'-11" | 29'-2" | 28'-5" | 27'-5" |
|  |  | 30 | 10 | 21'-1" | 19'-11" | 18'-8" | 24'-9" | 23'-10" | 22'-3" | 27'-2" | 26'-8" | 25'-4" | 29'-3" | 28'-9" | 28'-0" |
|  |  | 30 | 15 | 20'-2' | 19'-1" | 17'-8" | 23'-3" | 22'-8" | 21'-2" | 25'-6" | 24'-11" | 24'-1" | 27'-6" | 26'-10" | 26'-0" |
|  |  | 40 | 10 | 19'-2" | 18'-5" | 17'-4" | 22'-1" | 21'-10" | 20'-9" | 24'-3" | 23'-11" | 23'-5" | 26'-2" | 25'-9" | 25'-3" |
|  |  | 40 | 15 | 18'-5" | 17'-10" | 16'-8" | 21'-0" | 20'-7" | 19'-11" | 23'-1" | 22'-8" | 22'-0" | 24'-11" | 24'-2" | 22'-11" |
|  |  | 50 | 10 | 17'-8" | 17'-0" | 16'-2" | 20'-2" | 19'-11" | 19'-4" | 22'-2" | 21'-11" | 21'-6" | 23'-2" | 22'-7" | 21'-10" |
|  |  | 50 | 15 | 17'-0" | 16'-8" | 15'-9" | 19'-4" | 19'-0" | 18'-7" | 21'-0" | 20'-3" | 19'-4" | 21'-3" | 20'-7" | 19'-8" |
| 24" o.c. | Non- <br> Snow <br> 125\% | 20 | 10 | 21'-6" | 20'-4" | 18'-10" | 25'-9" | 24'-3" | 22'-6" | 29'-2' | 27'-7" | 25'-8" | 31'-5" | 30'-8" | 28'-5" |
|  |  | 20 | 15 | 20'-4" | 19'-1" | 17'-8" | 24'-4" | 22'-10" | 21'-1" | 26'-11" | 26'-0" | 24'-0" | 29'-0" | 28'-2" | 26'-8" |
|  |  | 20 | 20 | 19'-5" | 18'-2" | 16'-9" | 22'-11" | 21'-9" | 20'-0" | 25'-2" | 24'-3" | 22'-9" | 27'-1" | 26'-2" | 24'-11" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 20'-5" | 19'-4" | 18'-0" | 23'-7" | 23'-1" | 21'-6" | 25'-11" | 25'-5" | 24'-6" | 27'-11" | 27-4" | 26'-7" |
|  |  | 25 | 15 | 19'-4" | 18'-4" | 17'-0" | 22'-0" | 21'-5" | 20'-4" | 24'-2" | 23'-6" | 22'-8" | 26'-1" | 25'-4" | 24'-3" |
|  |  | 30 | 10 | 19'-4" | 18'-6" | 17'-3" | 22'-1" | 21-8" | 20'-8" | 24'-3" | 23'-10" | 23'-3" | 26'-2" | 25'-8" | 25'-0" |
|  |  | 30 | 15 | 18'-2" | 17'-8" | 16'-5" | 20'-9" | 20'-3" | 19'-7" | 22'-10" | 22'-3" | 21'-7" | 24'-6" | 23'-4" | 21'-11" |
|  |  | 40 | 10 | 17'-4" | 17'-0" | 16'-1" | 19'-9" | 19'-6" | 19'-1" | 21'-8" | 21'-3" | 20'-5" | 22'-2" | 21'-6" | 20'-8" |
|  |  | 40 | 15 | 16'-6" | 16'-2" | 15'-5" | 18'-9" | 18'-5" | 17'-8" | 19'-9" | 19'-0" | 18'-0" | 20'-1" | 19'-3" | 18'-3" |
|  |  | 50 | 10 | 15'-10" | 15'-7" | $14^{\prime}-11{ }^{\prime \prime}$ | 17'-11" | 17'-6" | 16'-11" | 18'-2" | 17'-9" | 17'-2" | 18'-6" | 18'-0" | 17'-5" |
|  |  | 50 | 15 | 15'-2" | 14'-11" | 14'-7" | 16'-5" | 15'-11" | 15'-2" | 16'-9" | 16'-2" | 15'-5" | 17'-0" | 16'-5" | 15'-8" |

- Table values are limited by shear, moment, total load deflection equal to $\mathrm{L} / 180$ and live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Table values represent the most restrictive of simple or multiple span applications. Analyze multiple span joists with the BC Calc software if the length of any span is less than half the length of an adjacent span.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ software.
- Slope roof joists at least $1 / 4$ " over 12 " to minimize ponding.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

Maximum clear span in feet and inches, based on horizontal spans.
115\% and 125\% Load Duration

|  |  |  |  | AJS® 190 Series 2½" Flange Width |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | $\begin{gathered} 911 / 2 \\ \text { AJS }{ }^{9} 190 \end{gathered}$ |  |  | $11^{7} / 8^{\prime \prime}$ <br> AJS® 190 |  |  | $\begin{gathered} 14 " \\ \text { AJS® } 190 \end{gathered}$ |  |  | $\begin{gathered} 16^{\prime \prime} \\ \text { AJS® } 190 \end{gathered}$ |  |  |
|  |  | $\begin{aligned} & \hline \text { Live } \\ & \text { Load } \\ & \text { [psf] } \end{aligned}$ | $\begin{aligned} & \hline \text { Dead } \\ & \text { Load } \\ & \text { [psf] } \\ & \hline \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{aligned} & 4 / 12 \\ & \text { or } \\ & \text { Less } \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ |
| 12" o.c. | NonSnow 125\% | 20 | 10 | 27'-9" | 26'-2" | 24'-3" | 33'-2' | 31'-3" | 29'-0" | 37'-9" | 35'-7" | 33'-0" | 41'-10" | 39'-6" | $36^{\prime}-7{ }^{\prime \prime}$ |
|  |  | 20 | 15 | 26'-3' | 24'-8" | 22'-9" | 31'-5" | 29'-6" | 27'-3" | 35'-9" | 33'-7" | 31'-0" | 39'-8" | 37'-3" | $34^{\prime}-4 "$ |
|  |  | 20 | 20 | 25'-1" | 23'-6" | 21-7" | 30'-0" | 28'-1" | 25'-9" | $34{ }^{\prime \prime}{ }^{\prime \prime}$ | 31'-11" | 29'-4" | 37'-10" | 35'-5" | $32^{\prime}-7{ }^{\prime \prime}$ |
|  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | 25 | 10 | 26'-4" | 24'-11" | 23'-2" | 31'-6" | 29'-9" | 27-8" | 35'-10" | 33'-10" | 31'-6" | 39'-9" | 37'-7" | 34'-11" |
|  |  | 25 | 15 | 25'-2' | 23'-8" | 21'-11" | 30'-0" | 28'-3" | 26'-2" | 34'-2" | 32'-2" | 29'-9" | 37'-11" | 35'-8" | 33'-0" |
|  |  | 30 | 10 | 25'-2' | 23'-10" | 22'-3" | 30'-1" | 28'-6" | 26'-7" | 34'-3" | 32'-5" | 30'-3" | 38'-0" | 36'-0" | 33'-7" |
|  |  | 30 | 15 | 24'-2' | 22'-9" | 21'-2" | 28'-10" | 27'-3" | 25'-3" | 32'-10" | 31'-0" | 28'-9" | 36'-5" | 34'-4" | 31'-11" |
|  |  | 40 | 10 | 22'-11" | 22'-0" | 20'-9" | 27'-5" | 26'-3" | 24'-10" | 31'-2' | 29'-11" | 28'-3" | 34'-7" | 33'-2" | 31'-4" |
|  |  | 40 | 15 | 22'7" | 21-4" | 19'-11" | 27-0" | $25^{\prime \prime}-6{ }^{\prime \prime}$ | 23'-9" | 30'-8" | 29'-1" | 27'-1" | 33'-10" | 32'-3" | 30'-0" |
|  |  | 50 | 10 | 21'-3" | 20'-4" | 19'-4" | 25'-4" | $24^{\prime}-4^{\prime \prime}$ | 23'-1" | 28'-11" | 27'-8" | $26^{\prime}-3$ " | 32'-0" | 30'-9" | 29'-2" |
|  |  | 50 | 15 | 21'-3" | 20'-3" | 18'-11" | 25'-4" | $24^{\prime}-2$ " | 22-7" | 28'-10" | 27'-6" | 25'-8" | $31^{\prime \prime}$-1" | 30'-6" | 28'-6" |
| 16" o.c. | Non- <br> Snow <br> 125\% | 20 | 10 | 25'-2' | 23'-9" | 22'-0" | 30'-1" | 28'-4" | 26'-4" | 34'-2" | 32'-3" | 29'-11" | 37'-11" | 35'-9" | 33'-2" |
|  |  | 20 | 15 | 23'-10" | 22'-4" | 20'-8" | 28'-6" | 26'-9" | 24'-8" | 32'-5" | 30'-5" | 28'-1" | 35'-11" | 33'-9" | 31-2" |
|  |  | 20 | 20 | 22'-9" | 21'-3" | 19'-7" | 27'-2" | 25'-5" | 23'-4" | $30^{\prime}-11^{\prime \prime}$ | 28'-11" | 26'-7" | 34'-3" | 32'-1" | 29'-6" |
|  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | 25 | 10 | 23'-10" | 22'-7" | 21'-0" | 28'-6" | 27'-0" | 25'-1" | 32'-6" | 30'-8" | 28'-7" | 36'-0" | 34'-1" | 31-8" |
|  |  | 25 | 15 | 22'-9" | 21'-5" | 19'-10" | 27'-2" | 25'-7" | 23'-9" | $31^{\prime}-0{ }^{\prime \prime}$ | 29'-2" | 27'-0" | 34'-3" | 32-4" | 29'-11" |
|  |  | 30 | 10 | 22'-10" | 21'-7" | 20'-2" | 27-3" | 25'-10" | 24'-1" | 31'-0" | 29'5" | 27'-5" | 34'-4" | 32'-7" | 30'-5" |
|  |  | 30 | 15 | 21'-10" | 20'-8" | 19'-2" | 26'-2" | 24'-8" | 22'-11" | 29'-9" | 28'-1" | 26'-1" | 32'-4" | 31'-2" | 28'-11" |
|  |  | 40 | 10 | 20'-9" | 19'-11" | 18'-10" | 24'-10" | 23'-10" | 22'-6" | 28'-3' | 27'-1" | 25'-7" | 30'-9" | 30'-1" | 28'-5" |
|  |  | 40 | 15 | 20'-5" | 19'-4" | 18'-0" | 24'-5" | 23'-1" | 21'-7" | 27'-2" | 26'-4" | 24'-6" | 29'-3" | 28'-8" | 27'-2" |
|  |  | 50 | 10 | 19'-2" | 18'-5" | 17'-6" | 22'-11" | 22'-0" | 20'-11" | 26'-1" | 25-1" | 23'-10" | 27'-10" | 27-2" | 26'-3" |
|  |  | 50 | 15 | 19'-2" | $18^{\prime}-4{ }^{\prime \prime}$ | 17'-1" | 22'-9" | 21'-11" | 20'-5" | 25'-0" | 24'-5" | 23'-3" | 25'-7" | 24'-9" | 23'-7" |
| 19.2" o.c. | Non- <br> Snow <br> 125\% | 20 | 10 | 23'-7" | 22'-3" | 20'-8" | 28'-3" | 26'-8" | 24'-9" | 32'-2" | 30'-4" | 28'-2" | 35'-8" | 33'-7" | 31'-3" |
|  |  | 20 | 15 | 22'-4" | 21'-0" | 19'-5" | 26'-9" | 25'-1" | 23'-2" | 30'-5" | 28'-7" | 26'-5" | 33'-9" | 31'-8" | 29'-3" |
|  |  | 20 | 20 | 21'-4" | 20'-0" | 18'-4" | 25'-6" | 23'-10" | 21'-11" | 29'-0" | 27'-2" | 25'-0" | 32'-2" | 30'-2" | 27'-9" |
|  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | 25 | 10 | 22'-5" | 21'-2" | 19'-9" | 26'-10" | 25'-4" | 23'-7" | 30'-6" | 28'-10" | 26'-10" | 33'-6" | 32'-0" | 29'-9" |
|  |  | 25 | 15 | 21'-4" | 20'-2" | 18-8" | 25-7" | 24'-1" | 22-3" | 29'-0" | 27'-5" | 25'-4" | $31^{\prime}-3 "$ | 30'-5" | 28'-2" |
|  |  | 30 | 10 | 21'-5" | 20'-4" | 18'-11" | 25'-7" | 24'-3" | 22'-8" | 29'-1" | 27'-7" | 25'-9" | 31'-4" | 30'-8" | 28'-7" |
|  |  | 30 | 15 | 20'-6" | 19'-5" | 18'-0" | 24'-7" | 23'-2" | 21'-6" | 27'-4" | 26'-4" | 24'-6" | 29'-6" | 28'-9" | 27'-2" |
|  |  | 40 | 10 | 19'-6" | $18^{\prime}-8{ }^{\prime \prime}$ | 17'-8" | 23'-3" | 22'-4" | 21'-1" | 26'-0" | 25'-5" | 24'-0" | 27'-9" | 27'-0" | 25'-11" |
|  |  | 40 | 15 | 19'-2" | $18^{\prime}-2{ }^{\prime \prime}$ | 16'-11" | 22'-6" | 21'-8" | 20'-3" | 24'-9" | 23'-10" | 22'-7" | 25'-2" | 24'-2" | 22'-11" |
|  |  | 50 | 10 | 18'-0" | 17'-3" | 16'-5" | 21'-6" | 20'-8" | 19-8" | 22'-10" | 22-3" | 21'-6" | 23'-2" | 22-7" | 21'-10" |
|  |  | 50 | 15 | 18'-0" | 17'-2" | 16'-1" | 20'-7" | 19'-11" | 19'-0" | 21'-0" | 20'-3" | 19'-4" | 21'-3" | 20'-7" | 19'-8" |
| 24" o.c. | Non- <br> Snow <br> 125\% | 20 | 10 | 21'-10" | 20'-8" | 19'-2" | 26'-2" | 24'-8" | 22'-11" | 29'-9" | 28'-1" | 26'-1" | 33'-0" | 31'-2" | 28'-11" |
|  |  | 20 | 15 | 20'-8" | 19'-5" | 18'-0" | 24'-9" | 23'-3" | 21'-6" | 28'-2' | 26'-6" | 24'-5" | 31'-1" | 29'-4" | 27'-1" |
|  |  | 20 | 20 | 19'-9" | 18'-6" | 17'-0" | 23'-7" | 22'-1" | 20'-4" | 26'-10" | 25'-2" | 23'-2" | 29'-0" | 27'-9" | 25'-3" |
|  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | 25 | 10 | 20'-9" | 19'-8" | 18'-3" | 24'-10" | 23'-6" | 21'-10" | 27'-9" | 26'-9" | 24'-11" | 29'-11" | 29'-4" | 27'-7" |
|  |  | 25 | 15 | 19'-9" | 18'-8" | 17'-3" | 23'-7" | 22'-3" | 20'-8" | 25'-11" | 25'-3" | 23'-6" | 27'-6" | 26'-1" | 24'-3" |
|  |  | 30 | 10 | 19'-10" | 18'-9" | 17'-6" | 23'-8" | 22'-6" | 21'-0" | 26'-0" | 25'-6" | 23'-10" | 27'-9" | 26'-9" | 25'-5" |
|  |  | 30 | 15 | 19'-0" | 17'-11" | 16'-8" | 22'-3" | 21'-5" | 19'-11" | 24'-2' | 23'-0" | 21-7" | 24'-6" | 23'-4" | 21'-11" |
|  |  | 40 | 10 | 18'-0" | 17'-3" | 16'-4" | 21'-2" | 20'-8" | 19'-7" | 21'-10" | 21-3" | 20'-5" | 22'-2" | 21'-6" | 20'-8" |
|  |  | 40 | 15 | 17'-8" | 16'-9" | 15'-8" | 19'-5" | 18'-8" | 17'-8" | 19'-9" | 19'-0" | 18'-0" | 20'-1" | 19'-3" | 18'-3" |
|  |  | 50 | 10 | 16'-8" | $16^{\prime}-0 "$ | 15'-2" | 17'-11" | 17'-6" | 16'-11" | 18'-2' | 17'-9" | 17'-2" | 18'-6" | 18'-0" | 17'-5" |
|  |  | 50 | 15 | $16^{\prime}-2{ }^{\prime \prime}$ | $15^{\prime}-8{ }^{\prime \prime}$ | 14'-10" | 16'-5" | 15'-11" | 15'-2" | 16'-9" | 16'-2" | 15'-5" | 17'-0" | 16'-5" | $15^{\prime}-8{ }^{\prime \prime}$ |

- Table values are limited by shear, moment, total load deflection equal to L/180 and live load deflection equal to L/240. Check the local building code for other deflection limits that may apply
- Table values represent the most restrictive of simple or multiple span applications. Analyze multiple span joists with the BC Calc software if the length of any span is less than half the length of an adjacent span.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less. 18" joists require web stiffeners at all bearing locations.

This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ software.

- Slope roof joists at least $1 / 4$ " over 12 " to minimize ponding.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

Maximum clear span in feet and inches, based on horizontal spans.

| 115\% and 125\% Load Duration |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | AJS® 25 Series - 91⁄2" - 16" Depths 3/8" Web Thickness - $31 / 2^{\prime \prime}$ Flange Width |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | $\begin{gathered} 91 / 2 " \\ \text { AJS }{ }^{\circledR} 25 \end{gathered}$ |  |  | $\begin{gathered} 117 / "^{10} \\ \text { AJSS } 25 \end{gathered}$ |  |  | $\begin{gathered} 14 " \\ \text { AJS® } 25 \end{gathered}$ |  |  | $\begin{gathered} 16 " \\ \text { AJS® } 25 \end{gathered}$ |  |  |
|  |  | $\begin{array}{\|l} \hline \text { Live } \\ \text { Load } \\ \text { [psf] } \end{array}$ | $\begin{array}{\|c\|} \hline \text { Dead } \\ \text { Load } \\ \text { [psf] } \end{array}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{aligned} & 4 / 12 \\ & \text { or } \\ & \text { Less } \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{aligned} & 4 / 12 \\ & \text { or } \\ & \text { Less } \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ |
| $\begin{aligned} & 12 " 1 \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & \text { Non- } \\ & \text { Snow } \\ & \text { 125\% } \end{aligned}$ | 20 | 10 | 30'-5" | 28'-9" | 26'-8" | 36'-4" | 34'-3" | 31'-10" | 41'-4" | 38'-11" | 36'-2" | 45'-9" | $43^{\prime}-2{ }^{\prime \prime}$ | 40'-1" |
|  |  | 20 | 15 | 28'-10" | 27'-1" | 25'-0" | 34'-5" | 32'-4" | 29'-10" | 39'-1" | 36'-9" | 33'-11" | $43^{\prime}-4{ }^{\prime \prime}$ | 40'-8" | $37^{\prime}-7{ }^{\prime \prime}$ |
|  |  | 20 | 20 | 27'-6" | 25'-9" | 23'-8" | 32'-10" | 30'-9" | 28'-3" | $37^{\prime}-4{ }^{\prime \prime}$ | 34'-11" | 32'-2" | $41^{\prime}-4^{\prime \prime}$ | 38'-9" | $35^{\prime}-7{ }^{\prime \prime}$ |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 28'-11" | 27'-4" | 25'-5" | 34'-6" | 32 -7" | 30'-4" | 39'-3" | 37'-1" | $34^{\prime}-6{ }^{\prime \prime}$ | $43^{\prime}-5^{\prime \prime}$ | 41'-1" | 38'-3" |
|  |  | 25 | 15 | 27'-7" | 25'-11" | 24'-0" | 32'-11" | 31'-0" | 28'-8" | $37^{\prime}-5^{\prime \prime}$ | 35'-3" | 32'-7" | $41^{\prime}-5^{\prime \prime}$ | 39'-0" | 36'-2" |
|  |  | 30 | 10 | 27'-8" | 26'-2" | 24'-5" | 33'-0" | $31{ }^{\prime}-3 "$ | 29'-2" | $37^{\prime}-6{ }^{\prime \prime}$ | 35'-6" | 33'-2" | $41^{\prime}-6{ }^{\prime \prime}$ | 39'-4" | 36'-8" |
|  |  | 30 | 15 | 26'-6" | 25'-0" | 23'-2" | $31^{\prime \prime} 8^{\prime \prime}$ | 29'-10" | 27'-8" | 35'-11" | 33'-11" | 31'-6" | 39'-10" | 37'-7" | 34'-11" |
|  |  | 40 | 10 | 25'-2" | 24'-1" | 22'-9" | $30^{\prime}-0^{\prime \prime}$ | 28'-9" | 27'-2" | 34'-1" | 32'-9" | $30^{\prime}-11^{\prime \prime}$ | 37'-10" | 36'-3" | $34^{\prime}-3{ }^{\prime \prime}$ |
|  |  | 40 | 15 | 24'-9" | 23'-5" | 21'-10" | 29'-7" | 28'-0" | 26'-1" | 33'-7" | 31'-10" | 29'-8" | $37^{\prime}-3^{\prime \prime}$ | $35^{\prime}-3$ " | 32'-10" |
|  |  | 50 | 10 | 23'-3" | 22'-4" | 21'-2" | 27'-9" | 26'-8" | 25'-4" | 31'-7" | $30^{\prime}-4 "$ | 28'-9" | 35'-0" | 33'-7" | 31'-11" |
|  |  | 50 | 15 | 23'-3" | 22'-2" | 20'-9" | 27'-9" | 26'-6" | 24'-9" | $31{ }^{\prime}-7{ }^{\prime \prime}$ | 30'-1" | 28'-2" | $35^{\prime}-0^{\prime \prime}$ | $33^{\prime}-4{ }^{\prime \prime}$ | 31'-2" |
| $\begin{aligned} & \text { 16" } \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & \text { Non- } \\ & \text { Snow } \\ & 125 \% \end{aligned}$ | 20 | 10 | 27'-7" | 26'-0" | 24'-2" | 32'-11" | 31'-1" | 28'-10" | 37'-5" | 35'-4" | 32'-9" | 41'-6" | 39'-2" | 36'-4" |
|  |  | 20 | 15 | 26'-1" | 24'-6" | 22'-8" | 31'-2' | 29'-3" | 27'-1" | 35'-5" | 33'-4" | 30'-9" | 39'-3" | 36'-11" | $34^{\prime}-1{ }^{\prime \prime}$ |
|  |  | 20 | 20 | 24'-11" | 23'-4" | $21^{\prime \prime} \mathbf{5}^{\prime \prime}$ | 29'-9" | 27'-10" | 25'-7" | $33^{\prime}-10{ }^{\prime \prime}$ | 31 '-8" | 29'-1" | $37^{\prime}-6 "$ | 35'-1" | 32'-3" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 26'-2" | 24'-9" | 23'-1" | 31'-3" | 29'-7" | 27'-6" | 35'-6" | $33^{\prime}-7{ }^{\prime \prime}$ | 31'-3" | $39^{\prime}-5^{\prime \prime}$ | 37'-3" | 34'-8" |
|  |  | 25 | 15 | 25'-0" | 23'-6" | 21'-9" | 29'-10" | 28'-1" | 26'-0" | 33'-11" | 31'-11" | 29'-7" | 37'-7" | $35^{\prime}-4{ }^{\prime \prime}$ | 32'-9" |
|  |  | 30 | 10 | $25^{\prime}-0{ }^{\prime \prime}$ | 23'-8" | 22'-1" | 29'-10" | 28'-4" | 26'-5" | 34'-0" | 32'-2" | 30'-0" | 37'-8" | 35'-8" | 33'-3" |
|  |  | 30 | 15 | 24'-0" | 22'-8" | 21'-0" | 28'-8" | 27'-0" | 25'-1" | 32'-7" | 30'-9" | 28'-6" | 36'-1" | 34'-1" | 31'-7" |
|  |  | 40 | 10 | 22'-9" | 21'-10" | 20'-7" | 27'-2" | 26'-1" | 24'-8" | $33^{\prime}-11^{\prime \prime}$ | 29'-8" | 28'-0" | $34^{\prime}-3^{\prime \prime}$ | 32'-10" | 31'-0" |
|  |  | 40 | 15 | 22'-5" | 21'-2" | 19'-9" | 26'-9" | 25'-4" | 23'-7" | 30'-5" | 28'-10" | 26'-10" | $33^{\prime}-8{ }^{\prime \prime}$ | 31'-11" | 29'-9" |
|  |  | 50 | 10 | 21'-0" | 20'-2" | 19'-2" | 25'-2" | 24'-2" | 22'-11" | 28'-7" | 27'-5" | 26'-1" | 31'-8" | 30'-5" | 28'-11" |
|  |  | 50 | 15 | 21'-0" | 20'-1" | 18'-9" | 25'-2" | 24'-0" | 22'-5" | 28'-7" | 27'-3" | 25'-6" | 29'-7" | 28'-8" | $27^{\prime}-4{ }^{\prime \prime}$ |
| $\begin{array}{\|l\|} 19.2^{\prime \prime} \\ \text { o.c. } \end{array}$ | Non- <br> Snow <br> 125\% | 20 | 10 | 25'-11" | 24'-5" | 22'-8" | 30'-11" | 29'-2" | 27'-1" | 35'-2" | 33'-2" | 30'-10" | 39'-0" | 36'-9" | 34'-2" |
|  |  | 20 | 15 | 24'-6" | 23'-0" | 21'-3" | 29'-3" | 27'-6" | 25'-5" | 33'-3" | 31'-3" | 28'-11" | 36'-11" | 34'-8" | 32'-0" |
|  |  | 20 | 20 | 23'-5" | 21'-11" | 20'-2" | 27'-11" | 26'-2" | 24'-1" | 31'-9" | 29'-9" | 27'-4" | $35^{\prime}-2^{\prime \prime}$ | 32'-11" | $30^{\prime}-4{ }^{\prime \prime}$ |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 24'-7" | 23'-3" | 21'-8" | 29'-4" | 27'-9" | 25'-10" | $33^{\prime}-5{ }^{\prime \prime}$ | $31^{\prime}-7{ }^{\prime \prime}$ | 29'-5" | $37^{\prime}-0{ }^{\prime \prime}$ | 35'-0" | 32'-7" |
|  |  | 25 | 15 | 23'-5" | 22'-1" | 20'-5" | 28'-0" | 26'-4" | 24'-5" | 31'-10" | $30^{\prime}-0$ " | 27'-9" | $35^{\prime}-3 "$ | $33^{\prime}-3 "$ | 30'-9" |
|  |  | 30 | 10 | 23'-6" | 22'-3" | 20'-9" | 28'-1" | 26'-7" | 24'-10" | 31'-11" | $30^{\prime}-3 "$ | 28'-3" | $35^{\prime}-4{ }^{\prime \prime}$ | $33^{\prime}-6{ }^{\prime \prime}$ | 31'-3" |
|  |  | 30 | 15 | 22'-6" | 21'-3" | 19'-9" | 26'-11" | 25'-5" | 23'-7" | 30'-7" | 28'-10" | 26'-10" | 33'-11" | $32^{\prime}-0$ " | 29'-8" |
|  |  | 40 | 10 | 21'-4" | 20'-6" | 19'-4" | 25'-6" | 24'-6" | 23'-2" | 29'-0" | 27'-10" | 26'-4" | 32'-2" | 30'-10" | 29'-2" |
|  |  | 40 | 15 | 21'-0" | 19'-11" | 18'-7" | 25'-1" | 23'-9" | 22'-2" | 28'-3" | 27'-0" | 25'-3" | 29'-1" | 28'-0" | 26'-6" |
|  |  | 50 | 10 | 19'-9" | 18'-11" | 18'-0" | 23'-7" | 22'-8" | 21'-6" | 26'-1" | 25'-5" | 24'-6" | 26'-10" | 26'-2" | 25'-3" |
|  |  | 50 | 15 | 19'-9" | 18'-10" | 17'-7" | 23'-3" | 22'-6" | 21'-0" | 23'-11" | 23'-2" | 22'-1" | 24'-8" | 23'-10" | 22'-9" |
| $\begin{aligned} & 24 " 1 \\ & \text { o.c. } \end{aligned}$ | $\begin{aligned} & \text { Non- } \\ & \text { Snow } \\ & 125 \% \end{aligned}$ | 20 | 10 | $24^{\prime}-0{ }^{\prime \prime}$ | 22'-8" | 21'-0" | 28'-8" | 27'-0" | 25'-1" | 32 -7" | 30'-9" | 28'-6" | 36'-1" | 34'-1" | 31'-7" |
|  |  | 20 | 15 | 22'-8" | 21'-4" | 19'-8" | 27'-1" | 25'-6" | 23'-6" | 30'-10" | 29'-0" | 26'-9" | 34'-2" | 32'-1" | 29'-8" |
|  |  | 20 | 20 | 21'-8" | 20'-3" | 18'-8" | 25'-10" | 24'-3" | 22'-3" | 29'-5" | 27'-6" | 25'-4" | 32'-7" | 30'-6" | 28'-1" |
|  | $\begin{array}{\|l\|} \hline \text { Snow } \\ 115 \% \end{array}$ | 25 | 10 | 22'-9" | 21'-6" | 20'-1" | 27'-2" | 25'-9" | 23'-11" | 30'-11" | 29'-3" | 27'-3" | $34^{\prime}-3 "$ | $32 \cdot$-5" | 30'-2" |
|  |  | 25 | 15 | 21'-8" | 20'-5" | 18'-11" | 25'-11" | 24'-5" | 22'-7" | 29'-5" | 27'-9" | 25'-9" | 31'-10" | 30'-2" | 28'-1" |
|  |  | 30 | 10 | 21'-9" | 20'-7" | 19'-3" | 25'-11" | 24'-7" | 23'-0" | 29'-6" | 28'-0" | 26'-2" | 32'-1" | 30'-11" | 28'-11" |
|  |  | 30 | 15 | 20'-10" | 19'-8" | 18'-3" | 24'-10" | 23'-6" | 21-10" | 27'-7" | 26'-3" | 24'-7" | 28'-4" | 27'0" | 25'-4" |
|  |  | 40 | 10 | 19'-9" | 18'-11" | 17'-11" | 23'-7" | 22'-8" | 21'-5" | 24'-11" | 24'-3" | 23'-3" | 25'-8" | 24'-11" | 23'-11" |
|  |  | 40 | 15 | 19'-5" | 18'-5" | 17'-2" | 21'-11" | 21'-1" | 19'-11" | 22'-7" | 21'-8" | 20'-7" | 23'-3" | 22'-4" | 21'-2" |
|  |  | 50 | 10 | 18'-3" | 17'-6" | 16'-8" | 20'-2" | 19'-8" | 19'-0" | 20'-9" | 20'-3" | 19'-7" | 21'-5" | 20'-11" | 20'-2" |
|  |  | 50 | 15 | 17'-11" | $17^{\prime}-4{ }^{\prime \prime}$ | $16^{\prime}-3{ }^{\prime \prime}$ | 18'-7" | 17'-11" | 17'-1" | 19'-1" | $18^{\prime}-6{ }^{\prime \prime}$ | 17'-8" | 19'-8" | 19'-0" | $18^{\prime}-2^{\prime \prime}$ |



Maximum clear span in feet and inches, based on horizontal spans.

| 115\% and 125\% Load Duration |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | AJS ${ }^{\circledR} 25$ Series — Deeper Depths - 18" - 24" Depths $3 / 8$ " Web Thickness - $3122^{\prime \prime}$ Flange Width |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | $\stackrel{18 "}{\text { AJS® }} 25$ |  |  | $\begin{gathered} 20 " \\ \text { AJS }^{\circledR} 25 \end{gathered}$ |  |  | $\stackrel{22^{\prime \prime}}{\text { AJS }{ }^{\circledR} 25}$ |  |  | $\stackrel{24 "}{\text { AJS® }} 25$ |  |  |
|  |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \\ & \text { [psff } \end{aligned}$ | $\begin{aligned} & \hline \text { Dead } \\ & \text { Load } \\ & \text { [psf] } \\ & \hline \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \\ \hline \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ |
| $\begin{aligned} & 12 " 1 \\ & \text { o.c. } \end{aligned}$ | Non- <br> Snow <br> 125\% | 20 | 10 | 48'-0" | 47'-5" | 44'-0" | $48{ }^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0$ " | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0$ " | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ |
|  |  | 20 | 15 | 47'-8" | 44'-9" | $41^{\prime}-3$ " | $48{ }^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 44'-9" | $48^{\prime}-0$ | 48'-0" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 48'-0" |
|  |  | 20 | 20 | 45'-5" | 42'-7" | 39'-1" | 48'-0" | $46^{\prime}-1{ }^{\prime \prime}$ | 42'-5" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | $45^{\prime}-7{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 47'-9" | 45'-2" | 42'-0" | 48'-0" | 48'-0" | 45'-6" | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ |
|  |  | 25 | 15 | 45'-7" | 42'-11" | 39'-8" | 48'-0" | 46'-6" | $43^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 46'-3" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ |
|  |  | 30 | 10 | 45'-8" | 43'-3" | 40'-4" | 48'-0" | 46'-10" | $43^{\prime}-8{ }^{\prime \prime}$ | $48^{\prime}-0^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | $47^{\prime}-0^{\prime \prime}$ | $48^{\prime}-0^{\prime \prime}$ | 48'-0" | $48^{\prime}-0^{\prime \prime}$ |
|  |  | 30 | 15 | 43'-10" | 41'-4" | 38'-4" | 47'-6" | 44'-9" | 41'-7" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 44'-8" | 48'-0" | 48'-0" | 47'-8" |
|  |  | 40 | 10 | $41^{\prime}-7{ }^{\prime \prime}$ | 39'-10" | 37'-7" | 45'-1" | 43'-2' | 40'-9" | 48'-0" | 46'-5" | 43'-10" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 46'-10" |
|  |  | 40 | 15 | 40'-11" | 38'-9" | 36'-1" | 44'-4" | 42'-0" | 39'-1" | 46'-10" | 45'-2" | 42'-0" | 48'-0" | 47'-11" | 44'-11" |
|  |  | 50 | 10 | 38'-6" | 36'-11" | 35'-1" | 41'-9" | 40'-0" | 38'-0" | 44'-11" | $43^{\prime}-0{ }^{\prime \prime}$ | 40'-10" | 46'-11" | 46'-0" | 43'-7" |
|  |  | 50 | 15 | 38'-6" | 36 '-8" | $34^{\prime}-3 "$ | 41'-2" | 39'-9" | 37'-2" | $43^{\prime}-1{ }^{\prime \prime}$ | $42^{\prime}-5^{\prime \prime}$ | 39'-11" | 44'-11" | 44'-3" | 42'-8" |
| $\begin{aligned} & \text { 16" } \\ & \text { o.c. } \end{aligned}$ | NonSnow 125\% | 20 | 10 | 45'-7" | 43'-0" | 39'-11" | 48'-0" | 46'-7" | $43^{\prime}-3 "$ | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0^{\prime \prime}$ | $46^{\prime}-6{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ |
|  |  | 20 | 15 | 43'-2" | 40'-7" | 37'-5" | 46'-10" | 43'-11" | 40'-7" | 48'-0" | 47'-3" | 43'-7" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 46'-7" |
|  |  | 20 | 20 | 41'-2" | 38'-7" | 35'-6" | 44'-8" | 41'-10" | 38'-5" | $48^{\prime}-0{ }^{\prime \prime}$ | 44'-11" | 41'-4" | $48^{\prime}-0^{\prime \prime}$ | 48'-0" | 44'-2" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 43'-4" | 40'-11" | 38'-1" | 46'-11" | 44'-4" | 41'-3" | 48'-0" | 47'-8" | 44'-5" | 48'-0" | 48'-0" | 47'-5" |
|  |  | 25 | 15 | $41^{\prime}-4{ }^{\prime \prime}$ | 38'-11" | 36'-0" | 44'-9" | 42'-2" | 39'-0" | $47^{\prime}-5^{\prime \prime}$ | $45 '-4 "$ | 41'-11" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 44'-10" |
|  |  | 30 | 10 | $41^{\prime}-5{ }^{\prime \prime}$ | 39'-2" | 36'-7" | 44'-10" | 42'-6" | 39'-8" | 47'-7" | $45 '$-8" | $42^{\prime}-7{ }^{\prime \prime}$ | 48'-0" | 48'-0" | $45 '$-6" |
|  |  | 30 | 15 | 39'-8" | 37'-5" | 34'-9" | 42'-9" | 40'-7" | $37^{\prime}-8{ }^{\prime \prime}$ | 44'-9" | $43^{\prime}-8{ }^{\prime \prime}$ | 40'-6" | 46'-8" | 45'-7" | 43'-3" |
|  |  | 40 | 10 | 37'-8" | 36'-2" | 34'-1" | 40'-8" | 39'-2' | 37'-0" | 42'-7" | 42'-0" | 39'-9" | 44'-5" | 43'-9" | 42'-5" |
|  |  | 40 | 15 | 36'-7" | 35'-1" | 32'-9" | 38'-8" | 37'-11" | 35'-5" | 40'-6" | 39'-9" | 38'-1" | 42'-3" | 41'-5" | 40'-4" |
|  |  | 50 | 10 | 34'-11" | 33'-6" | 31'-9" | 37'-2" | 36'-3" | $34^{\prime}-5^{\prime \prime}$ | 38'-11" | 38'-5" | 37'-0" | 40'-7" | 40'-1" | 39'-5" |
|  |  | 50 | 15 | 33'-8" | $33^{\prime}-1{ }^{\prime \prime}$ | $31^{\prime \prime} \mathbf{1 ' ~}^{\prime \prime}$ | 35'-7" | 35'-0" | $33^{\prime}-8{ }^{\prime \prime}$ | 37'-4" | 36'-8" | 35'-10" | 38'-11" | 38'-3" | $37^{\prime \prime}-5^{\prime \prime}$ |
| $\begin{gathered} 19.2 " \\ \text { o.c. } \end{gathered}$ | $\begin{array}{\|l\|l} \text { Non- } \\ \text { Snow } \\ 125 \% \\ \hline \end{array}$ | 20 | 10 | 42'-10" | 40'-5" | 37'-6" | 46'-5" | 43'-10" | 40'-8" | 48'-0" | 47'-1" | 43'-8" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 46'-8" |
|  |  | 20 | 15 | 40'-7" | 38'-1" | 35'-2" | 44'-0" | 41'-4" | 38'-2" | 47'-3" | 44'-5" | $41^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 47'-5" | 43'-9" |
|  |  | 20 | 20 | 38'-9" | 36'-3" | $33^{\prime}-4{ }^{\prime \prime}$ | 41'-11" | 39'-3" | 36'-2" | $45^{\prime}-0{ }^{\prime \prime}$ | $42^{\prime}-3 "$ | 38'-10" | 46'-11" | 45'-1" | 41'-6" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 40'-8" | 38'-6" | 35'-10" | 44'-1" | 41'-8" | 38'-10" | 46'-5" | 44'-10" | 41'-9" | 48'-0" | 47'-5" | 44'-7" |
|  |  | 25 | 15 | 38'-10" | 36'-6" | 33'-10" | 41'-4" | 39'-7" | 36'-8" | $43^{\prime}-3^{\prime \prime}$ | 42'-2" | 39'-5" | 45'-2" | 43'-11" | 42'-1" |
|  |  | 30 | 10 | 38'-11" | 36'-10" | 34'-4" | $41^{\prime}-5^{\prime \prime}$ | 39'-11" | $37^{\prime}-3^{\prime \prime}$ | $43^{\prime}-5^{\prime \prime}$ | 42'-8" | 40'-1" | $45^{\prime}-3 "$ | 44'-6" | 42'-9" |
|  |  | 30 | 15 | 36'-10" | 35'-2" | 32'-8" | 39'-0" | 38'-1" | $35^{\prime}-5^{\prime \prime}$ | 40'-10" | 39'-11" | 38'-1" | 42'-7" | 41'-7" | 40'-3" |
|  |  | 40 | 10 | 35'-1" | 33'-11" | 32'-1" | 37'-1" | 36'-7" | 34'-9" | 38'-10" | 38'-4" | 37'-4" | 40'-6" | 39'-11" | 39'-2" |
|  |  | 40 | 15 | 33'-4" | 32'-9" | 30'-9" | 35'-4" | 34'-7" | 33'-4" | 37'-0" | $36^{\prime}-3$ ' | 35'-3" | 38'-7" | 37'-10" | 36'-10" |
|  |  | 50 | 10 | 32'-0" | $31^{\prime}-5^{\prime \prime}$ | 29'-10" | 33'-11" | 33'-6" | 32'-4" | 35'-6" | 35'-1" | $34^{\prime}-6^{\prime \prime}$ | 37'-0" | 36'-7" | 35'-11" |
|  |  | 50 | 15 | 30'-8" | $30^{\prime}-2{ }^{\prime \prime}$ | 29'-2" | 32'-6" | 31'-11" | 31'-3" | $34^{\prime}-0^{\prime \prime}$ | 33'-6" | 32 '-8" | 35'-6" | 34'-11" | $34^{\prime}-1{ }^{\prime \prime}$ |
| $\begin{aligned} & 24 " 1 \\ & \text { o.c. } \end{aligned}$ | NonSnow 125\% | 20 | 10 | 39'-8" | 37'-5" | 34'-9" | 43'-0" | 40'-7" | 37'-8" | 46'-3" | $43^{\prime}$-8" | 40'-6" | $48^{\prime}-0{ }^{\prime \prime}$ | 46'-7" | $43^{\prime}-3 "$ |
|  |  | 20 | 15 | 37'-7" | 35'-4" | 32'-7" | 40'-9" | 38'-3" | 35'-4" | 43'-1" | 41'-2" | 38'-0" | 44'-11" | $43^{\prime}-7{ }^{\prime \prime}$ | 40'-7" |
|  |  | 20 | 20 | 35'-10" | 33'-7" | 30'-11" | 38'-5" | 36'-5" | 33'-6" | 40'-2" | 38'-10" | 36'-0" | 41'-11" | 40'-6" | 38'-5" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 37'-5" | 35'-8" | 33'-2' | 39'-7" | 38'-8" | $36^{\prime}-0{ }^{\prime \prime}$ | 41'-6" | 40'-8" | 38'-8" | $43^{\prime}-3 "$ | 42'-4" | 41'-2" |
|  |  | 25 | 15 | 34'-11" | 33'-10" | 31'-4" | 36'-11" | 35'-11" | $34^{\prime}-0^{\prime \prime}$ | 38'-8" | 37'-8" | 36'-4" | 40'-4" | 39'-3" | 37'-11" |
|  |  | 30 | 10 | 35'-0" | 34'-1" | 31'-10" | 37'-1" | 36'-5" | $34^{\prime}-6{ }^{\prime \prime}$ | 38'-10" | 38'-1" | 37'-1" | 40'-6" | 39'-9" | 38'-9" |
|  |  | 30 | 15 | 32'-11" | 32'-2" | 30'-3" | 34'-10" | 34'-0" | 32'-10" | 36'-6" | 35'-8" | 34'-6" | 38'-1" | 37'-2" | $36^{\prime}-0{ }^{\prime \prime}$ |
|  |  | 40 | 10 | $31^{\prime}-4{ }^{\prime \prime}$ | 30'-11" | 29'-8" | 33'-2" | 32'-8" | 32'-0" | 34'-9" | $34^{\prime}-3 "$ | $33^{\prime}-7{ }^{\prime \prime}$ | 36'-3" | 35'-8" | 35'-0" |
|  |  | 40 | 15 | 29'-10" | 29'-3" | 28'-5" | 31'-6" | 30'-11" | 30'-1" | 33'-0" | 32'-5" | 31'-6" | $34^{\prime}-6{ }^{\prime \prime}$ | 33'-9" | 32'-11" |
|  |  | 50 | 10 | 28'-7" | 28'-3" | 27'-8" | 30'-3" | 29'-11" | 29'-5" | 31'-9" | 31'-4" | 30'-10" | 33'-1" | 32'-8" | 32'-2" |
|  |  | 50 | 15 | 27'-5" | 27'-0" | 26'-4" | 29'-0" | 28'-7" | 27'-11" | 30'-5" | 29'-11" | 29'-3" | 31'-9" | $31^{\prime}-2$ " | 30'-6" |

- Table values are limited by shear, moment, total load deflection equal to L/180 and live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Table values represent the most restrictive of simple or multiple span applications. Analyze multiple span joists with the BC Calc software if the length of any span is less than half the length of an adjacent span.
- Table values assume: minimum bearing lengths, no web stiffeners for joist depths of 16 " and less, web stiffeners required at all bearing locations for 18 " and deeper joists.

This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ software.

- Slope roof joists at least $1 / 4$ " over 12 " to minimize ponding.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

Maximum clear span in feet and inches, based on horizontal spans.

| 115\% and 125\% Load Duration |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | AJS ${ }^{\circledR} 30$ Series - 18" - 24" Depths $3 / 8$ " Web Thickness - $31 / 2^{\prime \prime}$ Flange Width |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | $\begin{gathered} 18 " \\ \text { AJS } \end{gathered}$ |  |  | $\begin{gathered} 20 " \\ \text { AJS }{ }^{\circledR} 30 \end{gathered}$ |  |  | $\stackrel{22^{\prime \prime}}{\text { AJS }} 30$ |  |  | $\stackrel{24 "}{\text { AJS }} 2$ |  |  |
|  |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \\ & \text { [psf] } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \text { Dead } \\ \text { Load } \\ \text { [psf] } \\ \hline \end{array}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{aligned} & 4 / 12 \\ & \text { or } \\ & \text { Less } \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{aligned} & 8 / 12 \\ & \text { to } \\ & 12 / 12 \end{aligned}$ | $\begin{gathered} 4 / 12 \\ \text { or } \\ \text { Less } \end{gathered}$ | $\begin{gathered} 4 / 12 \\ \text { to } \\ 8 / 12 \end{gathered}$ | $\begin{gathered} 8 / 12 \\ \text { to } \\ 12 / 12 \end{gathered}$ |
| $\begin{array}{\|l\|l\|} \hline 12 " 1 \\ \text { o.c. } \end{array}$ | Non- <br> Snow <br> 125\% | 20 | 10 | 48'-0" | 48'-0" | 45'-5" | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0$ " | 48'-0" | 48'-0" | $48{ }^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 48'-0" |
|  |  | 20 | 15 | 48'-0" | $46^{\prime}-3{ }^{\prime \prime}$ | 42'-8" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 46'-2" | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ |
|  |  | 20 | 20 | 47'-0" | $44{ }^{\prime}-0{ }^{\prime \prime}$ | 40'-5" | 48'-0" | 47'-7" | 43'-9" | 48'-0" | 48'-0" | $47^{\prime}-1{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" |
|  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | 25 | 10 | 48'-0" | 46'-8" | 43'-5" | $48^{\prime}-0^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 47'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ |
|  |  | 25 | 15 | 47'-1" | 44'-4" | 41'-0" | 48'-0" | 48'-0" | 44'-5" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 47'-9" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" |
|  |  | 30 | 10 | 47'-2" | 44'-8" | 41'-8" | 48'-0" | 48'-0" | 45'-2" | 48'-0" | $48{ }^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 48'-0" | 48'-0" | 48'-0" |
|  |  | 30 | 15 | $45 \cdot-3 "$ | 42'-8" | 39'-7" | 48'-0" | 46'-3" | 42'-11" | 48'-0" | 48'-0" | 46'-1" | 48'-0" | 48'-0" | 48'-0" |
|  |  | 40 | 10 | 43'-0" | $41^{\prime}-2{ }^{\prime \prime}$ | 38'-11" | 46'-6" | 44'-7" | 42'-1" | 48'-0" | 47'-11" | 45'-3" | 48'-0" | 48'-0" | 48'-0" |
|  |  | 40 | 15 | 42'-4" | 40'-0" | 37'-3" | 45'-10" | 43'-4" | 40'-5" | 48'-0" | 46'-7" | 43'-5" | 48'-0" | 48'-0" | 46'-4" |
|  |  | 50 | 10 | 39'-10" | 38'-2' | 36'-3" | 43'-1" | 41'-4" | 39'-3" | 46'-4" | 44'-5" | 42'-2" | 48'-0" | 47'-6" | 45'-0" |
|  |  | 50 | 15 | 39'-10" | 37'-11" | 35'-5" | 43'-1" | 41'-1" | 38'-4" | $46^{\prime}-4$ " | 44'-2" | $41^{\prime}-3{ }^{\prime \prime}$ | 48'-0" | 47'-2" | $44^{\prime}-0^{\prime \prime}$ |
| $\begin{array}{\|l\|l\|} \hline 16 " 1 \\ \text { o.c. } \end{array}$ | Non- <br> Snow <br> 125\% | 20 | 10 | 47'-1" | 44'-5" | 41'-3" | 48'-0" | 48'-0" | 44'-8" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | $48^{\prime}-0$ " | 48'-0" |
|  |  | 20 | 15 | 44'-7" | 41'-11" | 38'-8" | 48'-0" | 45'-5" | 41'-11" | 48'-0" | 48'-0" | 45'-0" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" |
|  |  | 20 | 20 | 42'-7" | 39'-10" | 36'-8" | 46'-1" | 43'-2" | 39'-8" | 48'-0" | $46^{\prime}-5{ }^{\prime \prime}$ | 42'-8" | 48'-0" | 48'-0" | 45 '-7" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 44'-9" | 42'-4" | 39'-4" | 48'-0" | 45'-10" | 42'-8" | 48'-0" | 48'-0" | 45'-10" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" |
|  |  | 25 | 15 | 42'-8" | 40'-2" | 37'-2" | 46'-3" | 43'-6" | 40'-4" | 48'-0" | 46'-9" | 43'-4" | 48'-0" | 48'-0" | $46^{\prime}-3^{\prime \prime}$ |
|  |  | 30 | 10 | 42'-9" | 40'-6" | 37'-9" | $46^{\prime}-4{ }^{\prime \prime}$ | 43'-11" | 40'-11" | 48'-0" | 47'-2" | 44'-0" | 48'-0" | 48'-0" | 47'-0" |
|  |  | 30 | 15 | 41'-0" | 38'-8" | 35'-11" | 44'-5" | 41'-11" | 38'-11" | 47'-9" | 45'-1" | 41'-10" | 48'-0" | 48'-0" | 44'-8" |
|  |  | 40 | 10 | 38'-11" | 37'-4" | 35'-3" | 42'-2" | 40'-5" | 38'-2" | 45'-4" | 43'-6" | $41^{\prime}-0{ }^{\prime \prime}$ | 48'-0" | 46'-5" | 43'-10" |
|  |  | 40 | 15 | 38'-4" | 36 '-3" | 33'-10" | 41'-6" | 39'-4" | 36 '-7" | 44'-8" | 42'-3" | 39'-4" | 47'-7" | 45'-1" | 42'-0" |
|  |  | 50 | 10 | 36'-1" | $34^{\prime}-7{ }^{\prime \prime}$ | 32'-10" | 39'-1" | 37'-6" | $35 '-7{ }^{\prime \prime}$ | 42'-0" | 40'-3" | 38'-3" | 44'-10" | 43'-0" | 40'-10" |
|  |  | 50 | 15 | 36'-1" | $34^{\prime}-4{ }^{\prime \prime}$ | 32'-1" | 39'-1" | 37'-2" | 34'-9" | 42'-0" | 40'-0" | 37'-5" | 43'-10' | 42'-9" | 39'-11" |
| $\begin{array}{\|c\|c\|} \hline 19.2^{\prime \prime} \\ \text { o.c. } \end{array}$ | Non- <br> Snow <br> 125\% | 20 | 10 | 44'-3" | 41'-9" | 38'-9" | 47'-11" | 45'-3" | 42'-0" | 48'-0" | 48'-0" | 45'-1" | 48'-0" | $48^{\prime}-0{ }^{\prime \prime}$ | 48'-0" |
|  |  | 20 | 15 | 41'-11" | 39'-5" | 36'-4" | 45'-5" | 42'-8" | 39'-5" | 48'-0" | 45'-10" | 42'-4" | 48'-0" | 48'-0" | 45'-2" |
|  |  | 20 | 20 | 40'-0" | 37'-5" | $34^{\prime}-5^{\prime \prime}$ | 43'-4" | 40'-7" | 37'-4" | 46'-7" | 43'-7" | 40'-1" | 48'-0" | 46'-7" | 42'-10" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 42'-0" | 39'-9" | $37{ }^{-1} 0^{\prime \prime}$ | 45'-6" | 43'-1" | 40'-1" | 48'-0" | 46'-3" | 43'-1" | 48'-0" | 48'-0" | 46'-0" |
|  |  | 25 | 15 | 40'-1" | 37'-9" | 35'-0" | 43'-5" | 40'-11" | 37'-10' | $46^{\prime}-8{ }^{\prime \prime}$ | 43'-11" | 40'-8" | 48'-0" | 46'-11" | 43'-6" |
|  |  | 30 | 10 | 40'-2" | 38'-1" | 35'-6" | 43'-6" | 41'-3" | 38'-6" | 46'-9" | 44'-4" | 41'-4" | 48'-0" | 47'-4" | 44'-2" |
|  |  | 30 | 15 | 38'-6" | 36'-4" | 33'-9" | 41'-9" | 39'-5" | 36'-7" | 44'-10" | 42'-4" | 39'-4" | 47'-11" | 45'-3" | 42'-0" |
|  |  | 40 | 10 | 36'-7" | $35{ }^{\prime}-1$ " | 33'-1" | 39'-7" | 38'-0" | 35'-11" | 42'-7" | 40'-10" | 38'-7" | 45'-6" | 43'-7" | 41'-2" |
|  |  | 40 | 15 | 36'-0" | 34'-1" | 31'-9" | 39'-0" | 36'-11" | 34'-5" | 41'-8" | 39'-8" | 37'-0" | 43'-5" | 42'-5" | 39'-6" |
|  |  | 50 | 10 | 33'-10" | 32'-6" | 30'-10" | 36'-8" | 35'-2" | $33^{\prime}-5^{\prime \prime}$ | 39'-5" | 37'-10" | 35'-11" | 41'-8" | 40'-5" | 38'-4" |
|  |  | 50 | 15 | 33'-10" | 32'-3" | $30^{\prime}-2{ }^{\prime \prime}$ | 36'-7" | 34'-11" | 32'-8" | 38'-4" | 37'-7" | $35{ }^{\prime}-1{ }^{\prime \prime}$ | 40'-0" | 39'-4" | 37'-6" |
| $\begin{aligned} & 24 " \\ & \text { o.c. } \end{aligned}$ | Non- <br> Snow <br> 125\% | 20 | 10 | 41'-0" | 38'-8" | 35'-11" | $44^{\prime}-5^{\prime \prime}$ | 41'-11" | 38'-11" | 47'-9" | 45'-1" | 41'-10" | 48'-0" | 48'-0" | 44'-8" |
|  |  | 20 | 15 | 38'-10" | 36'-6" | 33'-8" | 42'-1" | 39'-6" | 36'-6" | 45'-3" | 42'-6" | 39'-3" | 48'-0" | 45'-4" | 41'-11" |
|  |  | 20 | 20 | 37'-0" | $34^{\prime}-8{ }^{\prime \prime}$ | 31'-11" | 40'-2" | 37'-7" | 34'-7" | 43'-2" | 40'-5" | 37'-2" | 46'-1" | 43'-2" | 39'-8" |
|  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | 25 | 10 | 38'-11" | 36'-10" | 34'-3" | 42'-2" | 39'-11" | 37'-2" | 45'-4" | 42'-11" | 39'-11" | 48'-0" | 45'-9" | 42'-8" |
|  |  | 25 | 15 | 37'-1" | 35'-0" | 32'-5" | 40'-3" | 37'-11" | 35'-1" | 43'-3" | 40'-9" | 37'-9" | 45'-5" | 43'-6" | 40'-3" |
|  |  | 30 | 10 | 37 '-3" | 35'-3" | 32'-11" | 40'-4" | 38'-2" | 35'-8" | 43'-4" | $41^{\prime \prime} 1^{\prime \prime}$ | 38'-4" | $45^{\prime}-7{ }^{\prime \prime}$ | 43'-10" | 40'-11" |
|  |  | 30 | 15 | 35'-8" | $33^{\prime}-8{ }^{\prime \prime}$ | 31'-3" | 38'-8" | 36'-6" | 33'-11" | 41'-1" | 39'-3" | $36^{\prime}-5^{\prime \prime}$ | 42'-10" | 41'-10" | 38'-11" |
|  |  | 40 | 10 | 33'-10' | 32'-6" | $30^{\prime}-8{ }^{\prime \prime}$ | $36^{\prime}-8{ }^{\prime \prime}$ | 35'-2" | $33^{\prime}-3{ }^{\prime \prime}$ | 39'-1" | 37'-10" | 35'-9" | 40'-10" | 40'-3" | 38'-2' |
|  |  | 40 | 15 | $33^{\prime}-4{ }^{\prime \prime}$ | $31^{\prime}-7{ }^{\prime \prime}$ | 29'-5" | 35'-6" | 34'-2" | 31'-10" | 37'-3" | 36'-6" | $34{ }^{\prime}-3 "$ | 38'-10" | 38'-1" | 36'-7" |
|  |  | 50 | 10 | 31'-4" | 30'-1" | 28'-7" | 33'-11" | 32'-7" | 30'-11" | 35'-9" | 35'-0" | $33^{\prime}-3^{\prime \prime}$ | 37'-3" | 36'-10" | 35'-6" |
|  |  | 50 | 15 | 30'-11" | 29'-10" | 27'-11" | 32'-8" | 32'-2" | $30^{\prime}-3 "$ | $34^{\prime}-3 "$ | $33^{\prime}-8{ }^{\prime \prime}$ | 32 '-6" | 35'-9" | 35'-2" | 34'-3" |

## Allowable Uniform Roof Load <br> (in pounds per linear foot [PLF])

## 115\% and 125\% Load Duration

| Use of these tables should be limited to roof slopes of $31 / 2^{\prime \prime}$ per foot or less. For steeper slopes, see pages 27-31. |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AJS ${ }^{\circledR} 20$ Series 2½" Flange Width |  |  |  |  |  |  |  |  |  |  |  |
|  | 9½" AJS ${ }^{\text {® }} 20$ |  |  | 117\%" AJS® 20 |  |  | 14" AJS® 20 |  |  | 16" AJS® 20 |  |  |
|  | Total | Load | Deflect. | Tota | Load | Deflect. | Total | oad | Deflect. | Tota | Load | Deflect. |
| Span Length | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) | L/240 | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | $\begin{aligned} & \text { Non- } \\ & \text { Snow } \\ & (125 \%) \end{aligned}$ | L/240 | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) | L/240 | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) | L/240 |
| 6 | 353 | 383 | - | 358 | 389 | - | 360 | 392 | - | 364 | 396 | - |
| 7 | 302 | 329 | - | 307 | 334 | - | 309 | 336 | - | 312 | 339 | - |
| 8 | 264 | 287 | - | 269 | 292 | - | 270 | 294 | - | 273 | 297 | - |
| 9 | 235 | 255 | - | 239 | 259 | - | 240 | 261 | - | 242 | 264 | - |
| 10 | 211 | 230 | - | 215 | 233 | - | 216 | 235 | - | 218 | 237 | - |
| 11 | 192 | 209 | - | 195 | 212 | - | 196 | 213 | - | 198 | 216 | - |
| 12 | 176 | 191 | - | 179 | 194 | - | 180 | 196 | - | 182 | 198 | - |
| 13 | 162 | 177 | - | 165 | 179 | - | 166 | 180 | - | 168 | 182 | - |
| 14 | 151 | 164 | - | 153 | 167 | - | 154 | 168 | - | 156 | 169 | - |
| 15 | 136 | 147 | 128 | 143 | 155 | - | 144 | 156 | - | 145 | 158 | - |
| 16 | 119 | 129 | 106 | 134 | 146 | - | 135 | 147 | - | 136 | 148 | - |
| 17 | 105 | 115 | 89 | 126 | 137 | - | 127 | 138 | - | 128 | 139 | - |
| 18 | 94 | 99 | 76 | 119 | 129 | - | 120 | 130 | - | 121 | 132 | - |
| 19 | 84 | 85 | 65 | 109 | 119 | 108 | 113 | 123 | - | 115 | 125 | - |
| 20 | 73 | 73 | 56 | 99 | 107 | 94 | 108 | 117 | - | 109 | 118 | - |
| 21 | 63 | 63 | 48 | 89 | 97 | 81 | 103 | 112 | - | 104 | 113 | - |
| 22 | 55 | 55 | 42 | 81 | 89 | 71 | 98 | 106 | - | 99 | 108 | - |
| 23 | 48 | 48 | 37 | 74 | 81 | 62 | 90 | 98 | - | 95 | 103 | - |
| 24 | 43 | 43 | 33 | 68 | 72 | 55 | 82 | 90 | 80 | 91 | 99 | - |
| 25 |  |  |  | 63 | 64 | 49 | 76 | 83 | 71 | 87 | 95 | - |
| 26 |  |  |  | 57 | 57 | 44 | 70 | 76 | 63 | 81 | 89 | - |
| 27 |  |  |  | 51 | 51 | 39 | 65 | 71 | 57 | 75 | 82 | - |
| 28 |  |  |  | 46 | 46 | 35 | 60 | 66 | 51 | 70 | 76 | 69 |
| 29 |  |  |  | 41 | 41 | 32 | 56 | 60 | 46 | 65 | 71 | 62 |
| 30 |  |  |  |  |  |  | 53 | 55 | 42 | 61 | 66 | 56 |
| 31 |  |  |  |  |  |  | 49 | 50 | 38 | 57 | 62 | 51 |
| 32 |  |  |  |  |  |  | 45 | 45 | 34 | 54 | 58 | 47 |
| 33 |  |  |  |  |  |  | 41 | 41 | 31 | 50 | 55 | 43 |
| 34 |  |  |  |  |  |  |  |  |  | 47 | 51 | 39 |
| 35 |  |  |  |  |  |  |  |  |  | 45 | 47 | 36 |
| 36 |  |  |  |  |  |  |  |  |  | 42 | 43 | 33 |
| 37 |  |  |  |  |  |  |  |  |  | 40 | 40 | 30 |
| 38 |  |  |  |  |  |  |  |  |  |  |  |  |

- Total Load values are limited by shear, moment, or deflection equal to L/180.
- Deflection values (Deflect.) are limited by live load deflection equal to $\mathrm{L} / 240$. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC CALC® software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least $1 / 4$ inch over 12 inches to minimize ponding.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.


## Allowable Uniform Roof Load <br> (in pounds per linear foot [PLF])

## 115\% and 125\% Load Duration

Use of these tables should be limited to roof slopes of $31 / 2^{\prime \prime}$ per foot or less.
For steeper slopes, see pages 27-31.

| Span Length | AJS® 190 Series 2½" Flange Width |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 91⁄2" AJS® 190 |  |  | 117/8" AJS® 190 |  |  | 14" AJS® 190 |  |  | 16" AJS® 190 |  |  |
|  | Total Load |  | Deflect. <br> L/240 | Total Load |  | Deflect.L/240 | Total Load |  | Deflect.L/240 | Total Load |  | Deflect. <br> L/240 |
|  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) |  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) |  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) |  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) |  |
| 6 | 353 | 383 | - | 358 | 389 | - | 360 | 392 | - | 364 | 396 | - |
| 7 | 302 | 329 | - | 307 | 334 | - | 309 | 336 | - | 312 | 339 | - |
| 8 | 264 | 287 | - | 269 | 292 | - | 270 | 294 | - | 273 | 297 | - |
| 9 | 235 | 255 | - | 239 | 259 | - | 240 | 261 | - | 242 | 264 | - |
| 10 | 211 | 230 | - | 215 | 233 | - | 216 | 235 | - | 218 | 237 | - |
| 11 | 192 | 209 | - | 195 | 212 | - | 196 | 213 | - | 198 | 216 | - |
| 12 | 176 | 191 | - | 179 | 194 | - | 180 | 196 | - | 182 | 198 | - |
| 13 | 162 | 177 | - | 165 | 179 | - | 166 | 180 | - | 168 | 182 | - |
| 14 | 151 | 164 | - | 153 | 167 | - | 154 | 168 | - | 156 | 169 | - |
| 15 | 141 | 153 | 134 | 143 | 155 | - | 144 | 156 | - | 145 | 158 | - |
| 16 | 132 | 143 | 111 | 134 | 146 | - | 135 | 147 | - | 136 | 148 | - |
| 17 | 121 | 123 | 94 | 126 | 137 | - | 127 | 138 | - | 128 | 139 | - |
| 18 | 104 | 104 | 79 | 119 | 129 | - | 120 | 130 | - | 121 | 132 | - |
| 19 | 89 | 89 | 68 | 113 | 123 | - | 113 | 123 | - | 115 | 125 | - |
| 20 | 77 | 77 | 59 | 107 | 116 | 98 | 108 | 117 | - | 109 | 118 | - |
| 21 | 67 | 67 | 51 | 102 | 111 | 85 | 103 | 112 | - | 104 | 113 | - |
| 22 | 58 | 58 | 44 | 93 | 97 | 74 | 98 | 106 | - | 99 | 108 | - |
| 23 | 51 | 51 | 39 | 85 | 86 | 65 | 94 | 102 | - | 95 | 103 | - |
| 24 | 45 | 45 | 34 | 76 | 76 | 58 | 90 | 98 | 84 | 91 | 99 | - |
| 25 | 40 | 40 | 30 | 67 | 67 | 51 | 86 | 94 | 75 | 87 | 95 | - |
| 26 |  |  |  | 60 | 60 | 46 | 80 | 87 | 67 | 84 | 91 | - |
| 27 |  |  |  | 54 | 54 | 41 | 75 | 78 | 60 | 80 | 88 | - |
| 28 |  |  |  | 48 | 48 | 37 | 69 | 70 | 54 | 78 | 84 | 72 |
| 29 |  |  |  | 43 | 43 | 33 | 63 | 63 | 48 | 75 | 81 | 65 |
| 30 |  |  |  |  |  |  | 57 | 57 | 44 | 70 | 76 | 59 |
| 31 |  |  |  |  |  |  | 52 | 52 | 40 | 66 | 70 | 54 |
| 32 |  |  |  |  |  |  | 47 | 47 | 36 | 61 | 64 | 49 |
| 33 |  |  |  |  |  |  | 43 | 43 | 33 | 58 | 59 | 45 |
| 34 |  |  |  |  |  |  | 40 | 40 | 30 | 54 | 54 | 41 |
| 35 |  |  |  |  |  |  |  |  |  | 49 | 49 | 38 |
| 36 |  |  |  |  |  |  |  |  |  | 45 | 45 | 35 |
| 37 |  |  |  |  |  |  |  |  |  | 42 | 42 | 32 |
| 38 |  |  |  |  |  |  |  |  |  |  |  |  |

- Total Load values are limited by shear, moment, or deflection equal to L/180.
- Deflection values (Deflect.) are limited by live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC CALC® software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least $1 / 4$ inch over 12 inches to minimize ponding.
- Table values assume minimum bearing lengths without web stiffeners for joist depths of 16 inches and less.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® ${ }^{\circledR}$ software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.


## Allowable Uniform Roof Load <br> (in pounds per linear foot [PLF])

## 115\% and 125\% Load Duration

Use of these tables should be limited to roof slopes of $31 / 2^{\prime \prime}$ per foot or less.
For steeper slopes, see pages 27-31.

| Span Length | AJS ${ }^{\circledR} 25$ Series - $91 / 2^{\prime \prime}-16$ " Depths $3 / 8^{\prime \prime}$ Web Thickness - $31 / 2^{\prime \prime}$ Flange Width |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 91⁄2" AJS® 25 |  |  | 111/8" AJS® 25 |  |  | 14" AJS® 25 |  |  | 16" AJS® 25 |  |  |
|  | Total Load |  | Deflect. <br> L/240 | Total Load |  | Deflect.L/240 | Total Load |  | Deflect. <br> L/240 | Total Load |  | Deflect.L/240 |
|  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | $\begin{gathered} \text { Non- } \\ \text { Snow } \\ (125 \%) \\ \hline \end{gathered}$ |  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | $\begin{gathered} \text { Non- } \\ \text { Snow } \\ (125 \%) \end{gathered}$ |  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | $\begin{gathered} \text { Non- } \\ \text { Snow } \\ (125 \%) \end{gathered}$ |  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | $\begin{gathered} \text { Non- } \\ \text { Snow } \\ (125 \%) \\ \hline \end{gathered}$ |  |
| 6 | 356 | 387 | - | 358 | 389 | - | 360 | 392 | - | 364 | 396 | - |
| 7 | 305 | 332 | - | 307 | 334 | - | 309 | 336 | - | 312 | 339 | - |
| 8 | 267 | 290 | - | 269 | 292 | - | 270 | 294 | - | 273 | 297 | - |
| 9 | 237 | 258 | - | 239 | 259 | - | 240 | 261 | - | 242 | 264 | - |
| 10 | 214 | 232 | - | 215 | 233 | - | 216 | 235 | - | 218 | 237 | - |
| 11 | 194 | 211 | - | 195 | 212 | - | 196 | 213 | - | 198 | 216 | - |
| 12 | 178 | 193 | - | 179 | 194 | - | 180 | 196 | - | 182 | 198 | - |
| 13 | 164 | 179 | - | 165 | 179 | - | 166 | 180 | - | 168 | 182 | - |
| 14 | 152 | 166 | - | 153 | 167 | - | 154 | 168 | - | 156 | 169 | - |
| 15 | 142 | 155 | - | 143 | 155 | - | 144 | 156 | - | 145 | 158 | - |
| 16 | 133 | 145 | - | 134 | 146 | - | 135 | 147 | - | 136 | 148 | - |
| 17 | 125 | 136 | 121 | 126 | 137 | - | 127 | 138 | - | 128 | 139 | - |
| 18 | 118 | 129 | 103 | 119 | 129 | - | 120 | 130 | - | 121 | 132 | - |
| 19 | 112 | 116 | 88 | 113 | 123 | - | 113 | 123 | - | 115 | 125 | - |
| 20 | 100 | 100 | 76 | 107 | 116 | - | 108 | 117 | - | 109 | 118 | - |
| 21 | 87 | 87 | 66 | 102 | 111 | - | 103 | 112 | - | 104 | 113 | - |
| 22 | 76 | 76 | 58 | 93 | 102 | - | 98 | 106 | - | 99 | 108 | - |
| 23 | 67 | 67 | 51 | 85 | 93 | - | 94 | 102 | - | 95 | 103 | - |
| 24 | 59 | 59 | 45 | 78 | 85 | 75 | 90 | 98 | - | 91 | 99 | - |
| 25 | 52 | 52 | 40 | 72 | 79 | 67 | 86 | 94 | - | 87 | 95 | - |
| 26 | 46 | 46 | 35 | 67 | 73 | 59 | 80 | 87 | - | 84 | 91 | - |
| 27 | 42 | 42 | 32 | 62 | 67 | 53 | 75 | 81 | - | 80 | 88 | - |
| 28 |  |  |  | 58 | 63 | 48 | 69 | 75 | - | 78 | 84 | - |
| 29 |  |  |  | 54 | 57 | 43 | 65 | 70 | 63 | 75 | 81 | - |
| 30 |  |  |  | 50 | 51 | 39 | 60 | 66 | 57 | 70 | 76 | - |
| 31 |  |  |  | 47 | 47 | 35 | 56 | 61 | 52 | 66 | 71 | - |
| 32 |  |  |  | 42 | 42 | 32 | 53 | 58 | 47 | 61 | 67 | - |
| 33 |  |  |  |  |  |  | 50 | 54 | 43 | 58 | 63 | - |
| 34 |  |  |  |  |  |  | 47 | 51 | 39 | 54 | 59 | 53 |
| 35 |  |  |  |  |  |  | 44 | 47 | 36 | 51 | 56 | 49 |
| 36 |  |  |  |  |  |  | 42 | 44 | 33 | 48 | 53 | 45 |
| 37 |  |  |  |  |  |  |  |  |  | 46 | 50 | 41 |
| 38 |  |  |  |  |  |  |  |  |  | 43 | 47 | 38 |

- Total Load values are limited by shear,
moment, or deflection equal to L/180.
- Deflection values (Deflect.) are limited by live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC CALC® software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least $1 / 4$ inch over 12 inches to minimize ponding.
- Table values assume: minimum bearing lengths, no web stiffeners for joist depths of

16 "and less, web stiffeners required at all bearing locations for 18" and deeper joists.

- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.


## Allowable Uniform Roof Load <br> (in pounds per linear foot [PLF])

## 115\% and 125\% Load Duration

Use of these tables should be limited to roof slopes of $31 / 2^{\prime \prime}$ per foot or less.
For steeper slopes, see pages 27-31.

| Span Length | AJS ${ }^{\circledR} 25$ Series - Deeper Depths - 18" - 24" Depths $3 / 8$ " Web Thickness - $31 / 2^{\prime \prime}$ Flange Width |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 18" AJS® 25 |  |  | 20" AJS® 25 |  |  | 22" AJS® 25 |  |  | 24" AJS® 25 |  |  |
|  | Total Load |  | Deflect.$\mathrm{L} / 240$ | Total Load |  | Deflect.L/240 | Total Load |  | Deflect. <br> L/240 | Total Load |  | Deflect. <br> L/240 |
|  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) |  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | $\begin{aligned} & \hline \text { Non- } \\ & \text { Snow } \end{aligned}$ $(125 \%)$ |  | $\begin{aligned} & \text { Snow } \\ & (115 \%) \end{aligned}$ | $\begin{gathered} \text { Non- } \\ \text { Snow } \\ (125 \%) \end{gathered}$ |  | $\begin{aligned} & \text { Snow } \\ & (115 \%) \end{aligned}$ | NonSnow (125\%) |  |
| 6 | 709 | 770 | - | 767 | 834 | - | 785 | 854 | - | 803 | 873 | - |
| 7 | 607 | 660 | - | 658 | 715 | - | 673 | 732 | - | 688 | 748 | - |
| 8 | 531 | 578 | - | 575 | 625 | - | 589 | 640 | - | 602 | 654 | - |
| 9 | 472 | 513 | - | 511 | 556 | - | 523 | 569 | - | 535 | 582 | - |
| 10 | 425 | 462 | - | 460 | 500 | - | 471 | 512 | - | 481 | 523 | - |
| 11 | 386 | 420 | - | 418 | 455 | - | 428 | 465 | - | 438 | 476 | - |
| 12 | 354 | 385 | - | 383 | 417 | - | 392 | 427 | - | 401 | 436 | - |
| 13 | 327 | 355 | - | 354 | 385 | - | 362 | 394 | - | 370 | 402 | - |
| 14 | 303 | 330 | - | 329 | 357 | - | 336 | 366 | - | 344 | 374 | - |
| 15 | 283 | 308 | - | 307 | 333 | - | 314 | 341 | - | 321 | 349 | - |
| 16 | 265 | 289 | - | 287 | 312 | - | 294 | 320 | - | 301 | 327 | - |
| 17 | 250 | 272 | - | 271 | 294 | - | 277 | 301 | - | 283 | 308 | - |
| 18 | 236 | 256 | - | 255 | 278 | - | 261 | 284 | - | 267 | 291 | - |
| 19 | 223 | 243 | - | 242 | 263 | - | 248 | 269 | - | 253 | 275 | - |
| 20 | 212 | 231 | - | 230 | 250 | - | 235 | 256 | - | 240 | 261 | - |
| 21 | 202 | 220 | - | 219 | 238 | - | 224 | 244 | - | 229 | 249 | - |
| 22 | 193 | 210 | - | 209 | 227 | - | 214 | 232 | - | 219 | 238 | - |
| 23 | 185 | 201 | - | 200 | 217 | - | 205 | 222 | - | 209 | 227 | - |
| 24 | 171 | 186 | - | 191 | 208 | - | 196 | 213 | - | 200 | 218 | - |
| 25 | 158 | 172 | - | 177 | 192 | - | 188 | 205 | - | 192 | 209 | - |
| 26 | 146 | 159 | - | 163 | 177 | - | 179 | 195 | - | 185 | 201 | - |
| 27 | 135 | 147 | - | 151 | 164 | - | 166 | 180 | - | 178 | 194 | - |
| 28 | 126 | 137 | 124 | 141 | 153 | - | 154 | 168 | - | 168 | 182 | - |
| 29 | 117 | 127 | 112 | 131 | 142 | - | 144 | 156 | - | 156 | 170 | - |
| 30 | 109 | 119 | 101 | 122 | 133 | - | 134 | 146 | - | 146 | 159 | - |
| 31 | 102 | 111 | 92 | 115 | 125 | - | 126 | 137 | - | 137 | 149 | - |
| 32 | 96 | 105 | 84 | 108 | 117 | 106 | 118 | 128 | - | 128 | 139 | - |
| 33 | 90 | 98 | 77 | 101 | 110 | 97 | 111 | 121 | - | 121 | 131 | - |
| 34 | 85 | 92 | 70 | 95 | 104 | 89 | 104 | 114 | - | 114 | 123 | - |
| 35 | 80 | 85 | 65 | 90 | 98 | 82 | 99 | 107 | - | 107 | 117 | - |
| 36 | 76 | 78 | 59 | 85 | 92 | 75 | 93 | 101 | - | 101 | 110 | - |
| 37 | 72 | 72 | 55 | 80 | 87 | 69 | 88 | 96 | 86 | 96 | 104 | - |
| 38 | 66 | 66 | 51 | 76 | 83 | 64 | 84 | 91 | 79 | 91 | 99 | - |
| 39 | 62 | 62 | 47 | 72 | 78 | 60 | 79 | 86 | 74 | 86 | 94 | - |
| 40 | 57 | 57 | 44 | 69 | 72 | 55 | 75 | 82 | 68 | 82 | 89 | - |

- Total Load values are limited by shear, moment, or deflection equal to L/180.
- Deflection values (Deflect.) are limited by live load deflection equal to L/240. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC CALC® ${ }^{\text {s }}$ software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least $1 / 4$ inch over 12 inches to minimize ponding.
- Table values assume: minimum bearing lengths, no web stiffeners for joist depths of 16" and less, web stiffeners required at all bearing locations for 18 " and deeper joists.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.


## Allowable Uniform Roof Load <br> (in pounds per linear foot [PLF])

## 115\% and 125\% Load Duration

Use of these tables should be limited to roof slopes of $31 / 2$ " per foot or less.
For steeper slopes, see pages 21-31.

| Span Length | AJS® 30 Series - Deeper Depths - 18" - 24" Depths $3 / 8$ " Web Thickness - $31 / 2^{\prime \prime}$ Flange Width |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 18" AJS® 30 |  |  | 20" AJS® 30 |  |  | 22" AJS® 30 |  |  | 24" AJS® 30 |  |  |
|  | Total Load |  | Deflect. <br> L/240 | Total Load |  | Deflect.L/240 | Total Load |  | Deflect. <br> L/240 | Total Load |  | Deflect. <br> L/240 |
|  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | NonSnow (125\%) |  | $\begin{aligned} & \text { Snow } \\ & (115 \%) \end{aligned}$ | $\begin{aligned} & \hline \text { Non- } \\ & \text { Snow } \end{aligned}$ $(125 \%)$ |  | $\begin{gathered} \text { Snow } \\ (115 \%) \end{gathered}$ | $\begin{gathered} \text { Non- } \\ \text { Snow } \\ (125 \%) \end{gathered}$ |  | $\begin{aligned} & \text { Snow } \\ & (115 \%) \end{aligned}$ | NonSnow (125\%) |  |
| 6 | 709 | 770 | - | 767 | 834 | - | 785 | 854 |  | 803 | 873 | - |
| 7 | 607 | 660 | - | 658 | 715 | - | 673 | 732 |  | 688 | 748 | - |
| 8 | 531 | 578 | - | 575 | 625 | - | 589 | 640 | - | 602 | 654 | - |
| 9 | 472 | 513 | - | 511 | 556 | - | 523 | 569 | - | 535 | 582 | - |
| 10 | 425 | 462 | - | 460 | 500 | - | 471 | 512 | - | 481 | 523 | - |
| 11 | 386 | 420 | - | 418 | 455 | - | 428 | 465 | - | 438 | 476 | - |
| 12 | 354 | 385 | - | 383 | 417 | - | 392 | 427 | - | 401 | 436 | - |
| 13 | 327 | 355 | - | 354 | 385 | - | 362 | 394 | - | 370 | 402 | - |
| 14 | 303 | 330 | - | 329 | 357 | - | 336 | 366 | - | 344 | 374 | - |
| 15 | 283 | 308 | - | 307 | 333 | - | 314 | 341 | - | 321 | 349 | - |
| 16 | 265 | 289 | - | 287 | 312 | - | 294 | 320 | - | 301 | 327 | - |
| 17 | 250 | 272 | - | 271 | 294 | - | 277 | 301 | - | 283 | 308 | - |
| 18 | 236 | 256 | - | 255 | 278 | - | 261 | 284 | - | 267 | 291 | - |
| 19 | 223 | 243 | - | 242 | 263 | - | 248 | 269 | - | 253 | 275 | - |
| 20 | 212 | 231 | - | 230 | 250 | - | 235 | 256 | - | 240 | 261 | - |
| 21 | 202 | 220 | - | 219 | 238 | - | 224 | 244 | - | 229 | 249 | - |
| 22 | 193 | 210 | - | 209 | 227 | - | 214 | 232 | - | 219 | 238 | - |
| 23 | 185 | 201 | - | 200 | 217 | - | 205 | 222 | - | 209 | 227 | - |
| 24 | 177 | 192 | - | 191 | 208 | - | 196 | 213 | - | 200 | 218 | - |
| 25 | 170 | 185 | - | 184 | 200 | - | 188 | 205 | - | 192 | 209 | - |
| 26 | 163 | 177 | - | 177 | 192 | - | 181 | 197 | - | 185 | 201 | - |
| 27 | 157 | 171 | 150 | 170 | 185 | - | 174 | 189 | - | 178 | 194 | - |
| 28 | 151 | 165 | 136 | 164 | 178 | - | 168 | 183 | - | 172 | 187 | - |
| 29 | 146 | 159 | 123 | 158 | 172 | 154 | 162 | 176 | - | 166 | 180 | - |
| 30 | 139 | 145 | 111 | 153 | 166 | 140 | 157 | 170 | - | 160 | 174 | - |
| 31 | 130 | 132 | 101 | 145 | 158 | 127 | 152 | 165 | - | 155 | 168 | - |
| 32 | 121 | 121 | 92 | 136 | 148 | 116 | 147 | 160 | 143 | 150 | 163 | - |
| 33 | 110 | 110 | 84 | 128 | 139 | 106 | 141 | 153 | 131 | 146 | 158 | - |
| 34 | 101 | 101 | 77 | 121 | 128 | 98 | 132 | 144 | 120 | 141 | 154 | - |
| 35 | 93 | 93 | 71 | 114 | 117 | 90 | 125 | 136 | 110 | 136 | 148 | 134 |
| 36 | 86 | 86 | 65 | 108 | 108 | 83 | 118 | 128 | 102 | 128 | 140 | 123 |
| 37 | 79 | 79 | 60 | 100 | 100 | 76 | 112 | 121 | 94 | 122 | 132 | 114 |
| 38 | 73 | 73 | 56 | 92 | 92 | 71 | 106 | 114 | 87 | 115 | 125 | 105 |
| 39 | 68 | 68 | 52 | 86 | 86 | 65 | 101 | 106 | 81 | 109 | 119 | 98 |
| 40 | 63 | 63 | 48 | 79 | 79 | 61 | 96 | 98 | 75 | 104 | 113 | 91 |

- Total Load values are limited by shear, moment, or deflection equal to $L / 180$.
- Deflection values (Deflect.) are limited by live load deflection equal to $L / 240$. Check the local building code for other deflection limits that may apply.
- Both the Total Load and Deflection columns must be checked. Where a Deflection value is not shown, the Total Load value will control.
- Table values apply to either simple or multiple span joists. Span is measured center to center of the minimum required bearing length. Analyze multiple span joists with the BC CALC ${ }^{\circledR}$ software if the length of any span is less than half the length of an adjacent span.
- Slope roof joists at least $1 / 4$ inch over 12 inches to minimize ponding.
of 16 " and less, web stiffeners required at all bearing locations for 18" and deeper joists.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC ${ }^{\circledR}$ software.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.
- Table values assume: minimum bearing lengths, no web stiffeners for joist depths

| Joist Series | Depth [inches] | Weight [plf] | Moment [ft-lbs] | $\left[\begin{array}{l} \mathrm{El} \times 106 \\ {\left[\mathrm{lb}-\mathrm{in}^{2}\right]} \end{array}\right.$ | $\begin{gathered} \mathrm{K} \times 10^{6} \\ {[\mathrm{lbs}]} \end{gathered}$ | Shear [lbs] | End Reaction [lbs] |  |  |  | Intermediate Reaction [lbs] |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | 11/2" Bearing (4) |  | 312/ Bearing |  | 31⁄2" Bearing |  | 51⁄4" Bearing |  |
|  |  |  |  |  |  |  | No WS(1) | WS(2) | No WS(1) | WS(2) | No WS(1) | WS(2) | No WS(1) | WS(2) |
| $\begin{gathered} \text { AJS® } \\ 20 \end{gathered}$ | 9112 | 2.5 | 3395 | 232 | 5.2 | 1250 | 950 | 1240 | 1175 | 1480 | 2350 | 2450 | 2350 | 2450 |
|  | 117/8 | 2.8 | 4400 | 394 | 6.6 | 1570 | 955 | 1335 | 1215 | 1595 | 2390 | 2800 | 2390 | 2800 |
|  | 14 | 3.0 | 5295 | 578 | 7.8 | 1850 | 960 | 1420 | 1250 | 1700 | 2430 | 3130 | 2430 | 3130 |
|  | 16 | 3.3 | 6140 | 786 | 9.0 | 2100 | 970 | 1500 | 1285 | 1800 | 2465 | 3435 | 2465 | 3435 |
| $\begin{gathered} \text { AJS® } \\ 190 \end{gathered}$ | 91/2 | 2.5 | 3895 | 244 | 5.2 | 1250 | 950 | 1240 | 1175 | 1480 | 2350 | 2450 | 2350 | 2450 |
|  | 117/8 | 2.8 | 5045 | 414 | 6.6 | 1570 | 955 | 1335 | 1215 | 1595 | 2390 | 2800 | 2390 | 2800 |
|  | 14 | 3.0 | 6070 | 608 | 7.8 | 1850 | 960 | 1420 | 1250 | 1700 | 2430 | 3130 | 2430 | 3130 |
|  | 16 | 3.3 | 7040 | 827 | 9.0 | 2100 | 970 | 1500 | 1285 | 1800 | 2465 | 3435 | 2465 | 3435 |
|  | 18 | 3.9 | 8045 | 1100 | 12.1 | 3010 | N/A ${ }^{(3)}$ | 2160 | N/A ${ }^{(3)}$ | 2620 | N/A ${ }^{(3)}$ | 4720 | N/A ${ }^{(3)}$ | 4720 |
|  | 20 | 4.2 | 8990 | 1402 | 13.5 | 3240 | N/A ${ }^{(3)}$ | 2160 | N/A ${ }^{(3)}$ | 2980 | N/A ${ }^{3}$ ) | 4780 | N/A ${ }^{3}$ ) | 5110 |
| $\begin{gathered} \text { AJS® } \\ 25 \end{gathered}$ | 91/2 | 3.1 | 5370 | 322 | 5.3 | 1250 | 950 | 1240 | 1175 | 1480 | 2600 | 2850 | 2600 | 2850 |
|  | 117/8 | 3.4 | 6960 | 545 | 6.7 | 1570 | 955 | 1335 | 1215 | 1595 | 2690 | 3190 | 2690 | 3190 |
|  | 14 | 3.7 | 8380 | 798 | 7.9 | 1850 | 960 | 1420 | 1250 | 1700 | 2770 | 3500 | 2770 | 3500 |
|  | 16 | 3.9 | 9720 | 1082 | 9.1 | 2100 | 970 | 1500 | 1285 | 1800 | 2850 | 3800 | 2850 | 3800 |
|  | 18 | 4.6 | 10975 | 1427 | 12.3 | 3010 | N/A ${ }^{(3)}$ | 2240 | N/A ${ }^{(3)}$ | 2620 | N/A ${ }^{(3)}$ | 4720 | N/A ${ }^{(3)}$ | 4720 |
|  | 20 | 4.9 | 12270 | 1813 | 13.7 | 3240 | N/A ${ }^{(3)}$ | 2490 | N/A ${ }^{(3)}$ | 2980 | N/A ${ }^{(3)}$ | 5110 | N/A ${ }^{(3)}$ | 5110 |
|  | 22 | 5.1 | 13455 | 2249 | 15.0 | 3470 | N/A ${ }^{(3)}$ | 2490 | N/A ${ }^{(3)}$ | 3150 | N/A ${ }^{(3)}$ | 5230 | N/A ${ }^{(3)}$ | 5505 |
|  | 24 | 5.4 | 14625 | 2737 | 16.5 | 3690 | N/A ${ }^{3}$ ) | 2490 | N/A ${ }^{(3)}$ | 3320 | N/A ${ }^{(3)}$ | 5345 | N/A ${ }^{(3)}$ | 5900 |
| $\begin{gathered} \text { AJS® } \\ 30 \end{gathered}$ | 18 | 4.6 | 13905 | 1575 | 12.3 | 3010 | N/A ${ }^{(3)}$ | 2240 | N/A ${ }^{(3)}$ | 2620 | N/A ${ }^{(3)}$ | 4720 | N/A ${ }^{(3)}$ | 4720 |
|  | 20 | 4.9 | 15540 | 1998 | 13.7 | 3240 | N/A ${ }^{(3)}$ | 2490 | N/A ${ }^{(3)}$ | 2980 | N/A ${ }^{(3)}$ | 5110 | N/A ${ }^{(3)}$ | 5110 |
|  | 22 | 5.1 | 17040 | 2477 | 15.0 | 3470 | N/A ${ }^{(3)}$ | 2490 | N/A ${ }^{(3)}$ | 3150 | N/A ${ }^{(3)}$ | 5230 | N/A ${ }^{(3)}$ | 5505 |
|  | 24 | 5.4 | 18525 | 3012 | 16.5 | 3690 | N/A ${ }^{(3)}$ | 2490 | N/A ${ }^{(3)}$ | 3320 | N/A ${ }^{(3)}$ | 5345 | N/A ${ }^{(3)}$ | 5900 |

NOTES:
(1) No web stiffeners required.
(2) Web stiffeners required.
(3) Not applicable, web stiffeners required.
(4) $13 / 4$ " minimum end bearing for 18 " and deeper joists.

- Moment, shear and reaction values based upon a load duration of $100 \%$ and may be adjusted for other load durations.
- Design values listed are applicable for Allowable Stress Design (ASD).
- No additional repetitive member increase allowed.

$$
\Delta=\frac{5 w l^{4}}{384 E I}+\frac{w l^{2}}{K}
$$

$\Delta=$ deflection [in]
$\mathrm{w}=$ uniform load [lb/in]
l = clear span [in]
EI = bending stiffness [lb-in²]
$\mathrm{K}=$ shear deformation coefficient [lb]

BUILDING CODE EVALUATION REPORT

- ICC ESR 1144 (IBC, IRC)


## AJS® Closest Allowable Nail Spacing / Diaphragm Design

Nailing Perpendicular to Glue Lines (Wide Face)


- For AJS® flange nailing, lateral and withdrawal design values may be determined per the National Design Specification (NDS) for Wood Construction, using a specific gravity of 0.42 .
- Nail spacing shall comply with the minimum spacing requirements prescribed by the NDS for nails installed in sawn lumber.
- AJS® joists may be substituted for sawn lumber framing in horizontal wood diaphragms, as referenced in section 2306.2 of the 2012 International Building Code to Table 4.2A of ANSI/ AF\&PA SDPWS-2002. Reductions in diaphragm values shall be calculated per note 2 using the 0.42 specific gravity value.
- Wood screws may be substituted for nails per approval of the local building official. Consult the fastener manufacturer for diaphragm values.

*18 - 24 inch deep rimboard are special order products, contact local supplier or Boise Cascade representative for product availability.



## BOISE CASCADE® Rimboard Properties

| Product | Vertical Load Capacity |  |  |  |  |  | Maximum Floor Diaphragm Lateral Capacity [ $\mathrm{lb} / \mathrm{ft}$ ] | Specific Gravity for Lateral Nail Design | Allowable Design Values |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Uniform [plf] |  |  | Point [lb] |  |  |  |  |  |  |  |  |
|  | $\begin{gathered} 16 " \\ \text { Depth } \\ \text { \& Less } \end{gathered}$ | $\begin{gathered} 18 " \& \\ 20 " \\ \text { Depth } \\ \text { \& Less } \end{gathered}$ | $\begin{gathered} 22^{\prime \prime} \text { \& } \\ 24 "^{\prime \prime} \\ \text { Depth } \\ \text { \& Less } \end{gathered}$ | $16 "$ Depth \& Less | $\begin{gathered} 18 " \& \\ 20 " \\ \text { Depth } \\ \text { \& Less } \end{gathered}$ | $\begin{gathered} 22 " \& \\ 24 " \\ \text { Depth } \\ \text { \& Less } \end{gathered}$ |  |  | Flexural Stress [lb/in²] | Modulus of Elasticity [lb/in²] | Horizonta Shear [lb/in²] | Compression Perpendicular to Grain [lb/in²] |
| $\begin{aligned} & \text { 1" BOISE CASCADE® } \\ & \text { RIMBOARD (2) \& } \\ & \text { 1" BOISE CASCADE® } \\ & \text { RIMBOARD OSB (2) } \end{aligned}$ | 3300 | 1650 | 1650 | 3500 | 3500 | 3500 | 180 | 0.5 | Limi | ed span ca | pabilities, | ee note 2 |
| 11⁄" BOISE CASCADE® RIMBOARD OSB (2) | 4400 | 3000 | 3000 | 3500 | 3500 | 3500 | 180 | 0.5 | Limi | d span ca | pabilities, | see note 2 |
| $\begin{gathered} \text { 15/16" } \\ \text { VERSA-LAM }^{\circledR} 1.41800(1) \end{gathered}$ | 6000 | 5450 | - | 4450 | 4450 | - | Permitted per building code for all nominal 2 " thick framing blocked and unblocked diaphragms (4" nail spacing \& greater) | 0.5 | 1800 | 1,400,000 | 225 | 525 |
| $\begin{gathered} 13 / 4 " \\ \text { VERSA-LAM }{ }^{\circledR} 2.03100 \text { (1) } \end{gathered}$ | 5700 | 4300 | - | 4300 | 3900 | - | Permitted per building code for all nominal 2" thick framing blocked and unblocked diaphragms (4" nail spacing \& greater) | 0.5 | 2800 | 2,000,000 | 285 | 750 |


|  | Closest Allowable Nail Spacing - Narrow Face [in] |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Product | $\begin{gathered} 8 \mathrm{~d} \\ \mathrm{Box} \end{gathered}$ | 8d Common | $\begin{gathered} \text { 10d \& 12d } \\ \text { Box } \end{gathered}$ | $\begin{aligned} & 16 d \\ & \text { Box } \end{aligned}$ | 10d, 12d Common \& 16d Sinker | 16d <br> Common |
| ```1" BOISE CASCADE® RIMBOARD (2)``` | 3 | 3 | - | - | - | - |
| 1 " or $111 / 8^{\prime \prime}$ BOISE CASCADE RIMBOARD OSB (2) | 3 | 3 | See note 2 for nailing information |  |  |  |
| 15/16" VERSA-LAM ${ }^{\circledR} 1.41800{ }^{(1)}$ | 3 | 3 | 3 | 3 | 4 | 6 |
| 13/4" VERSA-LAM ${ }^{\text {® }} 2.03100{ }^{(1)}$ | 2 | 3 | 3 | 3 | 4 | 6 |

## Notes

1. Per ICC ESR-1040.
2. See Performance Rated Rim Boards, APA EWS \#W345K for further product information.
3. Not all products and depths may be available, check with Boise Cascade representative for product availability.


When you specify VERSA-LAM® laminated veneer headers/beams, you are building quality into your design. They are excellent as floor and roof framing supports or as headers for doors, windows and garage doors and columns.

Because they have no camber, VERSA-LAM ${ }^{\circledR}$ LVL products provide flatter, quieter floors, and consequently, the builder can expect happier customers with significantly fewer call backs.

## VERSA-LAM ${ }^{\circledR}$ Beam Architectural Specifications

Scope: This work includes the complete furnishing and installation of all VERSALAM ${ }^{\circledR}$ beams as shown on the drawings, herein specified and necessary to complete the work.

Materials: Southern Pine or Douglas fir veneers, laminated in a press with all grain parallel with the length of the member. Glues used in lamination are phenol formaldehyde and isocyanate exterior-type adhesives which comply with ASTM D2559.

Design: VERSA-LAM ${ }^{\circledR}$ beams shall be sized and detailed to fit the dimensions and loads indicated on the plans. All designs shall be in accordance with allowable values developed in accordance with ASTM D5456 and listed in the governing
code evaluation service's report and section properties based upon standard engineering principles. Verification of design of the VERSA-LAM ${ }^{\circledR}$ beams by complete calculations shall be available upon request.

Drawings: Additional drawings showing layout and detail necessary for determining fit and placement in the buildings are (are not) to be provided by the supplier.

Fabrication: VERSA-LAM ${ }^{\circledR}$ beams shall be manufactured in a plant evaluated for fabrication by the governing code evaluation service and under the supervision of a third-party inspection agency listed by the corresponding evaluation service.

Storage and Installation: VERSALAM ${ }^{\circledR}$ beams, if stored prior to erection, shall be stored on stickers spaced a maximum of 15 ft . apart. Beams shall be stored on a dry, level surface and protected from the weather. They shall be handled with care so they are not damaged.
VERSA-LAM ${ }^{\circledR}$ beams are to be installed in accordance with the plans and Boise Cascade EWP's Installation Guide. Temporary construction loads which cause stresses beyond design limits are not permitted. Erection bracing shall be provided to assure adequate lateral support for the individual beams and the entire system until the sheathing material has been applied.
Codes: VERSA-LAM ${ }^{\circledR}$ beams shall be evaluated by a model code evaluation service.

## Allowable Holes in VERSA-LAM® Beams

## Notes

1. Square and rectangular holes are not permitted.
2. Round holes may be drilled or cut with a hole saw anywhere within the shaded area of the beam.
3. The horizontal distance between adjacent holes must be at least two times the size of the larger hole.
4. Do not drill more than three access holes in any four foot long section of beam.
5. The maximum round hole diameter permitted is:

| Beam Depth | Max. Hole Diameter |
| :---: | :---: |
| $51 / 2^{\prime \prime}$ | $3 / 4^{\prime \prime}$ |
| $71 / 4^{\prime \prime}$ | $1 "$ |
| $91 / 4^{\prime \prime}$ and greater | $2 "$ |


6. These limitations apply to holes drilled for plumbing or wiring access only. The size and location of holes drilled for fasteners are governed by the provisions of the National Design Specification ${ }^{\circledR}$ for Wood Construction.
7. Beams deflect under load. Size holes to provide clearance where required.
8. This hole chart is valid for beams supporting uniform load only. For beams supporting concentrated loads or for beams with larger holes, contact Boise Cascade EWP Engineering.

## VERSA-LAM® Beam Details



VERSA-LAM® Installation Notes

- Minimum of $1 / 2^{\prime \prime}$ air space between beam and wall pocket or adequate barrier must be provided between beam and concrete/masonry
- Adequate bearing shall be provided. If not shown on plans, please refer to load tables in
your region's Specifier Guide.
- VERSA-LAM ${ }^{\circledR}$ beams are intended for interior applications only and should be kept as dry as possible during construction
- Continuous lateral support of top of beam shall be provided (side or top bearing framing).


## Multiple Member Connectors

| Side-Loaded Applications |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Number <br> of <br> Members | Maximum Uniform Side Load [plf] |  |  |  |  |  |  |  |
|  | Nailed |  | $1 / 2$ " Dia. Through Bolt ${ }^{(1)}$ |  |  | 5/8" Dia. Through Bolt ${ }^{(1)}$ |  |  |
|  | $\begin{array}{\|c\|} \hline 2 \text { rows } 16 \mathrm{~d} \\ \text { Sinkers @ } \\ 12^{\prime \prime} \text { o.c. } \end{array}$ | 3 rows 16 d <br> Sinkers @ 12" o.c. | 2 rows @ 24" o.c. staggered | $\begin{aligned} & \hline 2 \text { rows @ } \\ & \text { 12" o.c. } \\ & \text { staggered } \end{aligned}$ | 2 rows @ 6" o.c. staggered | $\begin{aligned} & \hline 2 \text { rows @ } \\ & 24 \text { " o.c. } \\ & \text { staggered } \end{aligned}$ | $\begin{array}{\|l\|} \hline 2 \text { rows @ } \\ 12 " \text { o.c. } \\ \text { staggered } \end{array}$ | 2 rows @ 6" o.c. staggered |
| 13/4" VERSA-LAM ${ }^{\text {® }}$ (Depths of 18" and less) |  |  |  |  |  |  |  |  |
| 2 | 470 | 705 | 505 | 1010 | 2020 | 560 | 1120 | 2245 |
| $3{ }^{(2)}$ | 350 | 525 | 375 | 755 | 1515 | 420 | 840 | 1685 |
| 4(3) | use bolt schedule |  | 335 | 670 | 1345 | 370 | 745 | 1495 |
| 3½" VERSA-LAM ${ }^{\text {® }}$ |  |  |  |  |  |  |  |  |
| 2(3) | use bolt schedule |  | 855 | 1715 | N/A | 1125 | 2250 | N/A |
| 13/4" VERSA-LAM ${ }^{\circledR}$ (Depths of 24") |  |  |  |  |  |  |  |  |
| $\left\|\begin{array}{c} \text { Number } \\ \text { of } \\ \text { Members } \end{array}\right\|$ | Nailed |  | ½ Dia. Through Bolt ${ }^{(1)}$ |  |  | 5/8" Dia. Through Bolt ${ }^{(1)}$ |  |  |
|  | $\begin{array}{\|c\|} \hline 3 \text { rows 16d } \\ \text { Sinkers @ } \\ 12 " \text { o.c. } \\ \hline \end{array}$ | 4 rows 16d Sinkers @ 12" o.c. | $\begin{array}{\|l\|} \hline 3 \text { rows @ } \\ 24 " \text { o.c. } 8 \\ \text { staggered } \end{array}$ | $\begin{aligned} & \hline 3 \text { rows @ } \\ & \text { 18" o.c. 6" } \\ & \text { staggered } \end{aligned}$ | $\begin{array}{\|l\|l\|} \hline 3 \text { rows @ } \\ 12 " \text { ".c. } \mathbf{4 "}^{\prime} \\ \text { staggered } \end{array}$ | $\begin{array}{\|l\|} \hline 3 \text { rows @ } \\ 24 " \text { o.c. } 8 " \\ \text { staggered } \\ \hline \end{array}$ | $\left.\begin{array}{\|l\|} \hline 3 \text { rows @ } \\ 18 " \text { o.c. }{ }^{\prime \prime} \\ \text { staggered } \end{array} \right\rvert\,$ | $\begin{array}{\|l\|} \hline 3 \text { rows @ } \\ 12 " \text { o.c. } 4^{\prime \prime} \\ \text { staggered } \\ \hline \end{array}$ |
| 2 | 705 | 940 | 755 | 1010 | 1515 | 840 | 1120 | 1685 |
| 3(2) | 525 | 705 | 565 | 755 | 1135 | 630 | 840 | 1260 |
| $4{ }^{(3)}$ | use bolt | schedule | 505 | 670 | 1010 | 560 | 745 | 1120 |

1. Design values apply to common bolts that conform to ANSI/ ASME standard B18.21-1981 (ASTM A307 Grades A\&B, SAE J429 Grades 1 or 2 , or higher). A washer not less than a standard cut washer shall be between the wood and the bolt head and between the wood and the nut. The distance from the edge of the beam to the bolt holes must be at leas
$2^{\prime \prime}$ for $1 / 2$ bolts and $2^{1} / 2$ " for $5 / 8$ " bolts. Bolt holes shall be the same diameter as the bolt.
2. The nail schedules shown apply to both sides of a 3 -member beam.
3. 7" wide beams must be top-loaded or loaded from both sides (lesser side shall be no less than $25 \%$ of opposite side).

## Top-Loaded Applications

| Plies | Depth | Nailing | Maximum Uniform Load From One Side |
| :---: | :---: | :---: | :---: |
| (2) $13 / 4 \mathrm{l}$ plies | Depths $117 / 8$ \& less | 2 rows 16d box/sinker nails @ 12" o.c. | 400 plf |
|  | Depths 14" - 18" | 3 rows 16d box/sinker nails @ 12" o.c. | 600 plf |
|  | Depth $=24{ }^{\prime \prime}$ | 4 rows 16d box/sinker nails @ 12" o.c. | 800 plf |
| (3) $13 / 4$ " plies | Depths $117 / 8{ }^{\text {" }}$ \& less | 2 rows 16d box/sinker nails @ 12" o.c., both sides | 300 plf |
|  | Depths 14"-18" | 3 rows 16d box/sinker nails @ 12" o.c., both sides | 450 plf |
|  | Depth $=24{ }^{\prime \prime}$ | 4 rows 16d box/sinker nails @ 12" o.c., both sides | 600 plf |
| (4) $13 / 4$ " plies | Depths 18" \& less | 2 rows $1 / 2$ " bolts @ 24" o.c., staggered | 335 plf |
|  | Depth = 24" | 3 rows $1 / 2$ " bolts @ 24" o.c., staggered every 8" | 505 plf |
| (2) $3112 / \mathrm{plies}$ | Depths 18" \& less | 2 rows $1 / 2$ " bolts @ 24" o.c., staggered | 855 plf |
|  | Depth 20" - 24" | 3 rows $1 / 2$ " bolts @ 24" o.c., staggered every 8" | 1285 plf |

1. Beams wider than 7 " must be designed by the engineer of record.
2. All values in these tables may be increased by $15 \%$ for snow-load roofs and by $25 \%$ for non-snow load roofs where the building code allows.
3. Use allowable load tables or BC CALC® software to size beams.
4. An equivalent specific gravity of 0.5 may be used when designing specific connections with VERSA-LAM ${ }^{\circledR}$
5. Connection values are based upon the 2012 NDS.
6. FastenMaster TrussLok, Strong-Tie SDS and SDW, and USP WS screws may also be used to connect multiple member VERSALAM ${ }^{\circledR}$ beams, contact Boise Cascade EWP Engineering for urther information

## Designing Connections for Multiple VERSA-LAM ${ }^{\circledR}$ Members

When using multiple ply VERSA-LAM ${ }^{\circledR}$ beams to create a wider member, the connection of the plies is as critical as determining the beam size. When side loaded beams are not connected properly, the inside plies do not support their share of the load and thus the load-carrying capacity of the full member decreases significantly. The following is an example of how to size and connect a multiple-ply VERSA-LAM ${ }^{\circledR}$ floor beam.

Given: Beam shown below is supporting residential floor load (40 psf live load, 10 psf dead load) and is spanning $16^{\prime}-0^{\prime \prime}$. Beam depth is limited to $14^{\prime \prime}$


Find: A multiple $13 / 4$ " ply VERSA-LAM ${ }^{\circledR}$ that is adequate to support the design loads and the member's proper connection schedule.

1. Calculate the tributary width that beam is supporting: $14^{\prime} / 2+18^{\prime} / 2=16^{\prime}$
2. Use PLF tables on page 40 of ECG or BC CALC ${ }^{\circledR}$ to size beam. A Triple VERSA-LAM ${ }^{\oplus} 2.0310013 / 4^{\prime \prime} \times 14^{\prime \prime}$ is found to adequately support the design loads
3. Calculate the maximum plf load from one side (the right side in this case)

Max. Side Load $=\left(18^{\prime} / 2\right) \times(40+10 \mathrm{psf})=450$ plf
4. Go to the Multiple Member Connection Table, Side-Loaded Applications, $13 / 4$ " VERSA-LAM ${ }^{\circledR}$, 3 members.
5. The proper connection schedule must have a capacity greater than the max. side load:

Nailed: 3 rows 16 d sinkers @ 12" o.c: 525 plf is greater than 450 plf OK Bolts: $1 / 2{ }^{\prime \prime}$ diameter 2 rows @ 12" staggered: 755 plf is greater than 450 plf OK

## Heavy Storage: 250 psf Live Load / 25 psf Dead Load

|  | 1½' VERSA-LAM ${ }^{\circledR} 1.72650$ |  |  |  | 1½" VERSA-LAM® 1.72650 |  |  |  |  | 13/4" VERSA-LAM ${ }^{\circledR} 2.03100$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Sp | $71 / 4$ | 91/4 | 111/4 | 14 | 71/4 | 91/2 | 117/8 | 14 | 16 | 91/2 | 117/8 | 14 | 16 |
| 12 | 7'-8" | 9'-10" | 12'-0" | 15'-0" | 8'-1' | 10'-8" | 13'-4" | 15'-9" | 18'-1" | 11'-3" | 14'-1" | 16'-8' | 19'-1" |
| $16^{\prime \prime}$ | 7'-0' | 8'-11" | 10'-11" | 13'-7" | 7'-4" | 9'-8" | 12'-1' | 14'-4" | 16'-5" | 10'-3' | 12'-10" | 15'-1" | 17'-4' |
| 19.2" | $6^{\prime}-7{ }^{\prime \prime}$ | 8'-5" | 10'-3" | 12'-9" | 6'-11' | 9'-1" | 11'-5" | 13'-6" | 15'-5" | 9'-7" | 12'-0" | 14'-3" | 16'-3" |
| $24 "$ | 5'-10" | 7'-6" | 9'-1" | 11'-4" | 6'-5" | 8'-5' | 10'-7" | 12'-6" | 14'-3" | 8'-11" | 11'-2" | 13'-2" | 15'-1" |

- Loading based upon Heavy Storage - Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of L/240. Live load deflection limited to L/600 with 50 psf, L/360 with 250 psf.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.
- Table values assume web stiffeners at each bearing location, minimum $21 / 4$ " end bearing.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® ${ }^{\circledR}$ sizing software.
- Table based upon bearing supports of $2 \times 6$ framing.
- Bold spans indicate applications where bearing wider than $2 \times 6$ framing may be needed at intermediate supports.


## Light Storage: 125 psf Live Load / 25 psf Dead Load

| O.C. Spacing | 1½ VERSA-LAM ${ }^{\circledR} 1.72650$ |  |  |  | 13/4" VERSA-LAM ${ }^{\circledR} 1.72650$ |  |  |  |  | 13/4" VERSA-LAM® ${ }^{\text {® }} 2.03100$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $71 / 4$ | 91/4 | 111/4 | 14 | $71 / 4$ | 91/2 | 117/8 | 14 | 16 | 91/2 | 117/8 | 14 | 16 |
| 12" | 9'-9" | 12'-5" | 15'-2" | 18'-11" | 10'-3" | 13'-6" | 16'-10" | 19'-11" | 22'-9" | 14'-3" | 17'-10" | 21'-0" | 24'-1' |
| $16 "$ | 8'-10" | 11'-3" | 13'-9" | 17'-2" | 9'-3' | 12'-3" | 15'-4" | 18'-1" | 20'-8" | 12'-11" | 16'-2" | 19'-1" | 21'-10" |
| 19.2" | 8'-3' | 10'-7" | 12'-11" | 16'-2" | 8'-9" | 11'-6" | 14'-5" | 17'-0" | 19'-5" | 12'-2" | 15'-2" | 17'-11" | 20'-6" |
| $24 "$ | 7'-8" | 9'-10" | 12'-0" | 15'-0" | 8'-1' | 10'-8" | 13'-4" | 15'-9" | 18'-1" | 11'-3" | 14'-1" | 16'-8" | 19'-1" |

- Loading based upon Light Storage - Table 1607.1 of 2009/2012 International Building Code.
- Spans limited by allowable moment and reaction values, total load deflection of $\mathrm{L} / 240$. Live load deflection limited to L/600 with 50 psf, L/360 with 125 psf.
- Table values represent the most restrictive of simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports.

Table values assume web stiffeners at each bearing location, minimum $2 \frac{1}{4}$ " end bearing.

- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® ${ }^{\circledR}$ sizing software.
- Table based upon bearing supports of $2 \times 6$ framing.
- Bold spans indicate applications where bearing wider than $2 \times 6$ framing may be needed at intemediate supports.


# VERSA-LAM® 2.03100 (100\% Load Duration) 



- Total Load values are limited by shear, moment or deflection equal to L/240. Total Load values are the capacity of the beam in addition to its own weight.
Live Load values are limited by deflection equal to $L / 360$. Check the local building code for other deflection limits that may apply.
Where a Live Load value is not shown, the Total Load value will control.
- Table values represent the most restrictive of simple or multiple span applications. Span is measured center to center of the supports. Analyze multiple span beams with the BC CALC® software if the length of any span is less than half the length of an adjacent span.
- Table values assume that lateral support is provided at each support and continuously along the top edge and applicable compression edges of the beam.
- Table values for Minimum Required Bearing Lengths are based on the allowable compression design value perpendicular to grain for the beam and the Total Load value shown. Other design considerations, such as a weaker support material, may warrant longer bearing lengths. Table values assume that support is provided across the full width of the beam.
- For 2-ply, 3-ply or 4-ply beams; double, triple or quadruple Allowable Total Load and Allowable Live Load values. Minimum Required Bearing Lengths remain the same for any number of plies.
- $13 / 4$ inch members deeper than 14 inches are to be used as multiple-member beams only.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® software.


## VERSA-LAM® 2.03100 (115\% Load Duration)

|  |  |  | KEY TO TABLE |  |  |  |  | Top Figure |  |  | Allowable Total Load [plf] |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | Middle Figure - Allowable Live Load [plf] |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  | Bottom Figures |  |  | Minimum Required Bearing Length at End / Intermediate Supports [inches] |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Span [ft] | 13/4" VERSA-LAM ${ }^{\text {® }} 2.03100$ |  |  |  | Double Ply 13/4" VERSA-LAM ${ }^{\oplus} 2.03100$ or 3½" VERSA-LAM 2.03100 |  |  |  |  |  |  | Triple Ply 13/4" VERSA-LAM ${ }^{\circledR} 2.03100$ or 5¼" VERSA-LAM 2.03100 |  |  |  |  |  |  | Quadruple Ply $13 / 4^{\prime \prime}$ VERSA-LAM ${ }^{\circledR} 2.03100$ or 7" VERSA-LAM 2.03100 |  |  |  |  |  |
|  | 71/4" | 91⁄2" | 117/8" | 14" | 71/4" | 9112" | 117/8" | 14" | 16" | 18" | 24" | 91⁄2" | 117/8" | 14" | 16" | 18" | 20" | 24" | 117/8" | $14 "$ | 16" | 18" | 20" | 24" |
| 6 | 878 | 1223 | 1639 | 2065 | 1755 | 2446 | 3278 | 4130 | 5047 | 5232 | 5226 | 3669 | 4917 | 6195 | 7570 | 7848 | 7845 | 7838 | 6556 | 8260 | 10094 | 10463 | 10459 | 10451 |
|  | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |
|  | 2/5 | 2.8/7 | 3.8/9.4 | 4.7/11.8 | 2/5 | 2.8/7 | 3.8/9.4 | 4.7111 .8 | 5.8/14.5 | 6/15 | 6/15 | 2.8/7 | 3.8/9.4 | 4.7/11.8 | 5.8/14.5 | 6/15 | 6/15 | 6/15 | 3.8/9.4 | 7/111.8 | .8/14.5 | 6/15 | 6/15 | 6/15 |
| 8 | 598 | 858 | 1126 | 1389 | 1197 | 1717 | 2252 | 2779 | 3321 | 3915 | 3913 | 2575 | 3379 | 4168 | 4981 | 5872 | 5876 | 5870 | 4505 | 5558 | 6642 | 7829 | 7834 | 7826 |
|  | 482 | - | - | - | 965 | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |
|  | 1.8/4.6 | 2.6/6.6 | 3.5/8.6 | 4.3/10.6 | 1.8/4.6 | 2.6/6.6 | 3.5/8.6 | 4.3/10.6 | 1/12.7 | 6/15 | 6/15 | 2.6/6.6 | 3.5/8.6 | 3/10.6 | 1/12.7 | 6/15 | 6/15 | 6/15 | 3.5/8.6 | 3/10. | .1/12.7 | 6/15 | 6/15 | 6/15 |
| 10 | 326 | 637 | 857 | 1046 | 651 | 1274 | 1714 | 2092 | 2472 | 2880 | 3126 | 1912 | 2571 | 3138 | 3709 | 4320 | 4695 | 4688 | 3429 | 4184 | 4945 | 5759 | 6259 | 6251 |
|  | 247 | 556 | - | - | 494 | 1111 | - | - | - | - | - | 1667 | - | - | - | - | - | - | - | - | - | - | - | - |
|  | 1.5/3.1 | 2.4/6.1 | 3.3/8.2 | 4/10 | 1.5/3.1 | 2.4/6.1 | 3.3/8.2 | 4/10 | 4.7/11.9 | 5/13.8 | 6/15 | 2.4/6.1 | 3.3/8.2 | 4/10 | 4.7/11.9 | 5/13.8 | 6/15 | 6/15 | 3.3/8.2 | 4/10 | 4.7/11.9 | 5/13.8 | 6/15 | 6/15 |
| 11 | 244 | 526 | 765 | 931 | 487 | 1052 | 1531 | 1861 | 2192 | 2543 | 2839 | 1577 | 2296 | 2792 | 3288 | 3814 | 4265 | 4259 | 3062 | 3723 | 4383 | 5085 | 5687 | 5679 |
|  | 186 | 418 | - | - | 371 | 835 | - | - | - | - | - | 1253 | - | - | - | - | - | - | - | - | - | - | - | - |
|  | 1.5/3 | 2.2/5.6 | 3.2/8.1 | 3.9/9.8 | 1.5/3 | 2.2/5.6 | 3.2/8.1 | 3.9/9.8 | .6/11.6 | 4/13.4 | 6/15 | 2.2/5.6 | 3.2/8.1 | 3.9/9.8 | .6/11.65 | 4/13.4 | 6/15 | 6/15 | 3.2/8.1 | 3.9/9.8 | 4.6/11.6 | 4/13.4 | 6/15 | 6/15 |
| 12 | 187 | 424 | 674 | 838 | 374 | 848 | 1347 | 1676 | 1968 | 2276 | 2601 | 1272 | 2021 | 2514 | 2952 | 3414 | 3903 | 3901 | 2694 | 3353 | 3936 | 4552 | 5203 | 5201 |
|  | 143 | 322 | 628 | - | 286 | 643 | 1256 | - | - | - | - | 965 | 1884 | - | - | - | - | - | 2512 | - | - | - | - | - |
|  | 1.5/3 | 2/4.9 | 3.1/7.8 | 3.9/9.7 | 1.5/3 | 2/4.9 | 3.1/7.8 | 3.9/9.7 | 4.5/11.3 | 2/13.1 | 6/15 | 2/4.9 | 3.1/7.8 | 3.9/9.7 | 4.5/11.3 | 5.2/13.1 | 6/15 | 6/15 | 3.1/7.8 | 3.9/9.7 | 4.5/11.3 | .2/13.1 | 6/15 | 6/15 |
| 13 | 146 | 332 | 573 | 762 | 292 | 665 | 1146 | 1524 | 1785 | 2060 | 2399 | 997 | 1719 | 2287 | 2678 | 3089 | 3522 | 3598 | 2292 | 3049 | 3571 | 4119 | 4696 | 4797 |
|  | 112 | 253 | 494 | - | 225 | 506 | 988 | - | - | - | - | 759 | 1482 | - | - | - | - | - | 1976 | - | - | - | - | - |
|  | 1.5/3 | 1.7/4.2 | 2.9/7.2 | 3.8/9.5 | 1.5/3 | 1.7/4.2 | 2.9/7.2 | 2 3.8/9.5 | 4.5/11.2 | 5.1/12.9 | 6/15 | 1.7/4.2 | 2.9/7.2 | 3.8/9.5 | 4.5/11.2 | 5.1/12.95 | 5.9/14.7 | 6/15 | 2.9/7.2 | 3.8/9.5 | 4.5/11.2 | 5.1/12.95 | .9/14.7 | 6/15 |
| 14 | 116 | 265 | 493 | 674 | 233 | 530 | 987 | 1349 | 1634 | 1880 | 2226 | 796 | 1480 | 2023 | 2450 | 2821 | 3208 | 3338 | 1973 | 2697 | 3267 | 3761 | 4278 | 4451 |
|  | 90 | 203 | 396 | 648 | 180 | 405 | 791 | 1296 | - | - | - | 608 | 1187 | 1944 | - | - | - | - | 1582 | 2593 | - | - | - | - |
|  | 1.5/3 | 1.5/3.6 | 2.7/6.7 | 3.6/9.1 | 1.5/3 | 1.5/3.6 | $2.7 / 6.7$ | 7 3.6/9.1 | 4.4/11 | 5.1/12.7 | 6/15 | 1.5/3.6 | 2.716.7 | 3.6/9.1 | 4.4/11 | 5.1/12.75 | 5.8/14.4 | 6/15 | 2.716.7 | 3.6/9.1 | 4.4/11 | 5.1/12.75 | .8/14.4 | 6/15 |
| 15 | 94 | 215 | 423 | 586 | 188 | 429 | 846 | 1173 | 1505 | 1730 | 2076 | 644 | 1268 | 1759 | 2258 | 2595 | 2946 | 3113 | 1691 | 2346 | 3011 | 3459 | 3928 | 4151 |
|  | 73 | 165 | 322 | 527 | 146 | 329 | 643 | 1054 | - | - | - | 494 | 965 | 1581 | - | - | - | - | 1286 | 2108 | - | - | - | - |
|  | 1.5/3 | 1.5/3.1 | 2.5/6.1 | 3.4/8.5 | 1.5/3 | 1.5/3.1 | 2.5/6.1 | $13.4 / 8.5$ | 4.3/10.9 | 5/12.5 | 6/15 | 1.5/3.1 | 2.5/6.1 | 3.4/8.5 | 4.3/10.9 | 5/12.5 | 5.7/14.2 | 6/15 | 2.5/6.1 | 3.4/8.5 | 4.3/10.9 | 5/12.5 | 5.7/14.2 | 6/15 |
| 16 | 77 | 176 | 347 | 515 | 153 | 352 | 695 | 1029 | 1327 | 1601 | 1944 | 528 | 1042 | 1544 | 1990 | 2402 | 2723 | 2917 | 1389 | 2058 | 2653 | 3202 | 3630 | 3889 |
|  | 60 | 136 | 265 | 434 | 121 | 271 | 530 | 868 | 1296 | - | - | 407 | 795 | 1303 | 1944 | - | - | - | 1060 | 1737 | 2593 | - | - | - |
|  | 1.5/3 | 1.5/3 | 2.2/5.4 | 3.2/7.9 | 1.5/3 | 1.5/3 | 2.2/5.4 | 4 3.2/7.9 | 4.1/10.2 | 4.9/12.3 | 6/15 | 1.5/3 | 2.2/5.4 | 3.2/7.9 | 4.1/10.2 | 4.9/12.3 | 5.6/14 | 6/15 | 2.2/5.4 | 3.2/7.9 | 4.1/10.2 | .9/12.3 | 5.6/14 | 6/15 |
| 17 | 63 | 146 | 289 | 455 | 127 | 292 | 577 | 910 | 1173 | 1468 | 1829 | 438 | 866 | 1365 | 1760 | 2201 | 2531 | 2743 | 1154 | 1820 | 2346 | 2935 | 3374 | 3657 |
|  | 50 | 113 | 221 | 362 | 101 | 226 | 442 | 724 | 1081 | - | - | 339 | 663 | 1086 | 1621 | - | - | - | 884 | 1448 | 2161 | - | - | - |
|  | 1.5/3 | 1.5/3 | 1.9/4.8 | 3/7.5 | 1.5/3 | 1.5/3 | 1.9/4.8 | 3/7.5 | 3.9/9.6 | 4.8/12 | 6/15 | 1.5/3 | 1.9/4.8 | 3/7.5 | 3.9/9.6 | 4.8/12 | 5.5/13.8 | 6/15 | 1.9/4.8 | 3/7.5 | 3.9/9.6 | 4.8/12 | 5/13.8 | 6/15 |
| 18 | 53 | 122 | 242 | 399 | 106 | 244 | 484 | 799 | 1045 | 1307 | 1726 | 367 | 726 | 1198 | 1567 | 1961 | 2364 | 2588 | 968 | 1598 | 2089 | 2614 | 3151 | 3451 |
|  | 42 | 95 | 186 | 305 | 85 | 191 | 372 | 610 | 910 | 1296 | - | 286 | 558 | 915 | 1366 | 1944 | - | - | 744 | 1220 | 1821 | 2593 | - | - |
|  | 1.5/3 | 1.5/3 | 1.7/4.3 | 2.8/7 | 1.5/3 | 1.5/3 | 1.7/4.3 | 2.8/7 | 3.6/9.1 | 4.5/11.4 | 6/15 | 1.5/3 | 1.7/4.3 | 2.8/7 | 3.6/9.1 | 4.5/11.45 | 5.5/13.7 | 6/15 | 1.7/4.3 | 2.8/7 | 3.6/9.1 | 4.5/11.4 | .5/13.7 | 6/15 |
| 19 |  | 103 | 205 | 339 | 89 | 206 | 410 | 677 | 936 | 1171 | 1634 | 310 | 615 | 1016 | 1404 | 1757 | 2147 | 2450 | 820 | 1354 | 1872 | 2342 | 2862 | 3267 |
|  |  | 81 | 158 | 259 | 72 | 162 | 316 | 519 | 774 | 1102 | - | 243 | 475 | 778 | 1161 | 1653 | - | - | 633 | 1037 | 1548 | 2204 | - | - |
|  |  | 1.5/3 | 1.5/3.8 | 2.5/6.3 | 1.5/3 | 1.5/3 | 1.5/3.8 | 2.5/6.3 | 3.4/8.6 | 4.3/10.8 | 6/15 | 1.5/3 | 1.5/3.8 | 2.5/6.3 | 3.4/8.6 | 4.3/10.85 | 5.3/13.1 | 6/15 | 1.5/3.8 | 2.5/6.3 | 3.4/8.6 | 4.3/10.85 | .3/13.1 | 6/15 |
| 20 |  | 88 | 175 | 289 | 75 | 176 | 350 | 579 | 843 | 1055 | 1551 | 263 | 525 | 868 | 1265 | 1583 | 1934 | 2326 | 699 | 1157 | 1686 | 2110 | 2579 | 3101 |
|  |  | 69 | 136 | 222 | 62 | 139 | 271 | 445 | 664 | 945 | - | 208 | 407 | 667 | 996 | 1418 | - | - | 543 | 889 | 1327 | 1890 | - | - |
|  |  | 1.5/3 | 1.5/3.4 | 2.3/5.6 | 1.5/3 | 1.5/3 | 1.5/3.4 | 2.3/5.6 | 3.3/8.2 | 4.1/10.2 | 6/15 | 1.5/3 | 1.5/3.4 | 2.3/5.6 | 3.3/8.2 | 4.1/10.2 | 5/12.5 | 6/15 | 1.5/3.4 | 2.3/5.6 | 3.3/8.2 | 4.1/10.2 | 5/12.5 | 6/15 |
| 22 |  | 65 | 130 | 216 | 54 | 130 | 260 | 431 | 649 | 869 | 1407 | 194 | 390 | 647 | 973 | 1303 | 1593 | 2111 | 520 | 862 | 1297 | 1738 | 2124 | 2815 |
|  |  | 52 | 102 | 167 | 46 | 104 | 204 | 334 | 499 | 710 | - | 157 | 306 | 501 | 748 | 1065 | 1461 | - | 408 | 668 | 997 | 1420 | 1948 | - |
|  |  | 1.5/3 | 1.5/3 | 1.9/4.7 | 1.5/3 | 1.5/3 | 1.5/3 | 1.9/4.7 | 2.8/7 | 3.7/9.3 | 6/15 | 1.5/3 | 1.5/3 | 1.9/4.7 | 2.8/7 | 3.7/9.3 | 4.5/11.3 | 6/15 | 1.5/3 | 1.9/4.7 | 2.8/7 | 3.7/9.3 | 4.5/11.3 | 6/15 |
| 24 |  |  | 99 | 164 |  | 98 | 197 | 329 | 496 | 711 | 1259 | 146 | 296 | 493 | 744 | 1066 | 1334 | 1889 | 395 | 658 | 992 | 1422 | 1779 | 2518 |
|  |  |  | 79 | 129 |  | 80 | 157 | 257 | 384 | 547 | - | 121 | 236 | 386 | 576 | 820 | 1125 | - | 314 | 515 | 768 | 1094 | 1500 | - |
|  |  |  | 1.5/3 | 1.6/3.9 |  | 1.5/3 | 1.5/3 | 1.6/3.9 | 2.3/5.9 | 3.3/8.3 | 5.9/14.7 | 1.5/3 | 1.5/3 | 1.6/3.9 | 2.3/5.9 | 3.3/8.3 | 4.2/10.45 | 5.9/14.7 | 1.5/3 | 1.6/3.9 | 2.3/5.9 | 3.3/8.3 | 4.2/10.45 | 5.9/14.7 |
| 26 |  |  | 76 | 128 |  | 75 | 153 | 256 | 387 | 555 | 1069 | 112 | 229 | 383 | 580 | 833 | 1132 | 1604 | 305 | 511 | 773 | 1110 | 1510 | 2139 |
|  |  |  | 62 | 101 |  | 63 | 124 | 202 | 302 | 430 | 1020 | 95 | 185 | 304 | 453 | 645 | 885 | 1529 | 247 | 405 | 604 | 860 | 1180 | 2039 |
|  |  |  | 1.5/3 | 1.5/3.3 |  | 1.5/3 | 1.5/3 | 1.5/3.3 | $2 / 5$ | 2.8/7.1 | 5.4/13.5 | 1.5/3 | 1.5/3 | 1.5/3.3 | $2 / 5$ | 2.8/7.1 | 3.8/9.6 | 5.4/13.5 | 1.5/3 | 1.5/3.3 | $2 / 5$ | 2.8/7.1 | 3.8/9.6 5 | 5.4/13.5 |
| 28 |  |  | 60 | 101 |  | 58 | 120 | 202 | 306 | 441 | 919 | 87 | 180 | 303 | 459 | 661 | 914 | 1378 | 240 | 404 | 612 | 882 | 1219 | 1837 |
|  |  |  | 49 | 81 |  | 51 | 99 | 162 | 242 | 344 | 816 | 76 | 148 | 243 | 363 | 517 | 709 | 1224 | 198 | 324 | 484 | 689 | 945 | 1633 |
|  |  |  | 1.5/3 | 1.5/3 |  | 1.5/3 | 1.5/3 | 1.5/3 | 1.7/4.3 | 2.4/6.1 | 5/12.6 | 1.5/3 | 1.5/3 | 1.5/3 | 1.7/4.3 | 2.4/6.1 | 3.4/8.4 | 5/12.6 | 1.5/3 | 1.5/3 | 1.7/4.3 | 2.4/6.1 | 3.4/8.4 | 5/12.6 |
| 30 |  |  |  | 81 |  |  | 95 | 161 | 246 | 355 | 797 | 68 | 143 | 242 | 369 | 533 | 738 | 1196 | 190 | 323 | 492 | 710 | 984 | 1594 |
|  |  |  |  | 66 |  |  | 80 | 132 | 197 | 280 | 664 | 62 | 121 | 198 | 295 | 420 | 576 | 996 | 161 | 263 | 393 | 560 | 768 | 1327 |
|  |  |  |  | 1.5/3 |  |  | 1.5/3 | 1.5/3 | 1.5/3.7 | 2.1/5.3 | 4.7/11.7 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.7 | 2.1/5.3 | 2.9/7.3 | 4.7/11.7 | 1.5/3 | 1.5/3 | 1.5/3.7 | 2.1/5.3 | 2.9/7.3 | 4.7/11.7 |

[^0]
## VERSA-LAM ${ }^{\circledR}$ Roof Load Tables

## VERSA-LAM ${ }^{\circledR} 2.03100$ (125\% Load Duration)



[^1] edge and applicable compression edges of the beam.

# VERSA-LAM ${ }^{\circledR}$ Allowable Nailing and Design Values 

## Closest Allowable Nail Spacing

VERSA-LAM ${ }^{\circledR}$ \& VERSA-RIM ${ }^{\circledR}$ Products

| Nail Size |  |  |  |  |  |  | Nailing Perpendicular to Glue Lines (Wide Face) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \text { VERSA-LAM } ® \\ 1.4 \text { 1800 Rimboard } \\ 15 / 16^{\prime \prime} \end{gathered}$ |  | $\underset{13 / 4^{\prime \prime}}{\text { VERSA }^{\circledR}}$ |  | VERSA-LAM ${ }^{\circledR}$ 3½" \& Wider |  | All Products |  |
|  | O.C. [inches] | End [inches] | $\begin{gathered} \text { O.C. } \\ \text { [inches] } \end{gathered}$ | End [inches] | $\begin{aligned} & \text { O.C. } \\ & \text { [inches] } \end{aligned}$ | End [inches] | $\begin{aligned} & \text { O.C. } \\ & \text { [inches] } \end{aligned}$ | End [inches] |
| 8d Box | 3 | $11 / 2$ | 2 | 1 | 2 | $1 / 2$ | 2 | 1/2 |
| 8d Common | 3 | 2 | 3 | 2 | 2 | 1 | 2 | 1 |
| 10d \& 12d Box | 3 | 2 | 3 | 2 | 2 | 1 | 2 | 1 |
| 16d Box | 3 | 2 | 3 | 2 | 2 | 1 | 2 | 1 |
| 10d \& 12d Common | 4 | 3 | 4 | 3 | 2 | 2 | 2 | 2 |
| 16d Sinker | 4 | 3 | 4 | 3 | 2 | 2 | 2 | 2 |
| 16d Common | 6 | 4 | 6 | 3 | 2 | 2 | 2 | 2 |

- Offset and stagger nail rows from floor sheathing and wall sole plate.
- Simpson Strong-Tie A35 and LPT4 connectors may be attached to the side VERSA-LAM ${ }^{\circledast} /$ VERSA-RIM ${ }^{\oplus}$. Use nails as specified by Simpson Strong-Tie.

Nailing Parallel to Glue Lines
(Narrow Face)


Nailing Perpendicular to Glue Lines (Wide Face)
Nailing Notes

1) For $13 / 4$ " thickness and greater, 2 rows of nails (such as for a metal strap) are allowed (use $1 / 2^{\prime \prime}$ minimum offset between rows and stagger nails).

## VERSA-LAM ${ }^{\circledR}$ Design Values

| Grade | Width [in] | Depth [in] | Weight [lb/ft] | Allowable Shear [lb] | Allowable Moment [ft-lb] | Moment of Inertia [in4] | Grade | Width [in] | Depth <br> [in] | Weight [lb/ft] | Allowable Shear [lb] | Allowable Moment [ft-lb] | Moment of Inertia [in4] |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 11/2 | 3112 | 1.5 | 998 | 776 | 5.4 |  | $51 / 4$ | $51 / 4$ | 8.0 | 5237 | 6830 | 63.3 |
|  |  | $51 / 2$ | 2.4 | 1568 | 1821 | 20.8 |  |  | 51/2 | 8.4 | 5486 | 7457 | 72.8 |
|  |  |  |  |  |  |  |  |  | 71/4 | 11.0 | 7232 | 12566 | 166.7 |
|  |  | 71/4 | 3.2 | 2066 | 3069 | 47.6 |  |  | 91/4 | 14.1 | 9227 | 19908 | 346.3 |
|  |  | $31 / 2$ | 1.8 | 1164 | 1058 | 6.3 |  |  | 91/2 | 14.5 | 9476 | 20937 | 375.1 |
|  |  | 51/2 | 2.8 | 1829 | 2486 | 24.3 |  |  |  |  |  |  |  |
|  |  | 711/4 | 3.7 | 2411 | 4189 | 55.6 |  |  | 111/4 | 17.1 | 11222 | 28814 | 622.9 |
|  |  | 91/4 | 4.7 | 3076 | 6636 | 115.4 |  |  | 117/8 | 18.1 | 11845 | 31913 | 732.6 |
|  |  | 91/2 | 4.8 | 3159 | 6979 | 125.0 |  |  | 14 | 21.3 | 13965 | 43552 | 1200.5 |
|  | $13 / 4$ | 111/4 | 5.7 | 3741 | 9605 | 207.6 |  |  | 16 | 24.4 | 15960 | 56046 | 1792.0 |
|  |  | 117/8 | 6.0 | 3948 | 10638 | 244.2 |  |  |  |  |  |  |  |
|  |  | 14 | 7.1 | 4655 | 14517 | 400.2 |  |  | 18 | 27.4 | 17955 | 70011 | 2551.5 |
|  |  | 16 | 8.1 | 5320 | 18682 | 597.3 |  |  | 20 | 30.4 | 19950 | 85428 | 3500.0 |
|  |  | 18 | 9.1 | 5985 | 23337 | 850.5 |  |  | 24 | 36.5 | 23940 | 120549 | 6048.0 |
|  |  | 24 | 12.2 | 7980 | 40183 | 2016.0 |  |  | 911/4 | 16.6 | 12303 | 26544 | 461.7 |
|  |  | 51/2 | 5.6 | 3658 | 4971 | 48.5 |  |  | 9112 | 17.1 | 12635 | 27916 | 500.1 |
|  |  | $71 / 4$ | 7.4 | 4821 | 8377 | 111.1 |  |  | 9/2 | 17.1 | 12635 | 27916 |  |
|  |  | 91/4 | 9.4 | 6151 | 13272 | 230.8 |  |  | 111/4 | 20.2 | 14963 | 38419 | 830.6 |
|  |  | 91/2 | 9.6 | 6318 | 13958 | 250.1 |  |  | 117/8 | 21.4 | 15794 | 42550 | 976.8 |
|  | $311 / 2$ | 111/4 | 11.4 | 7481 | 19210 | 415.3 |  | 7 | 14 | 25.2 | 18620 | 58069 | 1600.7 |
|  | $31 / 2$ | 117/8 | 12.1 | 7897 | 21275 | 488.4 |  |  | 16 | 28.8 | 21280 | 74728 | 2389.3 |
|  |  | 14 | 14.2 | 9310 | 29035 | 800.3 |  |  | 18 | 32.4 | 23940 | 93348 | 3402.0 |
|  |  | 16 | 16.2 | 10640 | 37364 | 1194.7 |  |  |  |  |  |  |  |
|  |  | 18 | 18.3 | 11970 | 46674 | 1701.0 |  |  | 20 | 36.0 | 26600 | 113904 | 4666.7 |
|  |  | 20 | 20.3 | 13300 | 56952 | 2333.3 |  |  | 24 | 43.2 | 31920 | 160732 | 8064.0 |


|  |  | Modulus of Elasticity | Bending | Horizontal Shear | Tension Parallel to Grain | Compression Parallel to Grain | Compression Perpendicular to Grain | Equivalent Specific Gravity for Fastener Design |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Design Property | Grade | $\mathrm{E}\left(\mathrm{x} 10^{6} \mathrm{psi}\right)^{(1)}$ | $\mathrm{F}_{\mathrm{b}}(\mathrm{psi})^{(2)(3)}$ | $\mathrm{F}_{\mathrm{v}}(\mathrm{psi})^{(2)(4)}$ | $\mathrm{F}_{\mathrm{t}}(\mathrm{psi})^{(2)(5)}$ | $\mathrm{F}_{\text {cll }}(\mathrm{psi})^{(2)}$ | $\mathrm{F}_{\mathrm{c}} \perp(\mathrm{psi})^{(1)(6)}$ | (SG) |
| VERSA-LAM® Beams | 2.03100 | 2.0 | 3100 | 285 | 2150 | 3000 | 750 | 0.5 |
| VERSA-LAM® Studs | 1.72650 | 1.7 | 2650 | 285 | 1650 | 3000 | 750 | 0.5 |
| VERSA-LAM ${ }^{\text {® }}$ Columns | 1.82750 | 1.8 | 2750 | 285 | 1825 | 3000 | 750 | 0.5 |

1. This value cannot be adjusted for load duration.
2. This value is based upon a load duration of $100 \%$ and may be adjusted for other load durations.
3. Fiber stress bending value shall be multiplied by the depth factor, $(12 / \mathrm{d})^{1 / 9}$ where $\mathrm{d}=$ member depth [in].
4. Stress applied perpendicular to the gluelines.

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5. Tension value shall be multiplied by a length factor, (4/L) $)^{1 / 8}$ where L $=$ member length $[\mathrm{ft}]$. Use $\mathrm{L}=4$ for members less than four feet long.
6. Stress applied parallel to the gluelines.

* Design properties are limited to dry conditions of use where the maximum moisture content of the material will not exceed $16 \%$.



## VERSA-STUD® 1.72650

## Allowable Design Values

| Product | Bending <br> $\mathrm{F}_{\mathrm{b}}[\mathrm{psi}]$ | Compression <br> Parallel to Grain <br> $\mathrm{F}_{\mathrm{c}}[\mathrm{psi}]$ | Modulus of <br> Elasticity <br> $\mathrm{E}[\mathrm{psi}]$ | Horizontal <br> Shear <br> $\mathrm{F}_{\mathrm{v}}[\mathrm{psi}]$ |
| :---: | :---: | :---: | :---: | :---: |
| VERSA-STUD® 1.7 2650 | $\mathbf{2 6 5 0}$ | $\mathbf{3 0 0 0}$ | $\mathbf{1 , 7 0 0 , 0 0 0}$ | $\mathbf{2 8 5}$ |
| Spruce Pine Fir (North) \# 1/2 Grade | 875 | 1150 | $1,400,000$ | 135 |
| Hem-Fir\# Grade | 850 | 1300 | $1,300,000$ | 150 |
| Western Woods \# 2 Grade | 675 | 900 | $1,000,000$ | 135 |

## Notes:

- Design values are for loads applied to the narrow face of the studs.
- Dimension lumber values taken from 2012 NDS Supplement: Design Values for Wood Construction (per 2012 IBC/IRC).
- Repetitive member and size factors have not been applied.

For further design information, please see VERSA-STUD 1.72650 Eastern Tall Wall Guide.

## BC FRAMER

New BC FRAMER ${ }^{\circledR}$ represents a huge technological leap to help you improve the efficiency and profitability of your engineered wood products business. Boise Cascade will provide you what we believe is now the industry's best design software, offering far greater productivity than even our current version of BC FRAMER®. This new software package will help your design department work faster and accomplish more. You don't get paid to do drawings, but at least now you can do them in less time, and better.

- Shrink design time with BC FRAMER® model sharing.
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- Experience the efficiencies of BC FRAMER® whole house modeling.
- Draw floor and wall plans simultaneously with BC FRAMER ${ }^{\circledR}$ :
- Check the plan every possible way with BC FRAMER ${ }^{\circledR}$ full 3-D viewer.
- Create a master plan and multiple options that can be quickly selected and exported to a plot-specific file in a few minutes a fraction of the time it could have taken in the past.
Information can also be obtained at 1-800-405-5969 or email us at EWPSupport@BC.com.



## BC CALC® Sizing Software

This generation of BC CALC® was redesigned from the ground up with a focus on productivity and expanded functionality. BC CALC® ${ }^{\circledR}$ performs engineering analysis to help our customers size beams, joists, columns, studs and tall walls for their building projects. It is simple to use, yet flexible enough to analyze a variety of common applications. The user enters the member geometry, adds loads, holes and other relevant data and then selects a product for analysis. The program helps the user identify which of Boise Cascade's engineered wood products meet the demands of the application.
After the analysis has been run, the user can create an easy-toread PDF design report that clearly shows important span and load information as well as the analysis results.

Boise Cascade has
provided BC CALC® free of charge to the design community since 1994.


Analysis for Engineered Wood Products

## COMPUTER REQUIREMENTS

BC CALC ${ }^{\circledR}$ is now a web-based application available online and can be used on Windows or Apple operating systems via Internet Explorer, Edge, Chrome or Safari browsers. Additionally, iOS and Android tablets are now supported. For questions regarding BC CALC ${ }^{\circledR}$, call 1-800-405-5969 or email EWPSupport@BC.com.


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| Single Joist - Top Flange |  |  |  |  |  | Single Joist - Face Mount |  |  |  |  |  | Face Mount Skewed $45^{\circ}$ Joist Hanger |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| JoistDepth |  |  |  |  |  |  |  |  |  |  |  | Joist Depth |  |  | SUR/L |  |  |
|  | AJS® | Hanger | $\begin{aligned} & \text { Capacity } \\ & {[\mathrm{lbs}]} \end{aligned}$ | Header | ailing | Joist Depth | AJS® | Hanger | Capacity [lbs] | Header | $\text { ailing }_{\text {Joist }}$ |  |  |  |  |  |  |
| 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | ITS2.56/9.5 | 1006 | 6-10d | - | 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | IUS2.56/9.5 | 950 | 8-10d | - |  | AJS® | Hanger | Capacity [lbs] |  | Nailing |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Head |  |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | ITS3.56/9.5 | 1006 | 6-10d | - |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | IUS3.56/9.5 | 1006 | 10-10d | - | 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | SUR/L2.56/9 | 1139 | 14-16d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |
| 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | ITS2.56/11.88 | 1020 | 6-10d | - | 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | IUS2.56/11.88 | 1020 | 10-10d | - |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | SUR/L410 | 1076 | 14-16d | 6-16d |
|  | 25 |  |  |  |  |  | 25 |  |  |  |  | 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | SUR/L2.56/11 | 1174 | 16-16d | $2-10 \mathrm{dx} 1$ 1/2" |
|  | 30 | ITS3.56/11.88 | 1020 | 6-10d | - |  | 30 | IUS3.56/11.88 | 1020 | 12-10 | - |  | 25 | SUR/L410 | 1101 | 14-16d | 6-16d |
| 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | ITS2.56/14 | 1032 | 6-10d | - | 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | IUS2.56/14 | 1032 | 12-10d | - | 14" | $\begin{gathered} 30 \\ 20 \\ 190 \end{gathered}$ | SUR/L2.56/14 | 1204 | 18-16d | $2-10 \mathrm{~d} \times 11 / 2^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | ITS3.56/14 | 1032 | 6-10d | - |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | IUS3.56/14 | 1032 | 12-10d | - |  | 25 30 | SUR/L414 | 1123 | 18-16d | 8-16d |
| 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | ITS2.56/16 | 1048 | 6-10d | - | 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | IUS2.56/16 | 1048 | 14-10d | - | 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | SUR/L2.56/14 | 1235 | 18-16d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | ITS3.56/16 | 1048 | 6-10d | - |  | 25 30 | IUS3.56/16 | 1048 | 14-10d | - |  | 25 30 | SUR/L414 | 1127 | 18-16d | 8-16d |
| Double Joist - Top Flange |  |  |  |  |  | Double Joist - Face Mount |  |  |  |  |  | Field Slope and Skew Joist Hanger |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  | Joist Depth |  |  |  |  |  |
| Joist | AJS® | Hanger | Capacity | Nailing |  |  |  |  |  |  |  | AJS® | Hanger | Capacity [lbs] | Nailing |  |
| Depth |  | Hanger |  | Header | Joist | Joist <br> Depth |  |  | [lbs] | Header | Joist |  |  |  |  | Header | Joist |
| 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MIT39.5-2 | 2125 | 8-16d | 2-10dx11⁄2" | 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MIU5.12/9 | 2125 | 16-16d | 2-10dx11⁄2" |  | $91 / 2$ " | $\begin{gathered} 20 \\ 190 \end{gathered}$ | LSSUH310 | 1480 | 14-10d | $12-10 \mathrm{dx} 1^{112}{ }^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | B7.12/9.5 | 2720 | 14-16d | 6-16d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | HU410-2 | 2680 | 18-16d | 8-16d | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ |  | LSSU410 | 1480 | 14-10d | 12-10dx $1^{1 / 2}{ }^{\prime \prime}$ |
| 117/8" | 20 | MIT311.88-2 | 2170 | 8-16d | 2-10dx11⁄2 ${ }^{\text {" }}$ | 117/8" | 20 | MIU5.12/11 | 2170 | 20-16d | $2-10 \mathrm{dx1} 11 / 2^{\prime \prime}$ | 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | LSSUH310 | 1595 | 14-10d | 12-10dx11⁄2" |
|  | 190 25 | B7.12/11.88 | 2930 | 14-16d | 6-16d |  |  | HU412-2 | 2930 | 22-16d | 8-16d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | LSSU410 | 1595 | 14-10d | 12-10dx $1^{1 / 2}{ }^{\prime \prime}$ |
|  | 30 |  |  |  |  |  | 30 |  |  |  |  | 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | LSSUH310 | 1600 | 14-10d | 12-10dx11/2" |
| 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MIT314-2 | 2210 | 8-16d | $2-10 \mathrm{dx1} 1 / 2^{\prime \prime}$ | $14 "$ | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MIU5.12/14 | 2210 | 22-16d | 2-10dx11⁄2" |  | 190 25 30 | LSSU410 | 1625 | 14-10d | $12-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | B7.12/14 | 3120 | 14-16d | 6-16d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | HU414-2 | 3120 | 26-16d | 12-16d | $16 "$ | $\begin{gathered} 20 \\ 190 \end{gathered}$ | - | - | - | - |
| 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MIT5.12/16 | 2255 | 8-16d | $2-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ | $16 "$ | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MIU5.12/16 | 2255 | 24-16d | $2-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ |  | 25 30 | - | - | - | - |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | B7.12/16 | 3305 | 14-16d | 6-16d |  | 25 30 | HU414-2 | 3305 | 26-16d | 12-16d | SMMPSON |  |  | For more information, call Simpson Strong-Tie |  |  |
| Adjustable Height Joist Hanger |  |  |  |  |  | Variable Pitch Joist Connector |  |  |  |  |  | Strong-Tie $\quad$ at ${ }^{\oplus} \quad$ at $1-800-999-5099$ |  |  |  |  |  |
| THAI |  |  |  |  |  |  |  |  |  |  |  | General Notes <br> - Bold Italic hangers require web stiffeners. <br> - Capacities will vary with different nailing criteria and/or support conditions; contact supplier or Simpson Strong-Tie for further information. <br> - Capacity values shown are either hanger capacity values (see support requirements below) or AJS ${ }^{\circledast}$ Joist end reaction capacities - whichever is less. <br> - All capacity values are downward loads at $100 \%$ load duration. <br> - Use sloped seat hangers and beveled web stiffeners when |  |  |  |  |  |
| Joist Depth | AJS® | Hanger | $\begin{gathered} \text { Capacity } \\ \text { [bs] } \end{gathered}$ | Nailing |  | Joist <br> Depth | AJS® | Hanger | Capacity[lbs] Top Plate Rafter |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | Header | Joist | 91/2" | 20 |  |  |  |  | AJS ${ }^{\circledR}$ Joist slope exceeds $1 / 4$ " per foot. |  |  |  |  |  |
| 91/2" | 190 | THAI2.06/22 | 21181 | 6-10d | 2-10dx11/2" |  | 190 | VPA3 | 1006 | 9-10d | 2-10dx11⁄2" | - Leave $1 / 16^{\prime \prime}$ clearance ( $1 / 8^{\prime \prime}$ maximum) between the end of the supported joist and the head of the hanger. |  |  |  |  |  |
|  | 25 30 | THAI322 | 1393 | 6-10d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | VPA4 | 1006 | 11-10d | 2-10dx11/2" |  |  |  |  |  |  |  |  |  |  |  |
| 117/8" | 20 190 | THAI2.06/22 | 21443 | 6-10d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ | 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | VPA3 | 1020 | 9-10d | $2-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ | - For proper installation of the VPA, the $2-10 \mathrm{~d} \times 1^{1 / 2 "}$ joist nails through the bend tabs must be installed at approximately a 45 degree angle. |  |  |  |  |  |
|  | 25 30 | THA1422 | 1715 | 6-10d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |  | 25 30 | VPA4 | 1020 | 11-10d | $2-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ |  | $5 \text {-degree }$ port Re | ngle. <br> uirements |  |  |  |
| 14" | 20 190 | THAI2.06/22 | 21600 | 6-10d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ | 14" | 20 | VPA3 | 1032 | 9-10d | $2-10 \mathrm{dx} 1 \frac{1}{2}{ }^{\prime \prime}$ | - Support material assumed to be Boise Cascade structural composite lumber or sawn lumber (Douglas fir or southern |  |  |  |  |  |
|  | 25 30 | THAI3522 | 1600 | 6-10d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |  | 25 30 | VPA4 | 1032 | 11-10d | 2-10dx $11 / 2^{\prime \prime}$ |  | ine spec inimum | s). |  | double-joi | ist top mount |
| 16" | 20 | THAI3522 | 1582 | 6-10d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ | 16" | 20 | VPA3 | 1048 | 9-10d | $2-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ |  | angers is inimum | port width for fa |  | thangers | with 10 d and |
|  | 25 30 | THA1422 | 1715 | 6-10d | $2-10 \mathrm{dx1} 1 / 2^{\prime \prime}$ |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | VPA4 | 1048 | 11-10d | $2-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ |  |  |  |  |  |  |


| Single Joist - Top Flange |  |  |  |  |  | Single Joist - Face Mount |  |  |  |  |  | Face Mount Skewed $45^{\circ}$ Joist Hanger |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Joist Depth | AJS® | Hanger | Capacity |  | ailing Joist | Joist Depth | AJS® | Hanger | Capacity |  | ailing | Joist Depth | AJS® | Hanger | Capacity [lbs] |  | ${ }_{\text {ailing }}$ |
|  |  |  |  | Header | Joist |  |  |  |  | Header | Joist |  |  |  |  | Header | Joist |
| 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | TFL2595 | 1006 | 6-10d | 2-10dx $11 / 2{ }^{\prime \prime}$ | 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THF25925 | 1062 | 12-10d | 2-10dx $11 / 2^{\prime \prime}$ | 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | SKH2520L/R | 992 | 14-10d | $10-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | THO35950 | 1048 | 10-10d | 2-10dx $11 / 2{ }^{\prime \prime}$ |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | THF35925 | 1062 | 12-10d | 2-10dx $11 / 2^{\prime \prime}$ |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | SKH410L/R | 1062 | 16-16d | 10-16d |
| 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | TFL25118 | 1020 | 6-10d | 2-10dx $11 / 2{ }^{\prime \prime}$ | 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THF25112 | 1085 | 14-10d | 2-10dx $11 / 2^{\prime \prime}$ | 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | SKH2520L/R | 1003 | 14-10d | $10-10 \mathrm{dx1} 11 / 2^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | THO35118 | 1068 | 10-10d | 2-10dx $11 / 2$ |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | THF35112 | 1085 | 16-10d | 2-10dx $11 / 2{ }^{\prime \prime}$ |  | 25 30 | SKH410L/R | 1085 | 16-16d | 10-16d |
| 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | TFL2514 | 1032 | 6-10d | 2-10dx $11 / 2^{\prime \prime}$ | 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THF25140 | 1105 | 18-10d | 2-10dx $11 / 2^{\prime \prime}$ | 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | SKH2524L/R | 1014 | 16-10d | 10-10dx11⁄2" |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | THO35140 | 1086 | 12-10d | 2-10dx11⁄2" |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | THF35140 | 1105 | 20-10d | 2-10dx $11 / 2{ }^{\prime \prime}$ |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | SKH414L/R | 1105 | 22-16d | 10-16d |
| 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | TFL2516 | 1048 | 6-10d | 2-10dx $11 / 2^{\prime \prime}$ | 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THF25160 | 1127 | 22-10d | 2-10dx $11 / 2{ }^{\prime \prime}$ | 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | SKH2524L/R | 1029 | 16-10d | 10-10dx11⁄2" |
|  | 25 30 | THO35160 | 1107 | 12-10d | 2-10dx $11 / 2$ |  | 25 30 | THF35157 | 1127 | 22-10d | $2-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |  | 25 30 | SKH414L/R | 1127 | 22-16d | 10-16d |
| Double Joist - Top Flange |  |  |  |  |  | Double Joist - Face Mount |  |  |  |  |  | Field Slope and Skew Joist Hanger |  |  |  |  |  |
| $\begin{aligned} & \text { Joist } \\ & \text { Depth } \end{aligned}$ | THO Double |  |  |  |  | THF Double <br> HD |  |  |  |  |  |  |  |  |  |  |  |
|  | AJS® | Hanger | Capacity |  | ailing | Joist | AJS® | Hanger | Capacity |  | ailing | Joist | AJS® | Hanger | Capacity |  | Joist |
|  | AJS | Hanger | [lbs] | Header | Joist | Depth | AJS | Hanger | [lbs] | Header | Joist | Depth | AJS | Hanger | [lbs] | Header | Joist |
| 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THO25950-2 | 2237 | 10-16d | 6-10d | 91/2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THF25925-2 | 1390 | 12-10d | 6-10d | 9112" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | LSSH25 | 1420 | 14-16d | $12-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | BPH7195 | 2690 | 10-16d | 6-10d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | HD7100 | 1690 | 12-10d | 6-10d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | LSSH35 | 1420 | 14-16d | $12-10 \mathrm{dx} 1^{112} \mathbf{2}^{\prime \prime}$ |
| 117/8" | 2 | THO25118-2 | 2300 | 10-16d | 6-10d | 117/8" | 20 | THF25112-2 | 1855 | 16-10d | 6-10d | 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | LSSH25 | 1530 | 14-16d | $12-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |
|  |  | BPH71118 |  |  |  |  |  |  |  |  |  |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | LSSH35 | 1530 | 14-16d | $12-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ |  | 3060 | 10-16d | 6-10d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | HD7120 | 2255 | 16-10d | 6-10d | 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | LSSH25 | 1630 | 14-16d | 12-10dx $11 / 2^{\prime \prime}$ |
| 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THO25140-2 | 2355 | 12-16d | 6-10d | 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THF25140-2 | 2210 | 20-10d | 6-10d |  | 25 30 | LSSH35 | 1630 | 14-16d | $12-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | BPH7114 | 3260 | 10-16d | 6-10d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | HD7140 | 2820 | 20-10d | 8-10d | 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | LSSH35 | 1725 | 14-16d | $12-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |
| 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THO25160-2 | 2412 | 12-16d | 6-10d | 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | THF25160-2 | 2255 | 24-10d | 8-10d |  | 25 30 | LSSH35 | 1725 | 14-16d | $12-10 \mathrm{dx} 11 / 2^{\prime \prime}$ |
|  | 25 30 | BPH7116 | 3452 | 10-16d | 6-10d |  | 25 30 | HD7160 | 3305 | 24-10d | 8-10d | For more information, contact USP Structural Connectors at 1-800-328-5934 or |  |  |  |  |  |
| Adjustable Height Joist Hanger |  |  |  |  |  | Variable Pitch Joist Connector |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  | TMP |  |  |  |  |  | General Notes <br> www.uspconnectors.com <br> - Bold Italic hangers required web stiffeners. <br> - Capacities will vary with different nailing criteria and/or support conditions: contact supplier or USP Structural Connectors for further information. |  |  |  |  |  |
| Joist Depth | AJS® | Hanger | Capacity [lbs] | Header | $\begin{aligned} & \text { Jailing } \\ & \text { Joist } \end{aligned}$ | Joist Depth | AJS® | Hanger | Capacity [lbs] | $\begin{array}{r} \mathrm{Fa} \\ \hline \text { Top Plate } \end{array}$ | stener Rafter |  |  |  |  |  |  |
| 91⁄2" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MSH322 | 1270 | 16-10d | 4-10dx11/2" | $9112 "$ | $\begin{gathered} 20 \\ 190 \end{gathered}$ | TMP25 | 1175 | 6-10d | $4-10 \mathrm{dx} 1 \frac{1}{2}{ }^{\prime \prime}$ | Capacity values shown are either hanger capacity values (see support requirements below) or AJS® Joist end reaction capacities - whichever is less. All capacity values are downward loads at $100 \%$ load |  |  |  |  |  |
|  | 25 30 | MSH422IF | 1270 | 22-10d | 4-10d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | TMP4 | 1175 | 6-10d | $4-10 \mathrm{dx} \times 1 / 2^{\prime \prime}$ |  | ll capac uration | ity values are d | ownward | oads at 1 | 0\% load |
| 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MSH322 | 1367 | 16-10d | 4-10dx11/2" | 117/8" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | TMP25 | 1215 | 6-10d | $4-10 \mathrm{dx} 1^{1 / 2}{ }^{\prime \prime}$ | Use sloped seat hangers and beveled web stiffeners when AJS® Joist slope exceeds $1 / 4$ per foot. <br> Leave $1 / 16^{\prime \prime}$ clearance ( $1 / 8$ " maximum) between the end of the supported joist and the head of the hanger. <br> - For AJS® Joist applications, consult USP for capacity reduction. |  |  |  |  |  |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | MSH422IF | 1367 | 22-10d | 4-10d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | TMP4 | 1215 | 6-10d | 4-10dx $1^{1 / 2}{ }^{\prime \prime}$ |  |  |  |  |  |  |
| 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MSH322 | 1455 | 16-10d | 4-10dx $1^{1 / 2}{ }^{\prime \prime}$ | 14" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | TMP25 | 1250 | 6-10d | 4-10dx $1^{1 / 2}{ }^{\prime \prime}$ | Support Requirements <br> - Support material assumed to be Boise Cascade structural composite lumber or sawn lumber (Douglas fir or southern pine species). <br> - Minimum support width for single- and double-joist top mount hangers is 3 ": ( $11 / 2^{\prime \prime}$ for THO hangers). <br> - Minimum support width for face mount hangers with 10d and 16 d nails is $13 / 4$ " and $2 "$, respectively. |  |  |  |  |  |
|  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | MSH422IF | 1455 | 22-10d | 4-10d |  | $\begin{aligned} & 25 \\ & 30 \end{aligned}$ | TMP4 | 1250 | 6-10d | 4-10dx $1^{1 / 2}{ }^{\prime \prime}$ |  |  |  |  |  |  |
| 16" | $\begin{gathered} 20 \\ 190 \end{gathered}$ | MSH322 | 1413 | 16-10d | 4-10dx $1^{1 / 2}{ }^{\prime \prime}$ | $16 "$ | $\begin{gathered} 20 \\ 190 \end{gathered}$ | TMP25 | 1285 | 6-10d | 4-10dx $1^{1 / 2}{ }^{\prime \prime}$ |  |  |  |  |  |  |
|  | 25 30 | MSH422IF | 1413 | 22-10d | 4-10d |  | 25 30 | TMP4 | 1285 | 6-10d | 4-10dx $1^{1 / 2}{ }^{\prime \prime}$ |  |  |  |  |  |  |

Notes
NOTES
Boise Cascade Engineered Wood Products

## NOTES

Boise Cascade Engineered Wood Products

Boise Cascade Engineered Wood Products has a proven track record of providing quality wood products and a nationwide building materials distribution network for our customers, helping them to enhance their own businesses.

Boise Cascade Engineered Wood Products build better homes with stronger, stiffer floors using only wood purchased in compliance with a number of green building programs. Take a moment to view our sustainability certification site at http://www.bc.com/ sustainability/certification-audits/ or view our green brochure at www.bc.com/inst11.

Boise Cascade Engineered Wood Products throughout North America can now be ordered FSC ${ }^{\circledR}$ Chain-of-Custody (COC) certified, enabling homebuilders to achieve LEED® points residential and commercial green building programs including LEED for Homes and LEED for New Construction. Boise Cascade Engineered Wood Products are available as PEFC® ${ }^{\circledR}$ Chain-of-Custody certified, SFI® Chain-of-Custody certified and SFI® FiberSourcing certified, as well as NAHB Research Center Green Approved, enabling homebuilders to also obtain green building points through the Green Building Standards.

## Lifetime Guaranteed Quality and Performance

Boise Cascade warrants its $\mathrm{BCI}{ }^{\circledR}$ Joist, VERSA-LAM ${ }^{\circledR}$, and ALLJOIST® products to comply with our specifications, to be free from defects in material and workmanship, and to meet or exceed our performance specifications for the normal and expected life of the structure when correctly stored, installed and used according to our Installation Guide.


BOISE CASCADE, TREE-IN-A-CIRCLE, AJS, ALLJOIST, BCI, BC CALC, BC COLUMN, BC FRAMER, BOISE CASCADE RIM BOARD, BOISE GLULAM, SIMPLE FRAMING SYSTEM, VERSA-LAM, VERSA-RIM, VERSA-STRAND, and VERSA-STUD are trademarks of Boise Cascade Company or its affiliates.

For information about Boise Cascade Engineered Wood Products, including sales terms and conditions, warranties and disclaimers,
visit our website at www.BCewp.com
Your Dealer is:


[^0]:    Total Load values are limited by shear, moment or deflection equal to L/180. Total Load values are the capacity of the beam in addition to its own weight.
    Live Load values are limited by deflection equal to $L / 240$. Check the local building code for other deflection limits that may apply.
    Where a Live Load value is not shown, the Total Load value will control.
    Table values represent the most restrictive of simple or multiple span applications. Span is measured center to center of the supports. Analyze multiple span beams with the BC CALC® software if the length of any span is less than half the length of an adjacent span.
    Table values assume that lateral support is provided at each support and continuously along the top edge and applicable compression edges of the beam.

    Table values for Minimum Required Bearing Lengths are based on the allowable compression design value perpendicular to grain for the beam and the Total Load value shown. Other design considerations, such as a weaker support material, may warrant longer bearing lengths. Table values assume that support is provided across the full width of the beam.

    - For 2-ply, 3-ply or 4-ply beams; double, triple or quadruple Allowable Total Load and Allowable Live Load values. Minimum Required Bearing Lengths remain the same for any number of plies.
    - $13 / 4$ inch members deeper than 14 inches are to be used as multiple-member beams only.

    This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® software.

[^1]:    Total Load values are limited by shear, moment or deflection equal to L/180. Total Load values are the capacity of the beam in addition to its own weight.

    - Live Load values are limited by deflection equal to $L / 240$. Check the local building code for other deflection limits that may apply.
    - Where a Live Load value is not shown, the Total Load value will control.
    - Table values represent the most restrictive of simple or multiple span applications. Span is measured center to center of the supports. Analyze multiple span beams with the BC CALC® ${ }^{\circledR}$ software if the length of any span is less than half the length of an adjacent span.
    - Table values assume that lateral support is provided at each support and continuously along the top

    Table values for Minimum Required Bearing Lengths are based on the allowable compression design value perpendicular to grain for the beam and the Total Load value shown. Other design considerations, such as a weaker support material, may warrant longer bearing lengths. Table values assume that support is provided across the full width of the beam.
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